



	ABBREV	GENERA	\L LEG		
A.C.T. ADJ	ACOUSTICAL CEILING TILE ADJUSTABLE	JAN	JANITOR	(A)	COLUMN LIN
A.F.F. AL	ABOVE FINISHED FLOOR ALUMINUM	K.P.	KICK PLATE		
APPROX ARCH	APPROXIMATE ARCHITECTURAL	LAM LAV	LAMINATE LAVATORY		ELEVATION P
BD	BOARD	LBS LCP	POUNDS LEASE CONFIRMATION PLAN		— DRAWING NO
LDG	BUILDING	LH	LEFT HAND	2 A201 4	— PAGE NO.
_KG V	BLOCKING BEAM	LL LT	LANDLORD LIGHT TRACK		
0.	BOTTOM OF	MAHOG	MAHOGANY	5	SECTION KE
LG LR	CEILING CLEAR	MAX MECH	MAXIMUM MECHANICAL		
.M.U. .O.	CONCRETE MASONRY UNIT CASED OPENING	MIN MISC	MINIMUM MISCELLANEOUS	A101	— PAGE NO.
OL	COLUMN CONCRETE	M.O. MTD	MASONRY OPENING MOUNTED		171021101
ONSTR ONT	CONSTRUCTION CONTINUOUS	MTL MAT/MTL	METAL MATERIAL		ENLARGED A
PM ANAGER	CONSTRUCTION PROJECT	NAT	NATURAL FINISH		— ENLARGED A
PT TR	CARPET COUNTER	N.I.C. NO	NOT IN CONTRACT		
IR .T.	COUNTER CERAMIC TILE	NOM	NUMBER NOMINAL		
ET	DETAIL	N.T.S.	NOT TO SCALE		— DETAIL NUMI
A M	DIAMETER DIMENSION	PLUMB PLAM/PLM	PLUMBING PLASTIC LAMINATE	A101 -	— SHEET NUME
SP N	DISPENSER DOWN	PL PR	PLATE PAIR		
.O. R	DOOR OPENING DOOR	PT PTN	PAINT/PAINTED PARTITION		
ŴG	DRAWING	PLYWD	PLYWOOD		ROOM NO. TA
	EACH	Q.T.	QUARRY TILE		
۲.JT.	EXPANSION JOINT EQUAL	RAD	RADIUS		
QUIP W.C.	EQUIPMENT ELECTRIC WATER COOLER	REC REQ	RECESSED REQUIRED	1	PARTITION T
(H (STG	EXHAUST EXISTING	RH RM	RIGHT HAND ROOM		
ХT	EXTERIOR	R.O. RWD	ROUGH OPENING REDWOOD		DOOR NUMB
A. E.	FIRE ALARM FIRE EXTINGUISHER	R.W.L.	RAIN WATER LEADER		Doorthomb
E.C. N	FIRE EXTINGUISHER CABINET	S.C. S.F.	SOLID CORE SQUARE FOOT/FEET	$\langle 23 \rangle$	KEYNOTE
(T	FIXTURE FLUORESCENT	SHT SIM	SHEET SIMILAR		REINOTE
).C.	FACE OF CONCRETE	SPEC	SPECIFICATION	11'-0"	ELEVATION H
.S.	FACE OF STUDS FIRE RETARDANT	SQ S.S.	SQUARE STAINLESS STEEL	A.F.F.	ELEVATION
ATMENT	FOOT/FEET	STD STL	STANDARD STEEL		
RR C.	FURRING FIXTURE CONTRACTOR	T&G	TONGUE AND GROOVE		EXISTING PA TO REMAIN
;	GENERAL CONTRACTOR	THR T.O.	THRESHOLD TOP OF		NEW PARTIT
 WB	GLASS GYPSUM WALL BOARD	TYP	TYPICAL		EXISTING CC
Έ Έ	GYPSUM	U.O.N.	UNLESS OTHERWISE NOTED		TO BE REMO
C.	HOLLOW CORE	VCT	VINYL COMPOSITE TILE	-	
WD M.	HARDWOOD HOLLOW METAL	VERT	VERTICAL		NEW DOOR
AC	HEATING VENTILATING & AIR CONDITIONING	W/ W.C.	WITH WATER CLOSET		
V.H. V.	HOT WATER HEATER HOT WATER	WD W/O	WOOD WITHOUT		EXISTING DO
)R	HORIZONTAL			//	
	INSIDE DIAMETER INCH				
SUL T	INSULATION INTERIOR			CH GWB -	— CEILING HEI — CEILING MAT
CL	INCLUDES				
					FINISH TAG
				FL 01	
					ALIGN
					— AREA OF RE
					BREAK LINE
					CENTER LIN
					N.I.C NOT I

170 Mt Vernon St, West Roxbury, MA 02132

Permit Set 05/25/2022

Project Description:

EGEND

N LINE

ION KEY NG NO. 10.

N KEY NG NO.

GED AREA / DETAIL SYMBOL GED AREA

NUMBER NUMBER

NO. TAG

ION TYPE

UMBER

ION HEIGHT

IG PARTITION IAIN

RTITION

G CONSTRUCTION REMOVED

NG DOOR TO REMAIN

HEIGHT MATERIAL

OF REVISION

R LINE

NOT IN CONTRACT

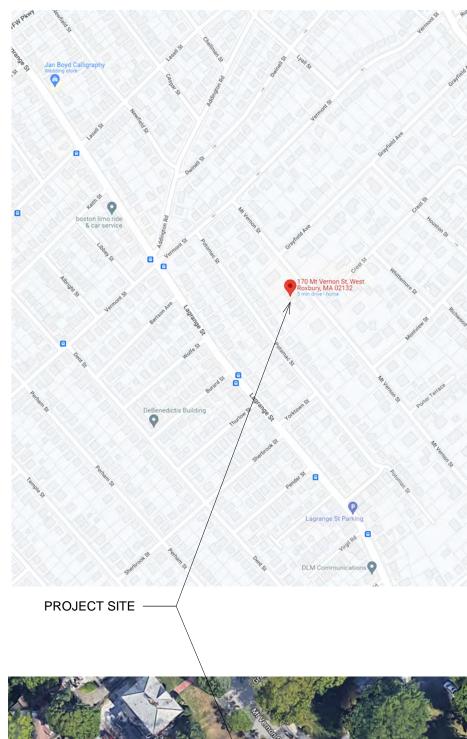
PROJECT DIRECTORY

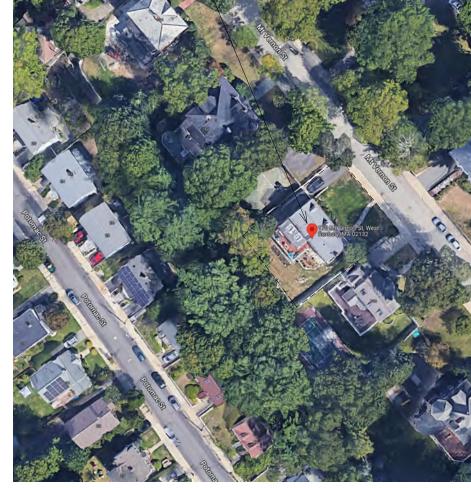
<u>CLIENT:</u> Kathleen Hickey & Alfonso Pruneda Fuentes 170 Mt. Vernon Street West Roxbury, MA 02132 khickey@gmail.com, apruneda@bu.edu (617) 775-1273

ARCHITECT: Derek Rubinoff, Architect 82 Spring St. West Roxbury, MA 02132-4316 CONTACT: Anat Beck-Nachtigal anat@derekrubinoff.com (617) 777-2183

STRUCTURAL: SSB Engineering, LLC 146 Front St. - Suite 301 Scituate, MA 02066 Contact: Tara Strassburg Email: tara@ssbengineering.com Mobile: (917) 733-1822

PROJECT LOCALE



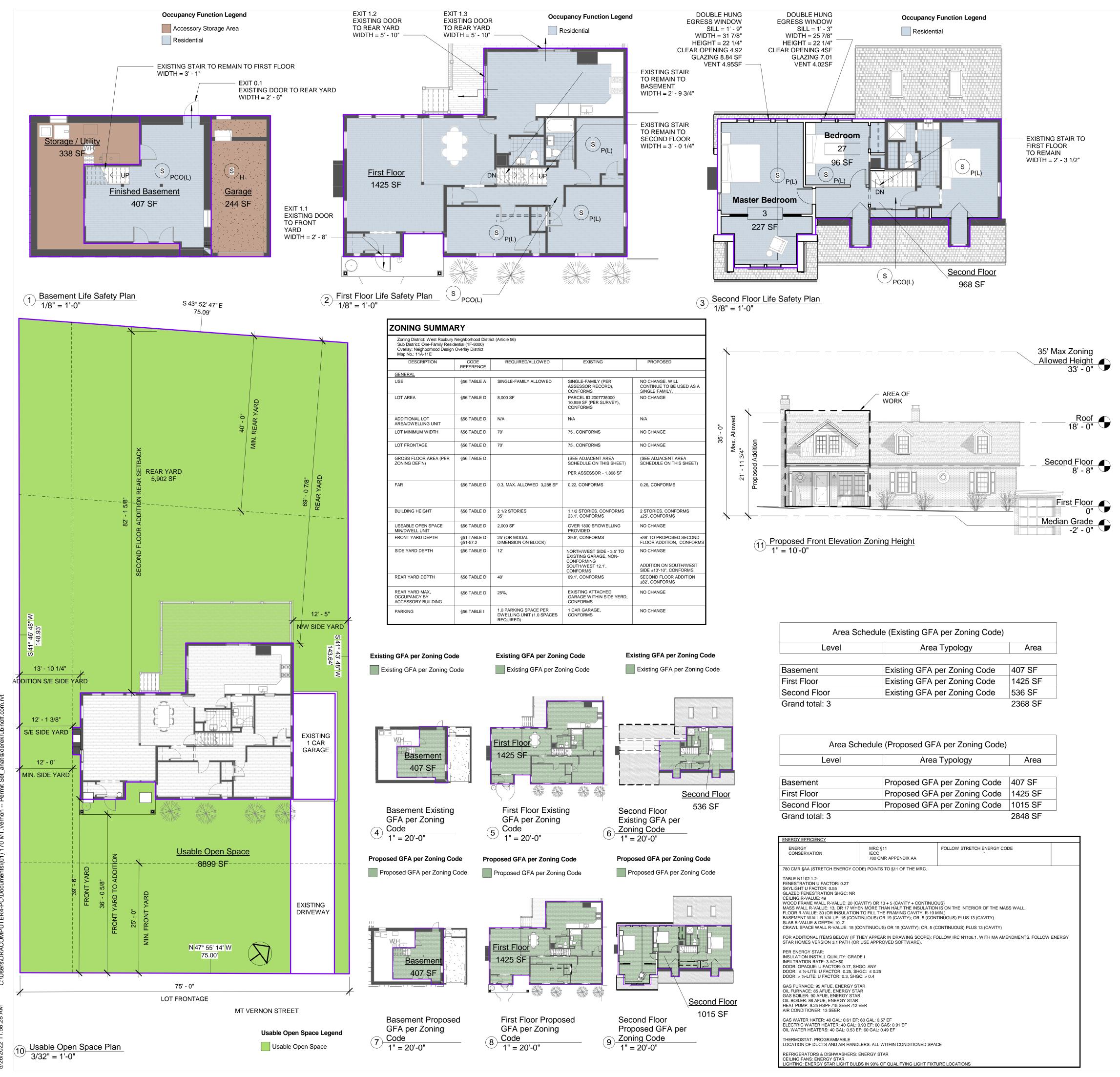


170 Mt. Vernon Addition & Renovation

Renovation and addition to an existing single family detached dwelling. Scope includes minor changes on the first floor, opening the stairwell, and a new front porch. Demo the presently-unfinished attic portion of the second floor and replace with two bedrooms under a new roof with dormers.

	Sheet List		
Sheet Number	Sheet Name	Current Revision	Currei Revisio Date
00-General			
G-0.0	Cover Sheet		
G-0.1	Code and General Notes		
01-Survey			
V-1	Existing Plot Plan		
V-2	Proposed Plot Plan		
02-Structur	al		
S-1	Cover Sheets and Structural Notes		
S-2	Foundation Plan		
S-3	First Floor Framing Plan		
S-4	Second Floor Framing Plan		
S-5 S-6	Attic Framing Plan Roof Framing Plan		
S-0 S-7	Wind Detailing		
S-8	Structural Sections and Details		
03-Archited			
A-1.0 A-1.1	Demo Floor Plans Demo Elevations		
A-2.1	Proposed Floor Plans		
A-2.2	Demo and Proposed Roof Plans		
A-2.3 A-3.1	Proposed RCPs Proposed Elevations		
A-3.2	Proposed Section 1, Hall Interior 3D View		
A-3.3	Proposed Section 2, Details & Schedules		
A-4.1	3D Views		
A-3.3	Interior 3D View Proposed Section 2, Details & Schedules		

$\mathbf{\tilde{A}}$ pal 316 **EEONIE**US Prin 132-2 G YERE Derek 82 Spi 617.5(© Copyright 2022 Derek Rubinoff, Architect <u>CLIENT:</u> Kathleen Hickey & Alfonso Pruneda Fuentes 170 Mt. Vernon Street West Roxbury, MA 02132 khickey@gmail.com, apruneda@bu.edu (617) 775-1273 ARCHITECT: Derek Rubinoff, Architect 82 Spring St. West Roxbury, MA 02132-4316 CONTACT: Anat Beck-Nachtigal anat@derekrubinoff.com (617) 777-2183 STRUCTURAL: SSB Engineering, LLC 146 Front St. - Suite 301 Scituate, MA 02066 Contact: **Tara Strassburg** Email: tara@ssbengineering.com Mobile: (917) 733-1822 ----05/25/2022Permit SetNoByDateDescription 170 Mt. Vernon Addition and Renovation WEST ROXBURY, 170 Mt Vernon St West Roxbury, MA 02132 Cover Sheet Checker Checked By: 2172 Job No: G-0.0



icle 56)		
REQUIRED/ALLOWED	EXISTING	PROPOSED
IGLE-FAMILY ALLOWED	SINGLE-FAMILY (PER ASSESSOR RECORD), CONFORMS	NO CHANGE. WILL CONTINUE TO BE USED AS A SINGLE FAMILY.
00 SF	PARCEL ID 2007735000 10,959 SF (PER SURVEY), CONFORMS	NO CHANGE
ł	N/A	N/A
	75', CONFORMS	NO CHANGE
	75', CONFORMS	NO CHANGE
	(SEE ADJACENT AREA SCHEDULE ON THIS SHEET)	(SEE ADJACENT AREA SCHEDULE ON THIS SHEET)
	PER ASSESSOR - 1,868 SF	
, MAX. ALLOWED 3,288 SF	0.22, CONFORMS	0.26, CONFORMS
/2 STORIES	1 1/2 STORIES, CONFORMS 23.1', CONFORMS	2 STORIES, CONFORMS ±25', CONFORMS
00 SF	OVER 1800 SF/DWELLING PROVIDED	NO CHANGE
(OR MODAL //ENSION ON BLOCK)	39.5', CONFORMS	±36' TO PROPOSED SECOND FLOOR ADDITION, CONFORMS
	NORTH/WEST SIDE - 3.5' TO EXISTING GARAGE, NON- CONFORMING	
	SOUTH/WEST 12.1', CONFORMS	ADDITION ON SOUTH/WEST SIDE ±13'-10", CONFORMS
	69.1', CONFORMS	SECOND FLOOR ADDITION ±82', CONFORMS
%,	EXISTING ATTACHED GARAGE WITHIN SIDE YERD, CONFORMS	NO CHANGE
PARKING SPACE PER VELLING UNIT (1.0 SPACES QUIRED)	1 CAR GARAGE, CONFORMS	NO CHANGE

Area Schedule (Existing GFA per Zoning Code)					
Level	Area Typology	Area			
Basement	Existing GFA per Zoning Code	407 SF			
First Floor	Existing GFA per Zoning Code	1425 SF			
Second Floor	Existing GFA per Zoning Code	536 SF			
Grand total: 3		2368 SF			

Area Schedule (Proposed GFA per Zoning Code)						
Area Typology	Area					
Proposed GFA per Zoning Code	407 SF					
Proposed GFA per Zoning Code	1425 SF					
Proposed GFA per Zoning Code	1015 SF					
	2848 SF					
	Area Typology Proposed GFA per Zoning Code Proposed GFA per Zoning Code					

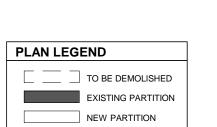
ENERGY	MRC §11	FOLLOW STRETCH ENERGY CODE	
CONSERVATION	IECC 780 CMR APPENDIX AA		
780 CMR §AA (STRETCH ENERGY COL	DE) POINTS TO §11 OF THE MRC.		
FLOOR R-VALUE: 30 (OR INSULATION BASEMENT WALL R-VALUE: 15 (CONT SLAB R-VALUE & DEPTH: 10, 2' CRAWL SPACE WALL R-VALUE: 15 (CO FOR ADDITIONAL ITEMS BELOW (IF TH	N MORE THAN HALF THE INSULATION TO FILL THE FRAMING CAVITY, R-19 N INUOUS) OR 19 (CAVITY); OR, 5 (CON DNTINUOUS) OR 19 (CAVITY); OR, 5 (C HEY APPEAR IN DRAWING SCOPE): FC	I IS ON THE INTERIOR OF THE MASS WALL. IIN.) IINUOUS) PLUS 13 (CAVITY)	NERGY
STAR HOMES VERSION 3.1 PATH (OR	USE APPROVED SOFTWARE).		
ER ENERGY STAR: ISULATION INSTALL QUALITY: GRADI IFILTRATION RATE: 3 ACH50 OOR: OPAQUE: U FACTOR: 0.17, SHC OOR: ≤ ½-LITE: U FACTOR: 0.25, SHC OOR: > ½-LITE: U FACTOR: 0.3, SHGC	GC: ANY GC: ≤ 0.25		
AS FURNACE: 95 AFUE, ENERGY STA DIL FURNACE: 85 AFUE, ENERGY STA AS BOILER: 90 AFUE, ENERGY STAR DIL BOILER: 86 AFUE, ENERGY STAR IEAT PUMP: 9.25 HSPF /15 SEER /12 E IR CONDITIONER: 13 SEER	R		
GAS WATER HATER: 40 GAL: 0.61 EF; ELECTRIC WATER HEATER: 40 GAL: 0. DIL WATER HEATERS: 40 GAL: 0.53 EF	.93 EF; 60 GAS: 0.91 EF		
THERMOSTAT: PROGRAMMABLE LOCATION OF DUCTS AND AIR HANDL	ERS: ALL WITHIN CONDITIONED SPAC	Æ	

FIRE ALARM LEGEND

s)

'′ <u>s</u>______

GC TO PROVIDE THE FOLLOWING WHERE MISSING:



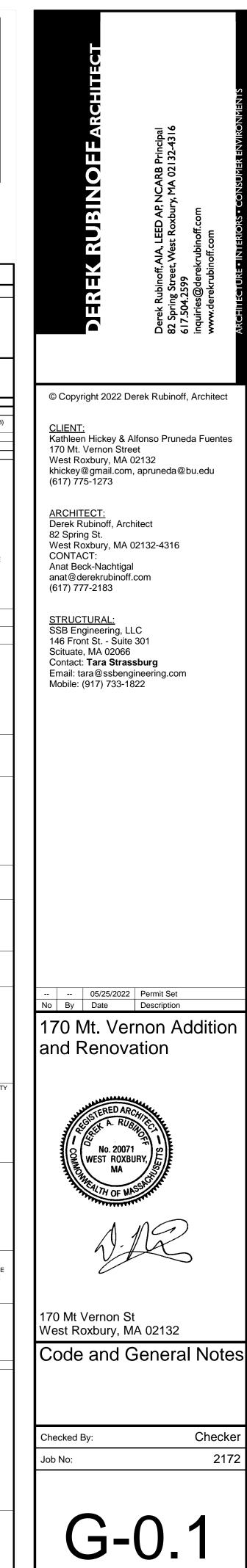
DETECTOR/CARBON MONOXIDE PCO(L) DETECTOR WITH BATTERY BACK-UP S P(L) PHOTOELECTRIC SMOKE DETECTOR WITH BATTERY BACK-UP

COMBO PHOTOELECTRIC SMOKE

HEAT ALARM

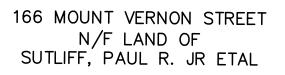
NOTE: G.C. TO VERIFY SMOKE & CO DETECTORS ARE INSTALLED AND OPERATING PER CODE.

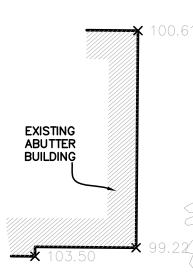
2. PLUMBING M/	n Ed. of the MA Residential Code A State Fuel Gas and Plumbing C ernational Mechanical Code	for 1- and 2-family Dwellings (MA State Building Code code (248 CMR)	(780 CMR), 2015 IRC)	
4. ELECTRICAL 20 5. FIRE PREVENTION M/ 6. ENERGY M/	05 National Electrical Code w/200 A Fire Prevention Regulations (52	3) Chapter 13, 2018 IECC (Stretch Energy Code)		
		ng single family detached dwelling. Scope includes mind ed attic portion of the second floor and replace with two		
DESCRIPTION	CODE REFERENCE	REQUIRED/ALLOWED	PROPOSED	
GENERAL USE GROUP	IBC §310	EXISTING: SINGLE-FAMILY DWELLING (R-3)	SINGLE-FAMILY DWELLING (
NUMBER OF STOREYS CONSTRUCTION TYPE		1 1/2	2 TYPE VB	
BUILDING PLANNING & CONS CLIMATE &	STRUCTION MRC TABLE R301.2(4)	GROUND SNOW LOAD	40 PSF	
GEOGRAPHIC DESIGN CRITERIA	MRC TABLE R301.2(1)	(TABLE R301.2 (4)) FLAT ROOF SNOW LOAD WIND DESIGN SPEED (TABLE R301.2 (4)) TOPOGRAPHIC EFFECTS SPECIAL WIND REGIONS WINDBORNE DEBRIS ZONE SEISMIC DESIGN CATEGORY WEATHERING FROST LINE DEPTH TERMITE (TABLE R301.2 (6)) WINTER DESIGN TEMP ICE BARRIER UNDERLAYMENT FLOOD HAZARDS (§322.0) AIR FREEZING INDEX (TABLE R403.3(2))	30 PSF MIN. V _{ut} = 128 MPH NO NO V _{asd} = 100±, THEREFORE: NO NO SEVERE 48" MODERATE TO HEAVY DRY BULB APPLICABLE TO ROOFING, S R905.2.7 MINIMAL; ZONE X APPLICABLE TO SHALLOW FOUNDATIONS; 1500 OR LES	
PROTECTION OF	MRC §R301.2.1.2	MEAN ANNUAL TEMP	51.3°F ± (10.7°C)	
OPENINGS				
WIND EXPOSURE CATEGORY	MRC §R301.2.1.4	EXPOSURE B (URBAN & SUBURBAN)	0 HOURS (FRONT/REAR)	
FIRE-RESISTANT CONSTRUCTION OF EXTERIOR WALLS	MRC TABLE R302.1	EXTERIOR WALLS PROJECTIONS OPENINGS IN WALLS PENETRATIONS CUT OFF ALL CONCEALED DRAFT	0 HOURS (FRONT/KEAR) 0 HOURS (5' MIN SEP DIST) 0 HOUR (≥5') UNLIMITED (< 3') 25% MAX OF WALL (3' MIN) UNLIMITED (5' MIN) COMPLY W/§R302.4 (<3') NONE REQUIRED (3' MIN) FIRE BLOCKING WILL BE	
		OPENINGS (BOTH HORZ. & VERT.) AND FORM AN EFFECTIVE FIRE BARRIER BETWEEN STOREYS AND BETWEEN TOP STOREY AND THE ROOF SPACE.	APPLIED IN ALL NEW PENETRATIONS.	
HABITABLE ROOMS	MRC §R303.1	GLAZING AREA = 8% OF FLOOR AREA. NATURAL VENTILATION OPENABLE AREA = 4% OF FLOOR AREA OR: WHERE ALLOWED BY \$310, MECH	COMPLIES, REVIEW LIFE SAFETY PLANS	
		VENTILATION ARTIFICIAL LIGHTING PER CODE		
DWELLING/GARAGE OPENING/PENETRATION PROTECTION	MRC §R302.5	NO GARAGE OPENINGS TO BDRMS, 20 MIN RATED DOORS OR 1 3/8" MIN. SOLID CORE PROTECTED PENETRATIONS	EXISTING TO REMAIN. NO PROPOSED WORK NEAR GARAGE	
DWELLING/GARAGE FIRE SEPARATION	IRC §R302.6	EXISTING ASSUMED TO COMPLY WITH THE FOLLOWING: FROM THE RESIDENCE & ATTICS: 5/8" TYPE X GWB APPLIED TO GARAGE SIDE FROM HABITABLE ROOMS ABOVE GARAGE: 5/8" TYPE X GWB FROM STRUCTURE SUPPORTING SEPARATING ASSEMBLIES: 5/8" TYPE X GWB		
MIN ROOM DIMENSIONS	MRC §R304	HABITABLE ROOMS: 70 SF MIN. ONE DIM 7' MIN. EX. KITCH. IF <5' HT, DOESN'T COUNT	COMPLIES	
CEILING HEIGHT	MRC §R305.1	HABITABLE SPACE, HALLWAYS, BATHROOMS, TOILET ROOMS, LAUNDRY ROOMS BASEMENTS BASEMENT BEAMS SLOPED CEILINGS	7' MIN.; COMPLIES 6'-8" MIN, 7'-2" EXISTING 6'-4" CLR MIN. 50% OF REQ. AREA HAS 7' CEILING; 5' MIN. FOR REQ. AREA	
EMERGENCY ESCAPE & RESCUE OPENINGS	MRC §R310	BATHROOM FIXTURE CLEARANCES ONE REQUIRED IN BASEMENTS, HABITABLE ATTICS, AND EVERY SLEEPING ROOM (INCLUDING EVERY BASEMENT SLEEPING ROOM); MIN. NET CLEAR OPENING OF 5.7 SF, EXCEPT GRADE FLOOR = 5 SF MIN; AND EXCEPT DOUBLE-HUNG WINDOWS OPENING MIN. 3.3 SF; MIN. NET CLEAR OPENING DIMENSIONS ARE 20" X 24" IN EITHER DIRECTION.	6'-8"; SEE EXCEPTION 2, COMPLIES COMPLIES, REVIEW LIFE SAU PLANS	
MEANS OF EGRESS	MRC §R311.1	ARE 20 X 24 IN ETITLER DIRECTION. 2 REQUIRED FROM EACH DWELLING AT A NORMAL LEVEL OF EXIT. 32" MIN. WIDTH (BETWEEN F.O. DOOR & STOP) FOR PRIMAR DOOR. LANDING REQUIRED BOTH SIDES, 36" MIN. IN DIR. OF TRAVEL. LANDING MUST BE NO LOWER THAN 1 1/2" BELOW T.O. THRESHOLD; IF DOOR SWINGS IN, EXTERIOR LANDING SHALL NOT BE MORE THAN 7 3/4" BELOW T.O. THRESHOLD.		
STAIRWAYS	MRC §R311.7	36" MIN WIDTH HANDRAIL ON ONE SIDE MIN. 6'-8" HEADROOM MIN. RISERS: 8 1/4" MAX TREADS: 9" MIN. LANDINGS 36" MIN. IN DIR. OF TRAVEL.	EXISTING TO REMAIN EXISTING BUILDING STAIRS ASSUMED TO HAVE MET CO AT THE TIME OF ORIGINAL BUILDING CONSTRUCTION.	
GUARDS	MRC §R312	REQUIRED IF 30" ABOVE GRADE MEASURED 36" PROUD. MIN. 36" HIGH (34" AT STAIRS) <4" SPHERE OPENING MAX.(4 3/8" @ STAIRS) <6" SPHERE MAX NEAR STAIR TREAD		
SPRINKLERED SMOKE ALARMS	MRC §R313.2 MRC §R314	NOT REQUIRED PHOTOELECTRIC LISTED PER UL 217 OR UL 268; TO BE INSTALLED PER MRC AND NFPA 72	NO COMPLIES	
		MONITOR WITH SUPERVISING STATION (PER NFPA 72) UNLESS ALARMS MEET §R314.4 INSTALL: -IN EACH SLEEPING ROOM -OUTSIDE EACH SEPARATE SLEEPING AREA NEAR BEDROOMS -ON EACH STOREY -NEAR THE BASE OF ALL STAIRS -FOR EACH 1200 SF AREA OR PART THEREOF INTERCONNECT ALARMS (ONE TRIGGERS ALL).		
CO ALARMS	MRC §315.1 MGL C148, §26F1/2 527 CMR §13.7.6; TABLE 13.7D 248 CMR NFPA 720	INSTALL UNITS LISTED PER UL 2034 OR UL 2075: -ONE PER EACH LEVEL WITH SLEEPING AREA -ONE PER HABITABLE PORTIONS OF BASEMENTS & ATTICS -OUTSIDE SLEEPING AREA BUT WITHIN 10' FROM BR DOOR	COMPLIES	



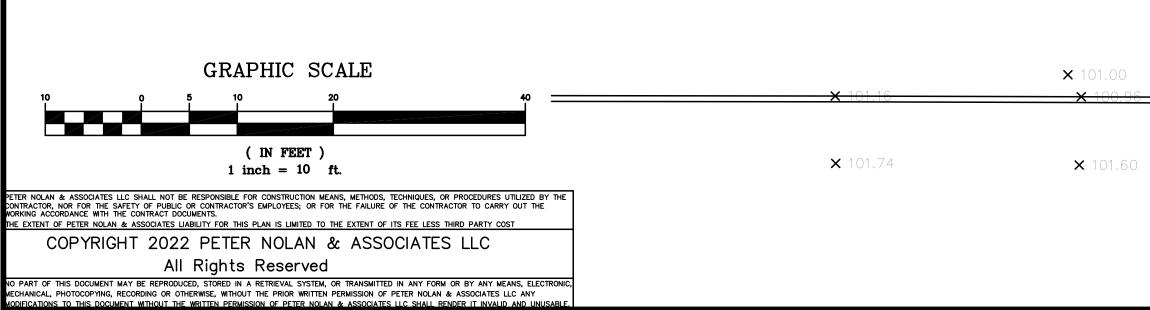
EXIST	ING LEGEND
22	SEWER LINE
S	SEWER MANHOLE
w	WATER LINE
G	GAS LINE
ل	UTILITY POLE
GV	GAS VALVE
—— E ——	OVERHEAD ELECTRIC SERVICE
\bowtie	WATER VALVE
	CATCH BASIN
	FENCE
205	CONTOUR LINE (MJR)
195	CONTOUR LINE (MNR)
×	SPOT GRADE
D	DRAIN MANHOLE
ж.	HYDRANT
k.	TREE

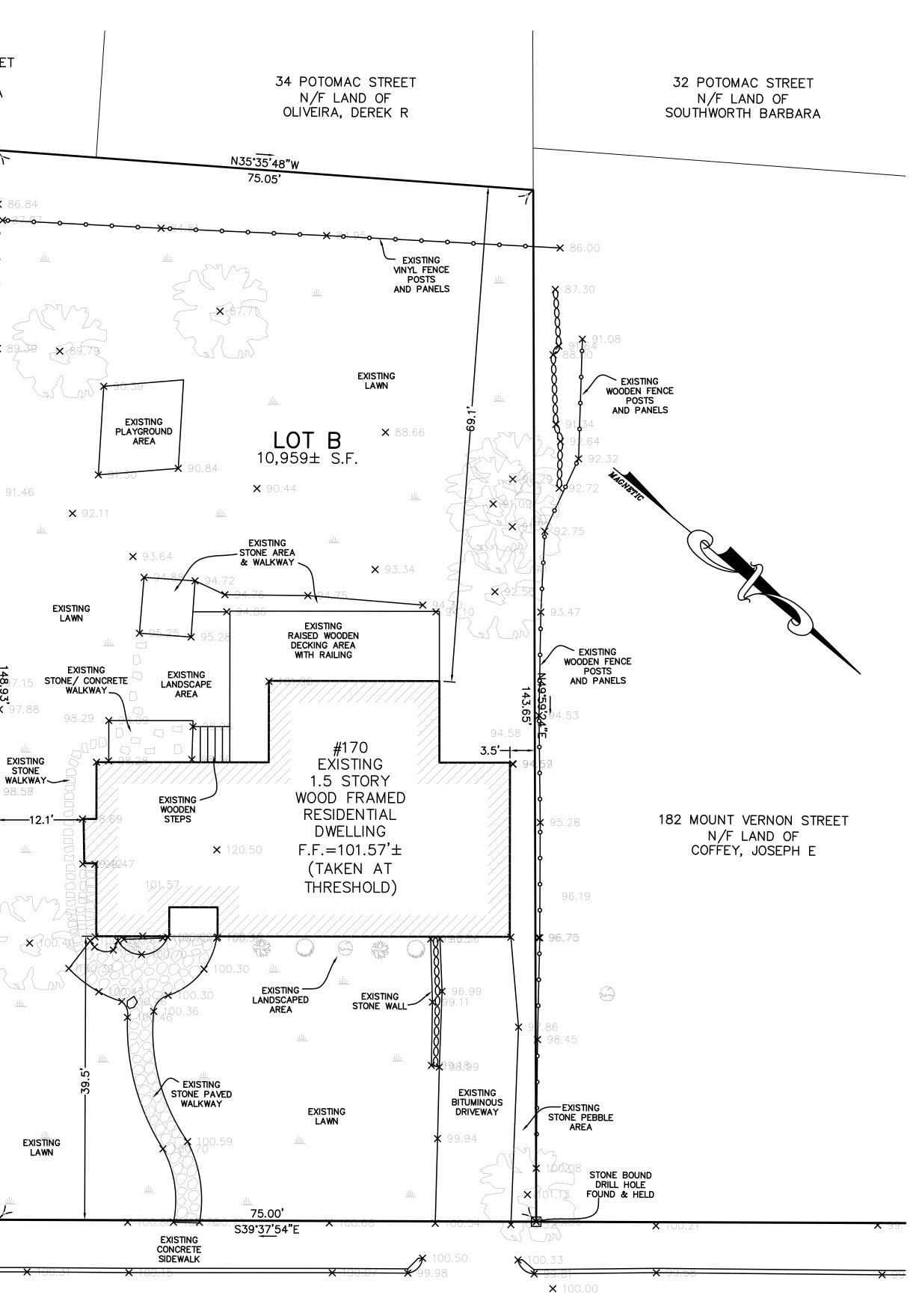
38 POTOMAC STREET N/F LAND OF OSNOWITZ, DEBRA





EXISTING VINYL FENCE POSTS AND PANELS ~





MOUNT VERNON STREET (PUBLIC WAY-VARIABLE WIDTH)

X 101.52

X 101.11

X 100.89

NOTES:

1. INFORMATION SHOWN ON THIS PLAN IS THE RESULT OF A FIELD SURVEY PERFORMED BY PETER NOLAN & ASSOCIATES LLC AS OF 12-08-2021.

2. DEED REFERENCE: BOOK 59814 PAGE 337, PLAN REFERENCE: BOOK 5955 PAGE 217, SUFFOLK COUNTY REGISTRY OF DEEDS.

3. THIS PLAN IS NOT INTENDED TO BE RECORDED.

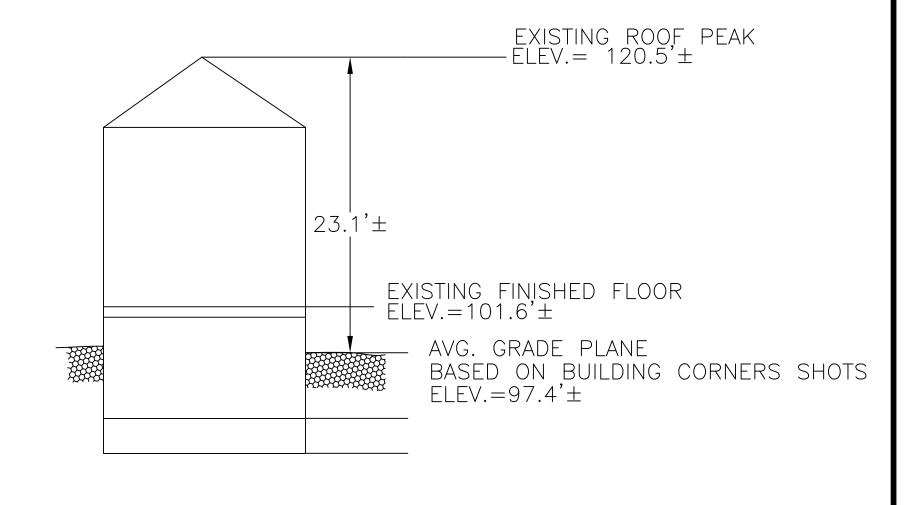
4. I CERTIFY THAT THE DWELLING SHOWN IS NOT LOCATED WITHIN A SPECIAL FLOOD HAZARD ZONE. IT IS LOCATED IN ZONE X, ON FLOOD HAZARD BOUNDARY MAP NUMBER 25025C0066G, PANEL NUMBER 0066G, COMMUNITY NUMBER: 250286, DATED SEPTEMBER 25, 2009.

5. THIS PLAN DOES NOT SHOW ANY UNRECORDED OR UNWRITTEN EASEMENTS WHICH MAY EXIST. A REASONABLE AND DILIGENT ATTEMPT HAS BEEN MADE TO OBSERVE ANY APPARENT USES OF THE LAND; HOWEVER THIS NOT CONSTITUTE A GUARANTEE THAN NO SUCH EASEMENTS EXIST.

6. FIRST FLOOR ELEVATIONS ARE TAKEN AT THRESHOLD.

7. NO RESPONSIBILITY IS TAKEN FOR ZONING TABLE AS PETER NOLAN & ASSOCIATES LLC ARE NOT ZONING EXPERTS. TABLE IS TAKEN FROM TABLE PROVIDED BY LOCAL ZONING ORDINANCE. CLIENT AND/OR ARCHITECT TO VERIFY THE ACCURACY OF ZONING ANÁLYSIS.

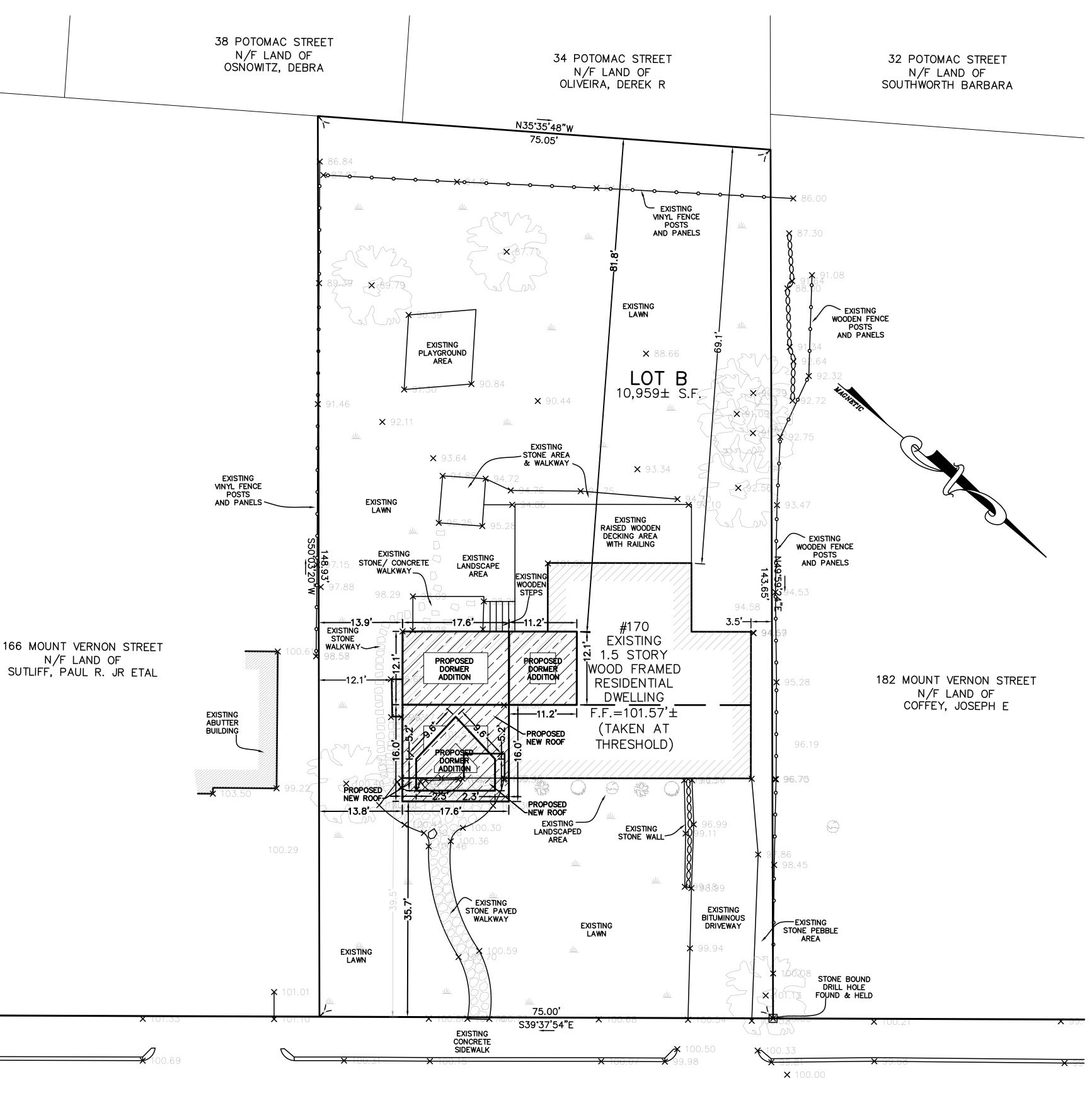
8. ZONING DISTRICT = 1F-8000 WEST ROXBURY NEIGHBORHOOD.

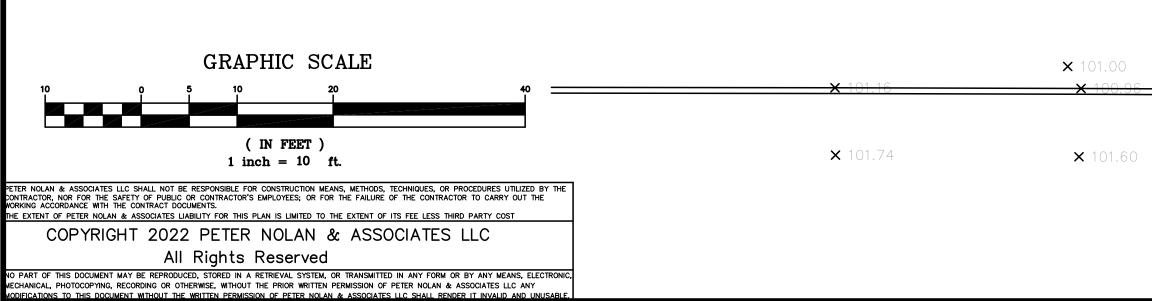




SCALE				
1"=10'				NTH OF MASS
DATE				PETER O
01/12/2022	REV	DATE	REVISION BY	
SHEET 1		17	NO. 49185	
PLAN NO. 1 OF 1			ABTONAL LAND SPEC	
CLIENT:			PLOT PLAN	SHEET NO.
DRAWN BY			OF LAND	
CHKD BY PJN			ETER NOLAN & ASSOCIATES LLC ND SURVEYORS/CIVIL ENGINEERING CONSULTANTS 697 CAMBRIDGE STREET, SUITE 103 BRIGHTON MA 02135	
APPD BY PJN		PHONE EM/	: 857 891 7478/617 782 1533 FAX: 617 202 5691 AIL: pnolan@pnasurveyors.com	

EXIST	TING LEGEND
22	SEWER LINE
S	SEWER MANHOLE
w	WATER LINE
G	GAS LINE
0	UTILITY POLE
GV	GAS VALVE
—— E ——	OVERHEAD ELECTRIC SERVICE
wv ⊠	WATER VALVE
	CATCH BASIN
	FENCE
205	CONTOUR LINE (MJR)
195	CONTOUR LINE (MNR)
Х	SPOT GRADE
D	DRAIN MANHOLE
ķ	HYDRANT
kang land	TREE





MOUNT VERNON STREET

X 101.52

X 101.11

X 100.89

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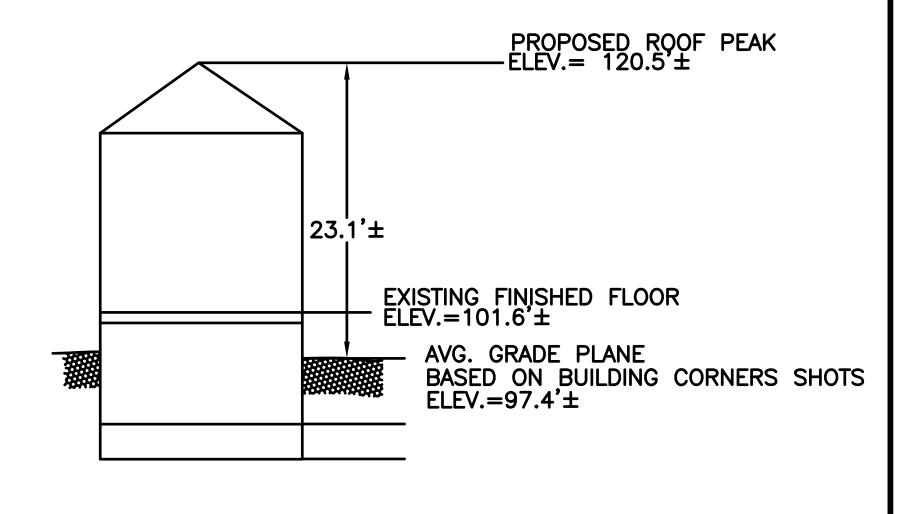
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8. ZONING DISTRICT = 1F-8000 WEST ROXBURY NEIGHBORHOOD.



PROPOSED PROFILE NOT TO SCALE

SCALE 1"=10'					JOF MA
DATE					PETER 6
05/25/2022	REV	DATE	REVISION	BY	
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PLAN NO. 1 OF 1			BOSTON, Massachusetts		AND ALL LAD STA
CLIENT:		F	PROPOSED PLOT PLAN		SHEET NO.
DRAWN BY			OF LAND		
CHKD BY PJN APPD BY PJN		PHONE	ETER NOLAN & ASSOCIATES LL ND SURVEYORS/CIVIL ENGINEERING CONSULTANT 697 CAMBRIDGE STREET, SUITE 103 BRIGHTON MA 02135 2: 857 891 7478/617 782 1533 FAX: 617 202 56 AIL: pholan@phasurveyors.cor	<u>-s</u> 91	V-2

STRUCTURAL DESIGN DRAWINGS

DESIGNER .

DEREK RUBINOFF 82 Spring Street, West Roxbury MA 02132

PROJECT SPECIFIC DESIGN CRITERIA

- 1. DESIGN CODES AND CRITERIA: THE MINIMUM STRUCTURAL DESIGN SHALL BE IN ACCORDANCE WITH THE MASSACHUSETTS STATE BUILDING CODE, IBC 2015, ASCE 7-10, AND ANSI/AWS D1.1 STRUCTURAL WELDING CODE - STEEL
- 2. IN ADDITION TO THE BUILDING DEAD LOADS, THE STRUCTURE IS DESIGNED FOR THE FOLLOWING LOADS:

CITY/TOWN OF DESIGN CRITERIA:	BOSTON, MA
<u>SNOW LOAD</u> : GROUND SNOW LOAD (p _g)	40 PSF
WIND LOAD: BASIC WIND SPEED (Vult)	128 MPH

- 3. LIVE LOAD REDUCTION SHALL BE IN ACCORDANCE WITH ASCE 7-10.
- SOIL BEARING CAPACITY: FOUNDATIONS HAVE BEEN DESIGNED TO BEAR ON UNDISTURBED SOIL HAVING AN ASSUMED ALLOWABLE BEARING CAPACITY OF 1 TON PER SQUARE FOOT. SOIL BEARING CAPACITY TO BE DETERMINED B' SOIL TESTS PRIOR TO CONSTRUCTION. IF BEARING MATERIALS WITH LOWER BEARING CAPACITY ARE ENCOUNTERED, THE UNDERLYING UNSUITABLE MATERIAL SHALL BE REMOVED AND REPLACED WITH SUITABLE MATERIAL TO BE APPROVED BY THE ENGINEER.

TIMBER FRAMING:

- 1. FOR ROUGH WINDOW & DOOR (BOTH INTERIOR & EXTERIOR) OPENING UP TO 3-FEET USE 2x6 HEADER BEAM; FOR 3- TO 6-FOOT OPENINGS USE 2x8 HEADER BEAMS: AND, FROM 6- TO 8-FOOT OPENINGS USE 2x10 HEADER BEAMS: AND DOUBLES FOR 2x4 WALLS & TRIPLES FOR 2x6 WALLS, EXCEPT AS NOTED OTHERWISE ON THE PLANS OR SPECIFICATIONS. IF LVLs ARE SPECIFIED ON THE PLANS, PROVIDE SOLID 4x4 POST SUPPORTS FOR DBL HEADERS & SOLID 4x6 OR 6x6 DFL #2 POSTS FOR TPL HEADERS OR AS OTHERWISE SPECIFIED ON THE PLAN. CONTINUE ALL STRUCTURAL POSTS DOWN TO FOUNDATION OR BEAMS BELOW (SOLID BLOCK TO DROP BEAMS) ...
- 2. ALL FRAMING LUMBER SHALL BE HEM-FIR GRADE #2 OR SPF (SPRUCE PINE FIR) GRADE #1 / #2 OR APPROVED EQUAL (UNLESS OTHERWISE SPECIFIED), AND SHALL MEET THE REQUIREMENTS OF THE AMERICAN FOREST AND PAPER ASSOCIATION. MINIMUM TIMBER FRAMING MATERIAL PROPERTIES:

ALLOWABLE BENDING STRESS (F _b):	875 PSI MIN.
ALLOWABLE COMPRESSION STRESS (Fc):	1,150 PSI MIN.
MODULUS OF ELASTICITY (E):	1,400,000 PSI MIN.

OTHER FRAMING MATERIAL FOR INTERIOR NON-LOAD BEARING STUDS MAY BE SUBSTITUTED ONLY UPON APPROVAL OF THE ENGINEER.

- 3. ALL EXTERIOR FRAMING SHALL BE PRESSURE TREATED (CCA TREATED) SOUTHERN YELLOW PINE GRADE #2.
- 4. BUILT-UP BEAMS SHALL BE SPIKED AS FOLLOWS:
 - 3-PLY MAXIMUM, UNLESS OTHERWISE NOTED USING LVLs AND CONVENTIONAL FRAMING LUMBER SHALL BE FULLY SPIKED TOGETHER WITH 2-10D NAILS AT 12" O.C.
 - 4-PLY BUILT-UP FRAMING AND LVLs ARE TO BE SPIKED TOGETHER WITH THREE (3) SIMPSON SDS $\frac{1}{4}$ "x6" SCREWS @ 12" O.C. OR AS OTHERWISE NOTED ON THE DRAWINGS; OR AS RECOMMENDED BY THE MANUFACTURER.
- 4. USE FULLY NAILED METAL CONNECTORS (TECO, SIMPSON; OR APPROVED EQUAL): JOIST OR BEAM HANGERS WHEN JOISTS OR BEAMS FRAME INTO ANOTHER JOIST OR BEAM MEMBER. PROVIDE METAL POST CAPS AND BASES FOR ALL POSTS.
- 5. PROJECT EXTERIOR WALL FRAMING TO BE 2x6 @16" O.C.

CAST IN PLACE CONCRETE:

- 1. CONCRETE WORK SHALL CONFORM TO THE LATEST AMERICAN CONCRETE INSTITUTE FOR "BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE" AND "SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS".
- CONCRETE: MINIMUM 28 DAY COMPRESSIVE STRENGTHS F'c (28-DAYS)

FOUNDATIONS	
SLAB-ON-GRADE	

CONCRETE SHALL HAVE A SLUMP OF NO MORE THAN 4 INCHES AND AIR
ENTRAPMENT OF 4-6%. THE USE OF CALCIUM CHLORIDE IS NOT PERMITTED.
PROVIDE PROPER CONCRETE PROTECTION OR HEAT IN COLD WEATHER AND
MAINTAIN PROPER CURING PROCEDURES IN ACCORDANCE WITH THE A.C.I.

3,000 psi

3,500 psi

3. BACKFILL UNDER ANY PORTION OF THE FOUNDATIONS SHALL BE COMPACTED IN 6 INCH LIFTS OF GRAVEL COMPACTED TO 90-95% OF MODIFIED PROCTOR DENSITY, AS APPROVED BY THE ENGINEER.

- 4. DO NOT BACKFILL EXTERIOR WALLS ANY HIGHER THAN 3 FEET ABOVE THE TOP OF FOOTING UNTIL PERMANENT STRUCTURAL SUPPORTS (FRAMED FLOORS AND SLABS) ARE IN PLACE. BRACE ALL WALLS AND GRADE BEAMS DURING BACKFILLING, IF NECESSARY.
- 5. NO FOUNDATION SHALL BE PLACED IN WATER OR ON FROZEN GROUND.
- 6. FOOTINGS SHALL BE PROTECTED AGAINST FROST UNTIL PROJECT IS COMPLETED.
- 7. NOTIFY BUILDING DEPARTMENT FOR INSPECTION AT LEAST 24 HOURS PRIOR TO SCHEDULED PLACEMENT OF CONCRETE.
- 8. PLACEMENT OF CONCRETE POURS SHOULD HAVE A VERTICAL 2"x4" KEY WITH CONTINUOUS REINFORCEMENT (40 BAR DIAMETER MIN.) THROUGH THE CONSTRUCTION JOINT.
- 9. DAMP PROOF ALL FOUNDATION WALLS BELOW GRADE, OTHER THAN FROST WALLS.

CAST IN PLACE CONCRETE REINFORCING:

- 1. REINFORCING BARS SHALL CONFORM TO ASTM A615 OR A706 GRADE 60
- 2. REINFORCE ALL SLAB AS FOLLOWS UNLESS OTHERWISE NOTED, FURNISH WWF IN FLAT SHEETS:
- SLABS ON GROUND:
- THE FOLLOWING MINIMUM CLEAR CONCRETE COVER SHALL BE PROVIDED UNLESS NOTED OTHERWISE ON THE DRAWINGS:
- 4. UNLESS NOTED OTHERWISE, BARS SHALL BE CONTINUOUS AND SHALL RUN CONTINUOUSLY AROUND CORNERS. BARS SHALL HAVE STANDARD HOOKS AT DISCONTINUOUS ENDS
- 5. SPLICES SHALL GENERALLY OCCUR AT MID-SPAN FOR TOP AND MIDDLE BARS AND AT SUPPORT FOR BOTTOM BARS AND SHALL BE STAGGERED. PROVIDE CLASS B SPLICES FOR ALL CONTINUOUS REINFORCEMENT, UNLESS OTHERWISE NOTED.
- 6. ALL REINFORCING SHALL BE DETAILED IN ACCORDANCE WITH ACI 315 MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES. PROVIDE BAR SUPPORTS, SPACERS, AND ACCESSORIES RECOMMENDED IN THE ACI DETAILING MANUAL, PUBLICATION SP-66. ALL REINFORCEMENT DETAILING, LAP SPLICES, AND EMBEDMENTS SHALL CONFORM TO THIS MANUAL. ALL ACCESSORIES, SUCH AS SLAB BOLSTERS AND BEAM AND SLAB CHAIRS IN CONTACT WITH EXPOSED SURFACES SHALL BE PLASTIC-COATED.
- 7. SET AND TIE ALL REINFORCEMENT BEFORE PLACING CONCRETE. SETTING DOWELS AND REINFORCEMENT INTO WET CONCRETE IS PROHIBITED.
- 8. MINIMUM ANCHORAGE SPLICE REQUIREMENTS FOR REINFORCING BARS, AND TEMPERATURE REINFORCEMENT IN ALL CONCRETE SLABS SHALL BE ACCORDING TO ACI 318, UNLESS OTHERWISE SHOWN ON DRAWINGS.
- 9. NO CONCRETE SHALL BE CAST BEFORE REVIEW AND APPROVAL OF THE REINFORCING AND EMBEDDED ITEMS HAVE BEEN OBTAINED FROM THE ENGINEER.
- 10. ANY ADDITIONAL DRILLING OR CORING SHALL NOT DAMAGE REINFORCING BARS.
- 11. SET ANCHOR BOLTS AND EMBEDDED PLATES REQUIRED FOR CONNECTION OF WORK BY OTHERS.

COORDINATION AND CONSTRUCTION:

- 1. FIELD VERIFY EXISTING DIMENSIONS AND ELEVATIONS WHICH AFFECT FABRICATION PRIOR TO SUBMITTAL OF SHOP DRAWINGS AND FABRICATION.
- 2. REFER TO ARCHITECTURAL AND MECHANICAL DRAWINGS FOR ADDITIONAL EMBEDDED ITEMS, SLEEVES, FLOOR PITCHES, FILLS, AND DEPRESSIONS.
- 3. STRUCTURAL FRAMING PLANS ARE TYPICALLY DRAWN AS REFLECTED PLANS SHOWING BEAMS, WALLS, AND COLUMNS ON THE UNDERSIDE OF THE LEVEL SHOWN.
- 4. BRACE ENTIRE STRUCTURE AS REQUIRED TO MAINTAIN STABILITY UNTIL COMPLETE AND FUNCTIONING AS THE DESIGNED UNIT.
- 5. DO NOT BACKFILL FOUNDATION WALLS SPANNING BETWEEN BASEMENT SLABS AND STRUCTURAL FLOORS UNTIL SUPPORTING SLABS ARE IN PLACE.
- 6. VERIFY EXACT SIZE AND LOCATION OF ALL WALL, FLOOR, AND ROOF OPENINGS PRIOR TO SUBMISSION OF SHOP DRAWINGS. SHOW ALL OPENINGS ON SHOP DRAWINGS.

PROJECT / CLIENT: **KATIE & ALFONSO RESIDENCE** 170 Mount Vernon Street, West Roxbury MA 02132

- 6x6-W1.4xW1.4 (21#) WWF
- CONCRETE CAST AGAINST EARTH, ALL BAR SIZES
- CONCRETE EXPOSED TO EARTH OR WEATHER, ALL BAR SIZES

- 8. IN ACCORDANCE WITH GENERALLY ACCEPTED CONSTRUCTION PRACTICES CONTRACTOR WILL BE SOLELY AND COMPLETELY RESPONSIBLE FOR CONDITIONS OF THE JOB SITE, INCLUDING SAFETY OF ALL PERSONS AND PROPERTY DURING PERFORMANCE OF THE WORK. THIS REQUIREMENT WILL APPLY CONTINUOUSLY AND NOT BE LIMITED TO WORKING HOURS.
- 9. THE DUTY OF THE ENGINEER TO CONDUCT CONSTRUCTION REVIEW OF CONTRACTORS PERFORMANCE IS NOT INTENDED TO INCLUDE REVIEW OF ADEQUACY OF CONTRACTORS SAFETY MEASURES IN, ON, OR NEAR THE CONSTRUCTION SITE.

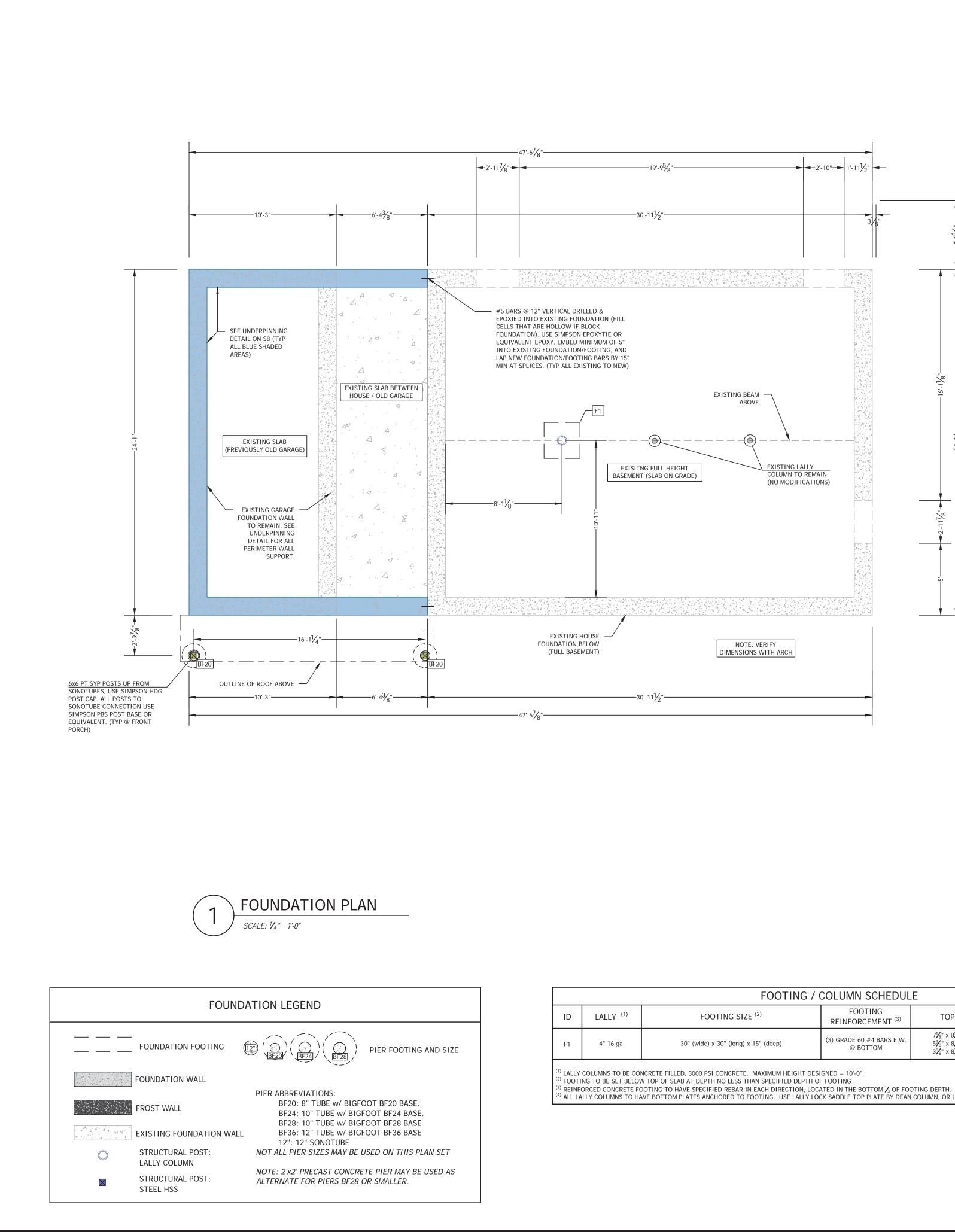




tara ym trasbury RA LYNN STRASSBURG, P

	DRAWING LIST
S1	COVER SHEET AND STRUCTURAL NOTES
S2	FOUNDATION PLAN
S3	FIRST FLOOR FRAMING PLAN
S4	SECOND FLOOR FRAMING PLAN
S5	ATTIC FRAMING PLAN
S6	ROOF FRAMING PLAN
S7	WIND DETAILING
S8	STRUCTURAL SECTIONS AND DETAILS

COMMENTS
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PROJECT: KATIE & ALFONSO RESIDENCE 170 MOUNT VERNON STREET, WEST ROXBURY MA 02132
SHEET TITLE: COVER SHEET and STRUCTURAL NOTES
DRAWN BY: D. Guerrero CHECKED BY: W. Green SCALE: NO SCALE DATE: May 03, 2022
S1 OF S8



FOOTING	/ COLUMN	SCHEDULE

(1)	FOOTING SIZE ⁽²⁾	FOOTING REINFORCEMENT ⁽³⁾	TOP PLATE	BOTTOM PLATE
l.	30" (wide) x 30" (long) x 15" (deep)	(3) GRADE 60 #4 BARS E.W. @ BOTTOM	7½" x 8¼" x ¼" [4 ply] 5¾" x 8¼" x ¼" [3 ply] 3½" x 8¼" x ¼" [2 ply]	5¾" x 5¾" x ¼"

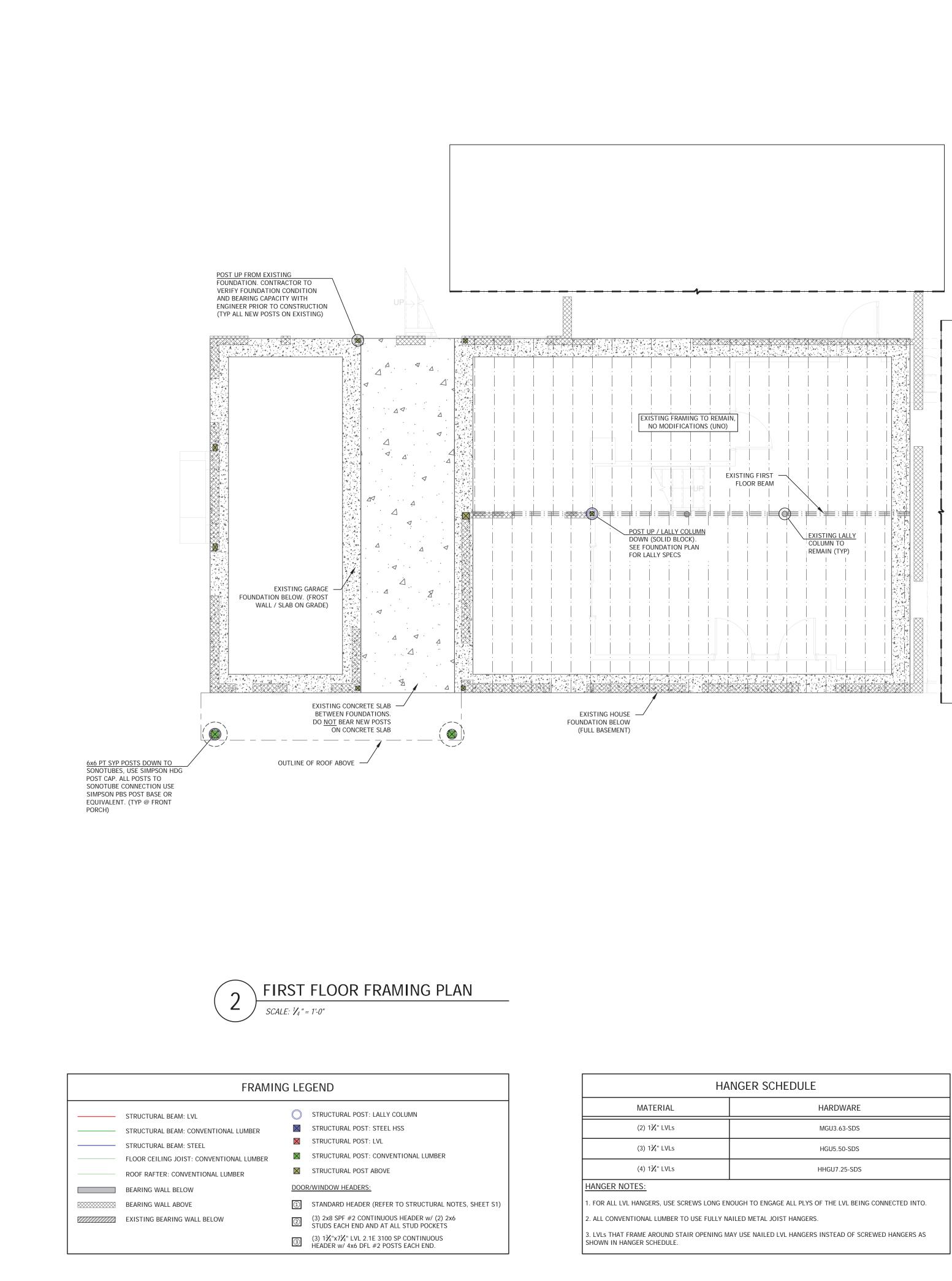
⁴⁾ ALL LALLY COLUMNS TO HAVE BOTTOM PLATES ANCHORED TO FOOTING. USE LALLY LOCK SADDLE TOP PLATE BY DEAN COLUMN, OR USE TOP PLATES SPECIFIED IN SCHEDULE ABOVE.

DRAWING NOTES:

- 1. FOUNDATION DESIGN IS BASED UPON AN ALLOWABLE SOIL BEARING PRESSURE OF 2000 PSF. SOIL BEARING MATERIAL CAPACITY TO BE DETERMINED BY SOIL TESTS PRIOR TO CONSTRUCTION. IF BEARING MATERIALS WITH A LOWER BEARING CAPACITY THAN 1 TON PER SQUARE FOOT ARE ENCOUNTERED AT THE SPECIFIED ELEVATIONS, THE UNDERLYING MATERIAL SHALL BE REMOVED AND REPLACED WITH SUITABLE MATERIAL TO BE APPROVED BY THE ENGINEER.
- 2. ALL BACKFILL UNDER STRUCTURAL SLABS, MATS, AND OTHER FOUNDATION ELEMENTS SHALL BE COMPACTED IN MAX 6" LIFTS TO 95 PERCENT OF MAXIMUM DRY DENSITY PER ASTM D1557, UNLESS OTHERWISE INDICATED OR SPECIFIED. FOUNDATION ELEMENTS SHALL REST ONLY ON SUITABLE UNDISTURBED OR COMPACTED STRUCTURAL FILL. STRUCTURAL FILL GRADATION SHALL BE NO LARGER THAN 1", BETWEEN 10% AND 60% PASSING THE NO. 20 SIEVE AND NO MORE THAN 5% PASSING THE NO. 200 SIEVE.
- PROVIDE 6" MINIMUM CRUSHED STONE UNDER CONCRETE SLAB. GRADATION 3. FOR CRUSHED STONE SHALL BE NO LARGER THAN 1", BETWEEN 10% AND AND 50% PASSING $\frac{1}{2}$ " AND NO MORE THAN 5% PASSING THE NO. 4 SIEVE. CRUSHED STONE REQUIRES COMPACTION BY MAKING AT LEAST THREE PASSES PER 6-INCH THICK LIFT (OR THINNER) BY A VIBRATORY PLATE COMPACTOR OR VIBRATORY ROLLER WITH MINIMUM STATIC WEIGHT OF 200 POUNDS. NO COMPACTION TESTING IS NECESSARY FOR THE CRUSHED STONE FILLS. CRUSHED STONE FILLS THICKER THAN 12 INCHES SHOULD BE PLACED IN ONE-FOOT LIFTS AND SHOULD BE MONITORED BY A TECHNICIAN OR GEOTECHNICAL ENGINEER.
- 4. PROVIDE SHEETING, BRACING AND UNDERPINNING TO PROTECT ADJACENT UTILITY STRUCTURES, AS REQUIRED.

TRUCTURAL ENGINEER:
VGINEER STAMP: TARA LYNN STRASSBURG CIVIL No. 55751 CIVIL No. 55751 CIVIL CIVIL NO. 55751 CIVIL CIVIL NO. 55751 CIVIL
COMMENTS
NO. DATE
₩ 20-0-020
PROJECT: KATIE & ALFONSO RESIDENCE 170 MOUNT VERNON STREET, WEST ROXBURY MA 02132
SHEET TITLE: FOUNDATION PLAN
DRAWN BY: D. Guerrero CHECKED BY: W. Green SCALE: 1/4" = 1'-0" DATE: May 03, 2022
S2 OF S8

- 5. OPEN EXCAVATIONS AROUND BUILDING PERIMETER MUST REMAIN DRY. REMOVE WATER FROM OPEN EXCAVATIONS PRIOR TO BACKFILLING.
- 6. SHORING AND BRACING FOR THE LATERAL SUPPORT OF EXCAVATION SHALL REMAIN IN PLACE UNTIL ALL PERMANENT STRUCTURAL SYSTEMS ARE COMPLETE AS APPROVED BY THE ENGINEER.
- 7. CONTRACTOR SHALL PROVIDE TEMPORARY BRACING FOR ALL FOUNDATION GRADE BEAMS DURING THE OPERATIONS OF BACKFILLING AND COMPACTION.
- 8. ALL REQUIRED INSERT SLEEVES, CONDUITS, EMBEDMENTS AND PENETRATIONS MUST BE VERIFIED WITH RESPECTIVE TRADES BEFORE CASTING CONCRETE.
- 9. NO FOUNDATION ELEMENT, BEAM OR SLABS SHALL BE PLACED ON FROZEN SOIL OR IN WATER.
- 10. THE OWNER AND ENGINEER ASSUME NO RESPONSIBILITY FOR THE VALIDITY OF THE SUBSURFACE CONDITIONS DESCRIBED ON THE DRAWINGS, SPECIFICATIONS, BORING LOGS, OR TEST PITS. THE DATA IS INCLUDED ONLY TO ASSIST THE CONTRACTOR DURING BIDDING AND SUBSEQUENT CONSTRUCTION AND REPRESENT CONDITIONS ONLY OF THESE SPECIFIED LOCATIONS AT THE PARTICULAR TIME THEY WERE MADE.
- 11. ALL ORGANIC SOILS SUCH AS TOPSOIL OR ORGANIC FILL FOUND NEAR THE SURFACE IN SLAB LOCATIONS MUST BE REMOVED. THE UPPER TWO FEET OF FILL AND ANY ORGANIC FILL MATERIALS EXPOSED AT THE BASE OF EXCAVATION SHOULD BE REMOVED TO INORGANIC FILL OR UNDISTURBED SILTY SANDS. COMPACTED STRUCTURAL FILL SHALL BE USED AS NEEDED TO GRADE BEFORE GRAVEL BASE AND SLAB PLACEMENT.



HANGER SCHEDULE			
ERIAL	HARDWARE		
3⁄4" LVLs	MGU3.63-SDS		
¾" LVLs	HGU5.50-SDS		
¾" LVLs	HHGU7.25-SDS		
	•		

DRAWING NOTES:

1. REFER TO ARCHITECTURAL PLANS FOR ELEVATIONS AND FLOOR LAYOUTS. NOTIFY ENGINEER IF CONDITIONS VARY FROM SHOWN ON THESE PLANS. REFER TO GENERAL STRUCTURAL NOTES (SHEET S1) FOR ADDITIONAL INFORMATION AND SPECIFICATIONS.

TIMBER FRAMING MATERIALS:

- 1. TIMBER FRAMING MEMBERS SHOWN ON THIS PLAN HAVE BEEN DESIGNED TO MEET THE STANDARD FRAMING SPECIFICATIONS, NOTED IN THE GENERAL STRUCTURAL NOTES ON SHEET S1 OF THIS PLAN SET.
- 2. TIMBER FRAMING MEETING STANDARD SPECIFICATIONS, IN GENERAL, WILL BE ABBREVIATED ON THESE STRUCTURAL PLANS, UNLESS NOTED SPECIFICALLY OTHERWISE ON STRUCTURAL PLANS - ALL TIMBER FRAMING MATERIALS ARE TO MEET THE FOLLOWING SPECIFICATIONS:

CONVENTIONAL LUMBER:

a. BOARDS & BEAMS

- INTERIOR (UNTREATED): SPRUCE-PINE-FIR (SPF), GRADE #2
- EXTERIOR (TREATED): SOUTHERN YELLOW PINE (SYP) PRESSURE TREATED (PT), GRADE #2.
- b. POSTS
- INTERIOR (UNTREATED) SPECIES AS NOTED ON DRAWING.
- DOUGLAS-FIR-LARCH (DFL), GRADE #2.
- EXTERIOR (TREATED): SOUTHERN YELLOW PINE (SYP) PRESSURE TREATED (PT), GRADE #2.

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	COMMENTS					
	NO. DATE					
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PROJECT:			170 MOUNT VERNON STREET,	WEST ROXBURY MA 02132		
SHEET TITLE:				FRAMING PLAN		
CH SC	ECKE ALE: TE:	D. G <i>DBY:</i> W. 1/4"	uerre Gree = 1'-)3, 20	n -0"		
	S	53	OF S	58		

ENGINEERED LUMBER:

a. I-JOISTS

b. BOARDS & BEAMS: • INTERIOR (UNTREATED): LAMINATED VENEER LUMBER (LVL) SOUTHERN-PINE (SP), GRADE 2.1E 3100 SP, WIDTH 1¾" (UNO).

• SEE TO ENGINEERING PLANS FOR SPECIFICATIONS - JOIST TYPE/GRADE VARIES. REFER TO JOIST

• JOIST HANGERS SHALL BE METAL AND ARE TO BE OF SUFFICIENT LOAD RATING TO CARRY DESIGN

BY MANUFACTURER (FASTENERS, STIFFENERS, ETC) TO OBTAIN PROPER JOIST HANGER CAPACITY.

LOADS, HANGER TYPE/STYLE IS CONTRACTOR PREFERENCE. FOLLOW INSTALLATION REQUIREMENTS

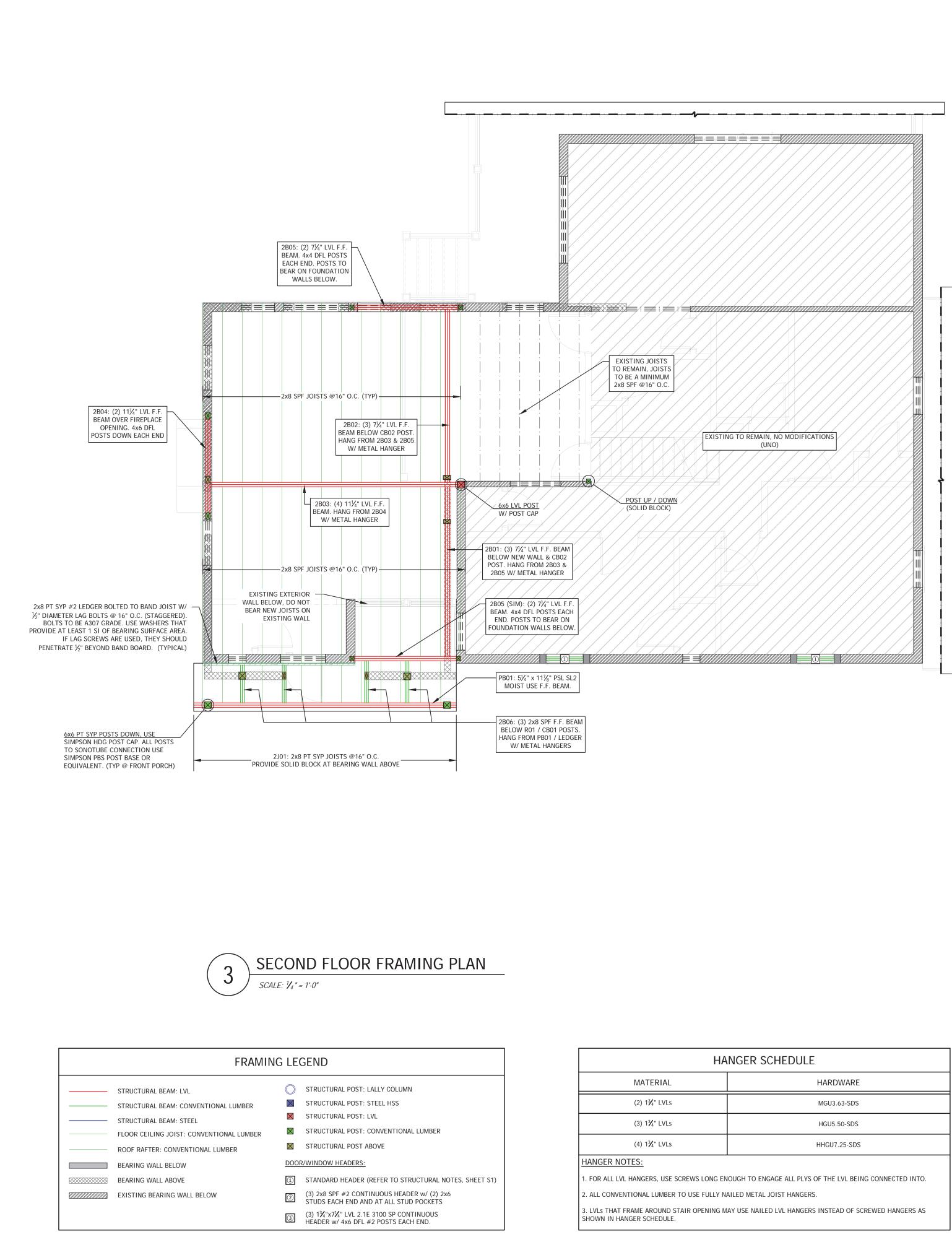
REQUIRED I-JOIST BRACING, STIFFENERS, and/or CONNECTORS.

MANUFACTURER INSTRUCTIONS (AS WELL AS STRUCTURAL PLANS AND CALCULATIONS) FOR

• EXTERIOR (TREATED): PARALLEL STRAND LUMBER (PSL) w/ PRESERVATIVE TREATMENT. (BEAM SIZE AS NOTED ON STRUCTURAL FRAMING PLANS). EXTERIOR PSL HORIZONTAL MEMBERS TO BE TRUSJOIST® 2.0E PARALLAM® PLUS PSL SL2 MOIST USE RATED; OR AN APPROVED EQUIVALENT BY ENGINEER.

c. POSTS:

• INTERIOR (UNTREATED): LAMINATED VENEER LUMBER (LVL) SOUTHERN-PINE (SP), GRADE 1.8E 2650 • EXTERIOR (TREATED): PARALLEL STRAND LUMBER (PSL) w/ PRESERVATIVE TREATMENT. (POST SIZE AS NOTED ON PLAN). EXTERIOR PSL HORIZONTAL MEMBERS TO BE TRUSJOIST ® 2.0E PARALLAM® PLUS PSL SL2 MOIST USE RATED; OR AN APPROVED EQUIVALENT BY ENGINEER.



HANGER SCHEDULE			
ERIAL	HARDWARE		
4" LVLs	MGU3.63-SDS		
4" LVLs	HGU5.50-SDS		
4" LVLs	HHGU7.25-SDS		

DRAWING NOTES:

1. REFER TO ARCHITECTURAL PLANS FOR ELEVATIONS AND FLOOR LAYOUTS. NOTIFY ENGINEER IF CONDITIONS VARY FROM SHOWN ON THESE PLANS. REFER TO GENERAL STRUCTURAL NOTES (SHEET S1) FOR ADDITIONAL INFORMATION AND SPECIFICATIONS.

TIMBER FRAMING MATERIALS:

- 1. TIMBER FRAMING MEMBERS SHOWN ON THIS PLAN HAVE BEEN DESIGNED TO MEET THE STANDARD FRAMING SPECIFICATIONS, NOTED IN THE GENERAL STRUCTURAL NOTES ON SHEET S1 OF THIS PLAN SET.
- 2. TIMBER FRAMING MEETING STANDARD SPECIFICATIONS, IN GENERAL, WILL BE ABBREVIATED ON THESE STRUCTURAL PLANS, UNLESS NOTED SPECIFICALLY OTHERWISE ON STRUCTURAL PLANS - ALL TIMBER FRAMING MATERIALS ARE TO MEET THE FOLLOWING SPECIFICATIONS:

CONVENTIONAL LUMBER:

a. BOARDS & BEAMS

- INTERIOR (UNTREATED): SPRUCE-PINE-FIR (SPF), GRADE #2
- EXTERIOR (TREATED): SOUTHERN YELLOW PINE (SYP) PRESSURE TREATED (PT), GRADE #2.
- b. POSTS
- INTERIOR (UNTREATED) SPECIES AS NOTED ON DRAWING.
- DOUGLAS-FIR-LARCH (DFL), GRADE #2.
- EXTERIOR (TREATED): SOUTHERN YELLOW PINE (SYP) PRESSURE TREATED (PT), GRADE #2.

S	STRUCTURAL ENGINEER:
	ENGINEER STAMP: TARA LYNN STRASSBURG CIVIL No. 55751 Java Lynn Strassburg ARA LYNN STRASSBURG, P.E.
	COMMENTS
	NO-V-CER NO.
	PROJECT: KATIE & ALFONSO RESIDENCE 170 MOUNT VERNON STREET, WEST ROXBURY MA 02132
	SHEET TITLE: SECOND FLOOR FRAMING PLAN
	DRAWN BY: D. Guerrero CHECKED BY: W. Green SCALE: 1/4" = 1'-0" DATE: May 03, 2022
	S4 OF S8

ENGINEERED LUMBER:

a. I-JOISTS

b. BOARDS & BEAMS: • INTERIOR (UNTREATED): LAMINATED VENEER LUMBER (LVL) SOUTHERN-PINE (SP), GRADE 2.1E 3100 SP, WIDTH 1¾" (UNO).

• JOIST HANGERS SHALL BE METAL AND ARE TO BE OF SUFFICIENT LOAD RATING TO CARRY DESIGN

BY MANUFACTURER (FASTENERS, STIFFENERS, ETC) TO OBTAIN PROPER JOIST HANGER CAPACITY.

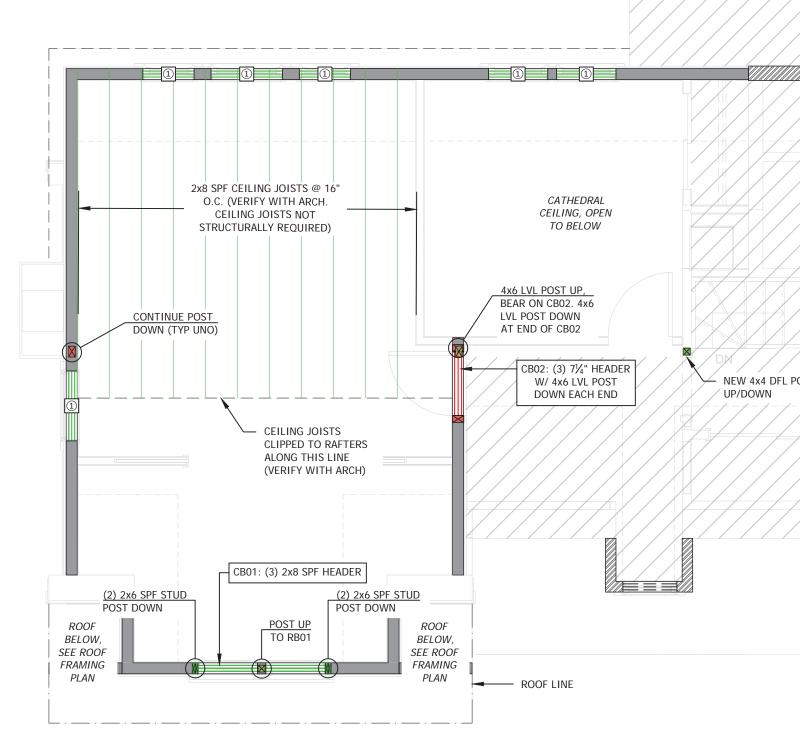
LOADS, HANGER TYPE/STYLE IS CONTRACTOR PREFERENCE. FOLLOW INSTALLATION REQUIREMENTS

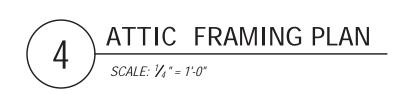
• EXTERIOR (TREATED): PARALLEL STRAND LUMBER (PSL) w/ PRESERVATIVE TREATMENT. (BEAM SIZE AS NOTED ON STRUCTURAL FRAMING PLANS). EXTERIOR PSL HORIZONTAL MEMBERS TO BE TRUSJOIST® 2.0E PARALLAM® PLUS PSL SL2 MOIST USE RATED; OR AN APPROVED EQUIVALENT BY ENGINEER.

c. POSTS:

• INTERIOR (UNTREATED): LAMINATED VENEER LUMBER (LVL) SOUTHERN-PINE (SP), GRADE 1.8E 2650 • EXTERIOR (TREATED): PARALLEL STRAND LUMBER (PSL) w/ PRESERVATIVE TREATMENT. (POST SIZE AS NOTED ON PLAN). EXTERIOR PSL HORIZONTAL MEMBERS TO BE TRUSJOIST ® 2.0E PARALLAM® PLUS PSL SL2 MOIST USE RATED; OR AN APPROVED EQUIVALENT BY ENGINEER.

• SEE TO ENGINEERING PLANS FOR SPECIFICATIONS - JOIST TYPE/GRADE VARIES. REFER TO JOIST MANUFACTURER INSTRUCTIONS (AS WELL AS STRUCTURAL PLANS AND CALCULATIONS) FOR REQUIRED I-JOIST BRACING, STIFFENERS, and/or CONNECTORS.





MAT
(2) 13
(3) 13
(4) 1 3
HANGER NOTES:
1. FOR ALL LVL HANGE
2. ALL CONVENTIONAL
3. LVLs THAT FRAME AF SHOWN IN HANGER SC

POST	EXISTING TO	REMAIN, NO M (UNO)		

HA	NGER SCHEDULE
ERIAL	HARDWARE
4" LVLs	MGU3.63-SDS
4" LVLs	HGU5.50-SDS
4" LVLs	HHGU7.25-SDS

GERS, USE SCREWS LONG ENOUGH TO ENGAGE ALL PLYS OF THE LVL BEING CONNECTED INTO. LUMBER TO USE FULLY NAILED METAL JOIST HANGERS.

AROUND STAIR OPENING MAY USE NAILED LVL HANGERS INSTEAD OF SCREWED HANGERS AS CHEDULE.

DRAWING NOTES:

1. REFER TO ARCHITECTURAL PLANS FOR ELEVATIONS AND FLOOR LAYOUTS. NOTIFY ENGINEER IF CONDITIONS VARY FROM SHOWN ON THESE PLANS. REFER TO GENERAL STRUCTURAL NOTES (SHEET S1) FOR ADDITIONAL INFORMATION AND SPECIFICATIONS.

TIMBER FRAMING MATERIALS:

- 1. TIMBER FRAMING MEMBERS SHOWN ON THIS PLAN HAVE BEEN DESIGNED TO MEET THE STANDARD FRAMING SPECIFICATIONS, NOTED IN THE GENERAL STRUCTURAL NOTES ON SHEET S1 OF THIS PLAN SET.
- 2. TIMBER FRAMING MEETING STANDARD SPECIFICATIONS, IN GENERAL, WILL BE ABBREVIATED ON THESE STRUCTURAL PLANS, UNLESS NOTED SPECIFICALLY OTHERWISE ON STRUCTURAL PLANS - ALL TIMBER FRAMING MATERIALS ARE TO MEET THE FOLLOWING SPECIFICATIONS:

CONVENTIONAL LUMBER:

a. BOARDS & BEAMS

- INTERIOR (UNTREATED): SPRUCE-PINE-FIR (SPF), GRADE #2
- EXTERIOR (TREATED): SOUTHERN YELLOW PINE (SYP) PRESSURE TREATED (PT), GRADE #2.
- b. POSTS
- INTERIOR (UNTREATED) SPECIES AS NOTED ON DRAWING.
- DOUGLAS-FIR-LARCH (DFL), GRADE #2.
- EXTERIOR (TREATED): SOUTHERN YELLOW PINE (SYP) PRESSURE TREATED (PT), GRADE #2.

	TRUCTURAL ENGINEER: SSB Engineering, LLC 146 Front Street, Scituate MA 02066 www.ssbengineering.com 857.504.1065 NGINEER STAMP:
Τ/	TARA LYNN STRASSBURG CIVIL NO. 55751 Jana Lynn Strasburg Jana Lynn Strasburg ARA LYNN STRASSBURG, P.E.
	COMMENTS
	NZO-V-CER NO.
	PROJECT: KATIE & ALFONSO RESIDENCE 170 MOUNT VERNON STREET, WEST ROXBURY MA 02132
	sheet title ATTIC FRAMING PLAN
	DRAWN BY: D. Guerrero CHECKED BY: W. Green SCALE: 1/4" = 1'-0" DATE: May 03, 2022
	S5 OF S8

ENGINEERED LUMBER:

a. I-JOISTS

b. BOARDS & BEAMS: • INTERIOR (UNTREATED): LAMINATED VENEER LUMBER (LVL) SOUTHERN-PINE (SP), GRADE 2.1E 3100 SP, WIDTH 1¾" (UNO).

• SEE TO ENGINEERING PLANS FOR SPECIFICATIONS - JOIST TYPE/GRADE VARIES. REFER TO JOIST

• JOIST HANGERS SHALL BE METAL AND ARE TO BE OF SUFFICIENT LOAD RATING TO CARRY DESIGN

BY MANUFACTURER (FASTENERS, STIFFENERS, ETC) TO OBTAIN PROPER JOIST HANGER CAPACITY.

REQUIRED I-JOIST BRACING, STIFFENERS, and/or CONNECTORS.

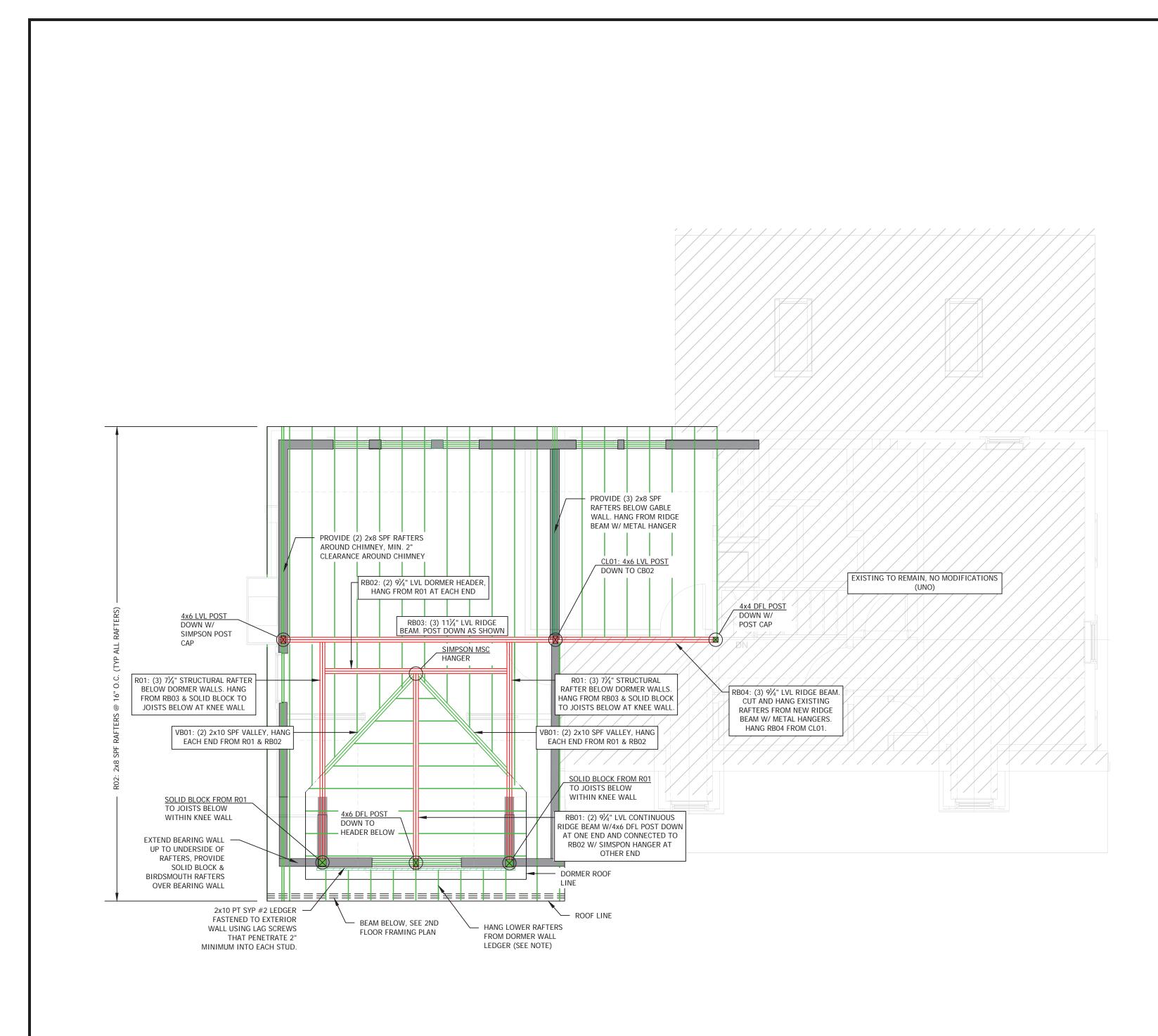
MANUFACTURER INSTRUCTIONS (AS WELL AS STRUCTURAL PLANS AND CALCULATIONS) FOR

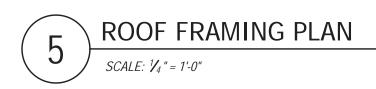
LOADS, HANGER TYPE/STYLE IS CONTRACTOR PREFERENCE. FOLLOW INSTALLATION REQUIREMENTS

• EXTERIOR (TREATED): PARALLEL STRAND LUMBER (PSL) w/ PRESERVATIVE TREATMENT. (BEAM SIZE AS NOTED ON STRUCTURAL FRAMING PLANS). EXTERIOR PSL HORIZONTAL MEMBERS TO BE TRUSJOIST® 2.0E PARALLAM® PLUS PSL SL2 MOIST USE RATED; OR AN APPROVED EQUIVALENT BY ENGINEER.

c. POSTS:

• INTERIOR (UNTREATED): LAMINATED VENEER LUMBER (LVL) SOUTHERN-PINE (SP), GRADE 1.8E 2650 • EXTERIOR (TREATED): PARALLEL STRAND LUMBER (PSL) w/ PRESERVATIVE TREATMENT. (POST SIZE AS NOTED ON PLAN). EXTERIOR PSL HORIZONTAL MEMBERS TO BE TRUSJOIST ® 2.0E PARALLAM® PLUS PSL SL2 MOIST USE RATED; OR AN APPROVED EQUIVALENT BY ENGINEER.





NG LE	GEND
\bigcirc	STRUCTURAL POST: LALLY COLUMN
\times	STRUCTURAL POST: STEEL HSS
\mathbf{X}	STRUCTURAL POST: LVL
\boxtimes	STRUCTURAL POST: CONVENTIONAL LUMBER
\mathbf{X}	STRUCTURAL POST ABOVE
<u>D00</u>	R/WINDOW HEADERS:
1	STANDARD HEADER (REFER TO STRUCTURAL NOTES, SHEET S1)
2	(3) 2x8 SPF #2 CONTINUOUS HEADER w/ (2) 2x6 STUDS EACH END AND AT ALL STUD POCKETS
3	(3) $1\frac{3}{4}$ "x7 $\frac{1}{4}$ " LVL 2.1E 3100 SP CONTINUOUS HEADER w/ 4x6 DFL #2 POSTS EACH END.

Ν	ΛA	T
(:	2)	13⁄
(:	3)	1 ³ /
(4	4)	1 ³ ⁄
HANGER NOTES	<u>S:</u>	
1. For all LVL Ha	NG	6EF
2. ALL CONVENTIO	NA	٩L
3. LVLs THAT FRAN SHOWN IN HANGE		

HA	NGER SCHEDULE
ERIAL	HARDWARE
4" LVLs	MGU3.63-SDS
4" LVLs	HGU5.50-SDS
4" LVLs	HHGU7.25-SDS

ERS, USE SCREWS LONG ENOUGH TO ENGAGE ALL PLYS OF THE LVL BEING CONNECTED INTO. LUMBER TO USE FULLY NAILED METAL JOIST HANGERS.

ROUND STAIR OPENING MAY USE NAILED LVL HANGERS INSTEAD OF SCREWED HANGERS AS CHEDULE.

DRAWING NOTES:

1. REFER TO ARCHITECTURAL PLANS FOR ELEVATIONS AND FLOOR LAYOUTS. NOTIFY ENGINEER IF CONDITIONS VARY FROM SHOWN ON THESE PLANS. REFER TO GENERAL STRUCTURAL NOTES (SHEET S1) FOR ADDITIONAL INFORMATION AND SPECIFICATIONS.

TIMBER FRAMING MATERIALS:

- 1. TIMBER FRAMING MEMBERS SHOWN ON THIS PLAN HAVE BEEN DESIGNED TO MEET THE STANDARD FRAMING SPECIFICATIONS, NOTED IN THE GENERAL STRUCTURAL NOTES ON SHEET S1 OF THIS PLAN SET.
- 2. TIMBER FRAMING MEETING STANDARD SPECIFICATIONS, IN GENERAL, WILL BE ABBREVIATED ON THESE STRUCTURAL PLANS, UNLESS NOTED SPECIFICALLY OTHERWISE ON STRUCTURAL PLANS - ALL TIMBER FRAMING MATERIALS ARE TO MEET THE FOLLOWING SPECIFICATIONS:

CONVENTIONAL LUMBER:

- a. BOARDS & BEAMS
- INTERIOR (UNTREATED): SPRUCE-PINE-FIR (SPF), GRADE #2
- EXTERIOR (TREATED): SOUTHERN YELLOW PINE (SYP) PRESSURE TREATED (PT), GRADE #2.
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- DOUGLAS-FIR-LARCH (DFL), GRADE #2.
- EXTERIOR (TREATED): SOUTHERN YELLOW PINE (SYP) PRESSURE TREATED (PT), GRADE #2.

	SS 14 <i>WV</i> 85	RUCTURAL ENGINEER: Source of the second sec
-		TARA LYNN STRASSBURG CIVIL No. 55751 BIGISTERE CIVIL No. 55751 CONTRASSBURG, P.E.
		COMMENTS
		NZO-V-CER NZO-V-CER
		PROJECT: KATIE & ALFONSO RESIDENCE 170 MOUNT VERNON STREET, WEST ROXBURY MA 02132
		SHEET TITLE: ROOF FRAMING PLAN
		DRAWN BY: D. Guerrero CHECKED BY: W. Green SCALE: 1/4" = 1'-0" DATE: May 03, 2022
		S6 OF S8

ENGINEERED LUMBER:

a. I-JOISTS

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LOADS, HANGER TYPE/STYLE IS CONTRACTOR PREFERENCE. FOLLOW INSTALLATION REQUIREMENTS

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MANUFACTURER INSTRUCTIONS (AS WELL AS STRUCTURAL PLANS AND CALCULATIONS) FOR

• EXTERIOR (TREATED): PARALLEL STRAND LUMBER (PSL) w/ PRESERVATIVE TREATMENT. (BEAM SIZE AS NOTED ON STRUCTURAL FRAMING PLANS). EXTERIOR PSL HORIZONTAL MEMBERS TO BE TRUSJOIST® 2.0E PARALLAM® PLUS PSL SL2 MOIST USE RATED; OR AN APPROVED EQUIVALENT BY ENGINEER.

c. POSTS:

• INTERIOR (UNTREATED): LAMINATED VENEER LUMBER (LVL) SOUTHERN-PINE (SP), GRADE 1.8E 2650 • EXTERIOR (TREATED): PARALLEL STRAND LUMBER (PSL) w/ PRESERVATIVE TREATMENT. (POST SIZE AS NOTED ON PLAN). EXTERIOR PSL HORIZONTAL MEMBERS TO BE TRUSJOIST ® 2.0E PARALLAM® PLUS PSL SL2 MOIST USE RATED; OR AN APPROVED EQUIVALENT BY ENGINEER.

	BRACED WALL REQUIREMENTS (130-140MPH	H) PER R602.10	FOR WSP	
		REQUIRED	ACTUAL	COMPLIANCE
R602.10.1.3	MAXIMUM BRACED WALL LINE SPACING	60 FEET	50 FEET	PASS
R602.10.3(1)	BRACING REQUIREMENTS BASED ON WIND SPEED (<130MPH)			
	# FEET OF BRACED WALL @1ST FLOOR	11 FEET	12 FEET	PASS
R602.10.5	MINIMUM LENGTH OF BRACED WALL PANELS	4 FEET	4 FEET	PASS

BRACED WALL LINE

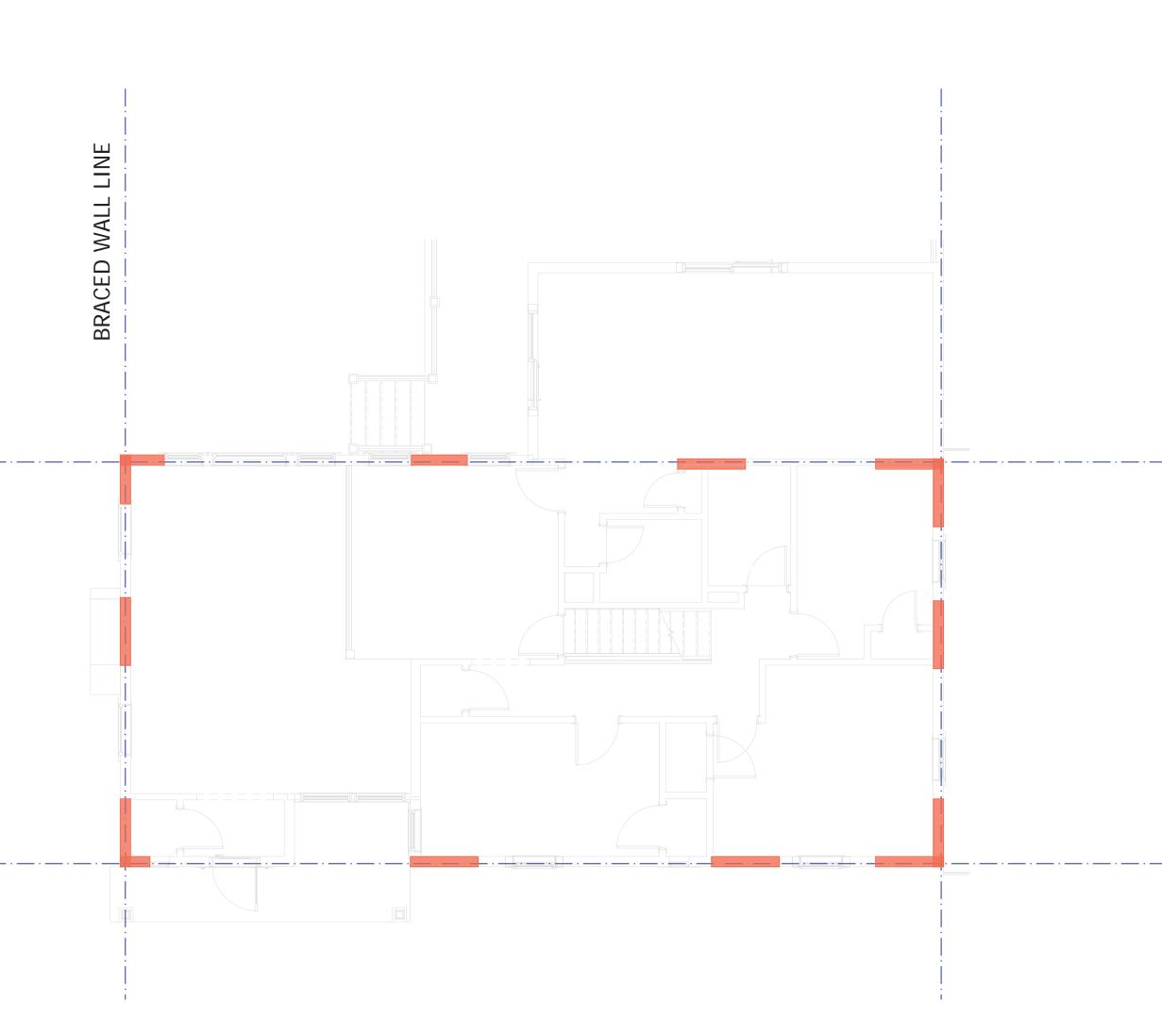
BRACED WALL PLAN - 1ST FLOOR

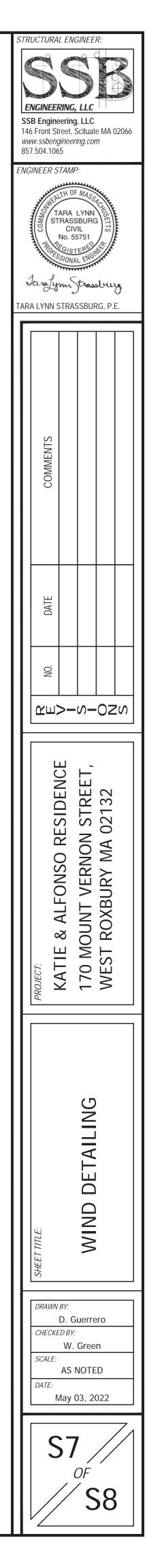
SCALE: 1/8 " = 1'-0"

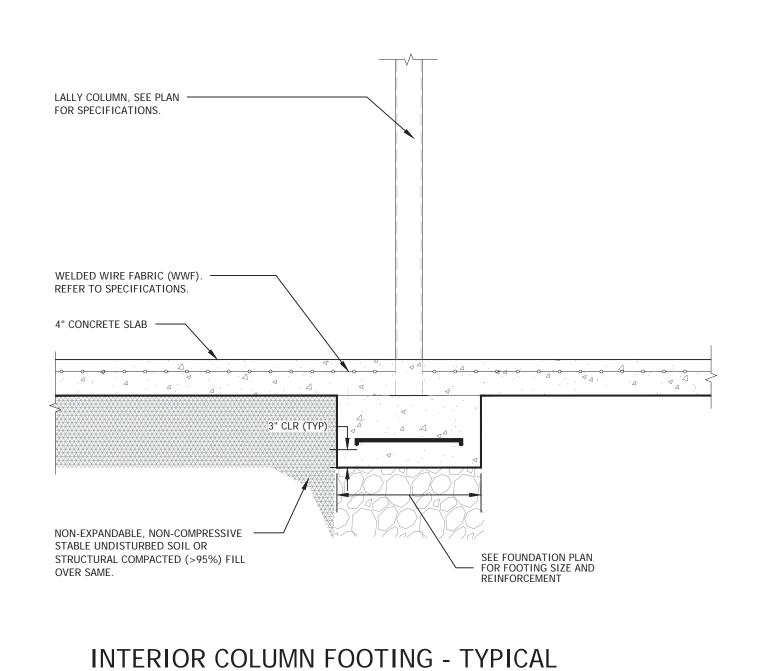
							ULTIMATE DESIGN	WIND SPEED (MPH)
MINIMU		MINIMUM WOOD	MINIMUM NOMINAL MAXIMUM WALL PANEL NAIL SPACING PANEL THICKNESS STUD SPACING WIND EXPOSU		JRE CATEGORY				
SIZE	PENETRATION (IN.)	SPAN RATING	(INCHES)	(INCHES)	EDGES (INCHES O.C.)	FIELD (INCHES O.C.)	В	с	D
6d COMMON (2.0" x 0.113")	1.5	24/0	3/8	16	6	12	140	115	110
	1.75	24/14	7/16	16	6	12	170	140	135
3d COMMON (2.5" x 0.131")	1.75	24/16	// 10	24	6	12	140	115	110

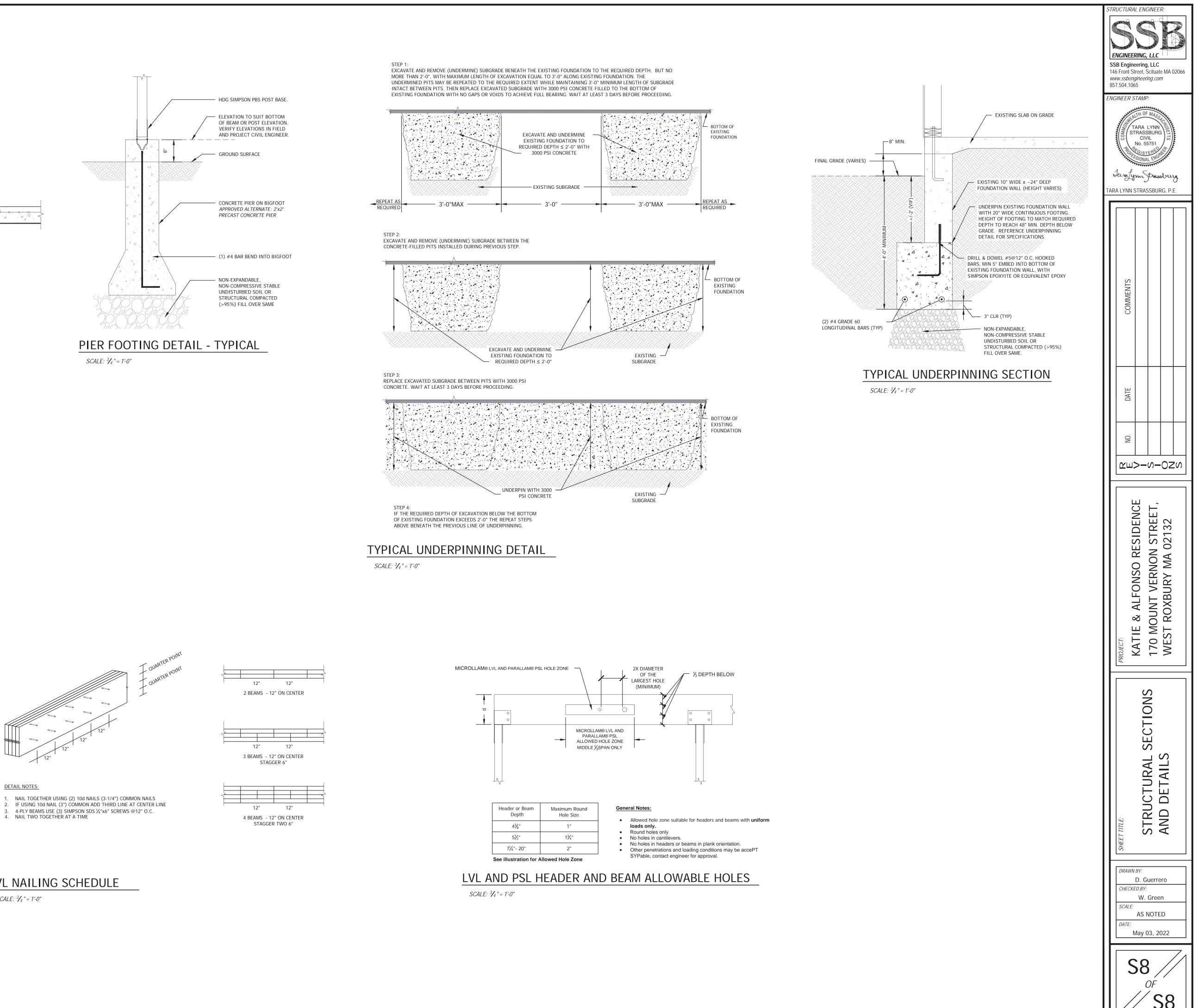
FOR SI: 1 INCH = 25.4 MM, 1 FOOT = 304.8 MM, 1 MILE PER HOUR = 0.447 M/S; 1 KSI = 6.895 MPA.

1. PANEL STRENGTH AXIS PARALLEL OR PERPENDICULAR TO SUPPORTS. THREE-PLY PLYWOOD SHEATHING WITH STUDS SPACED MORE THAN 16 INCHES ON CENTER SHALL BE APPLIED WITH PANEL STRENGTH AXIS PERPENDICULAR TO SUPPORTS. 2. TABLE IS BASED ON WIND PRESSURES ACTING TOWARD AND AWAY FROM BUILDING SURFACES IN ACCORDANCE WITH SECTION R301.2. LATERAL BRACING REQUIREMENTS SHALL BE IN ACCORDANCE WITH SECTION R602.10. 3. WOOD STRUCTURAL PANELS WITH SPAN RATINGS OF WALL-16 OR WALL-24 SHALL BE PERMITTED AS AN ALTERNATE TO PANELS WITH A 24/0 SPAN RATING. PLYWOOD SIDING RATED 16 O.C. OR 24 O.C. SHALL BE PERMITTED AS AN ALTERNATE TO PANELS WITH A 24/16 SPAN RATING. WALL-16 AND PLYWOOD SIDING 16 O.C. SHALL BE USED WITH STUDS SPACED NOT MORE THAN 16 INCHES ON CENTER.

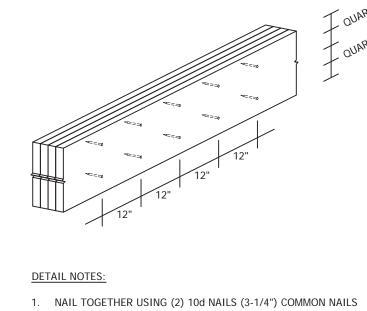








SCALE: 3⁄4 " = 1'-0"

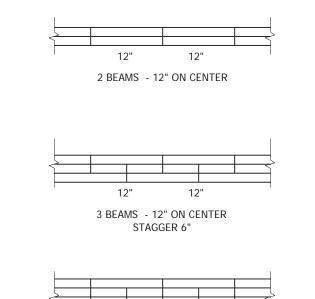


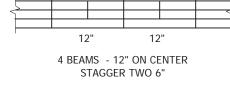
2. IF USING 10d NAIL (3") COMMON ADD THIRD LINE AT CENTER LINE

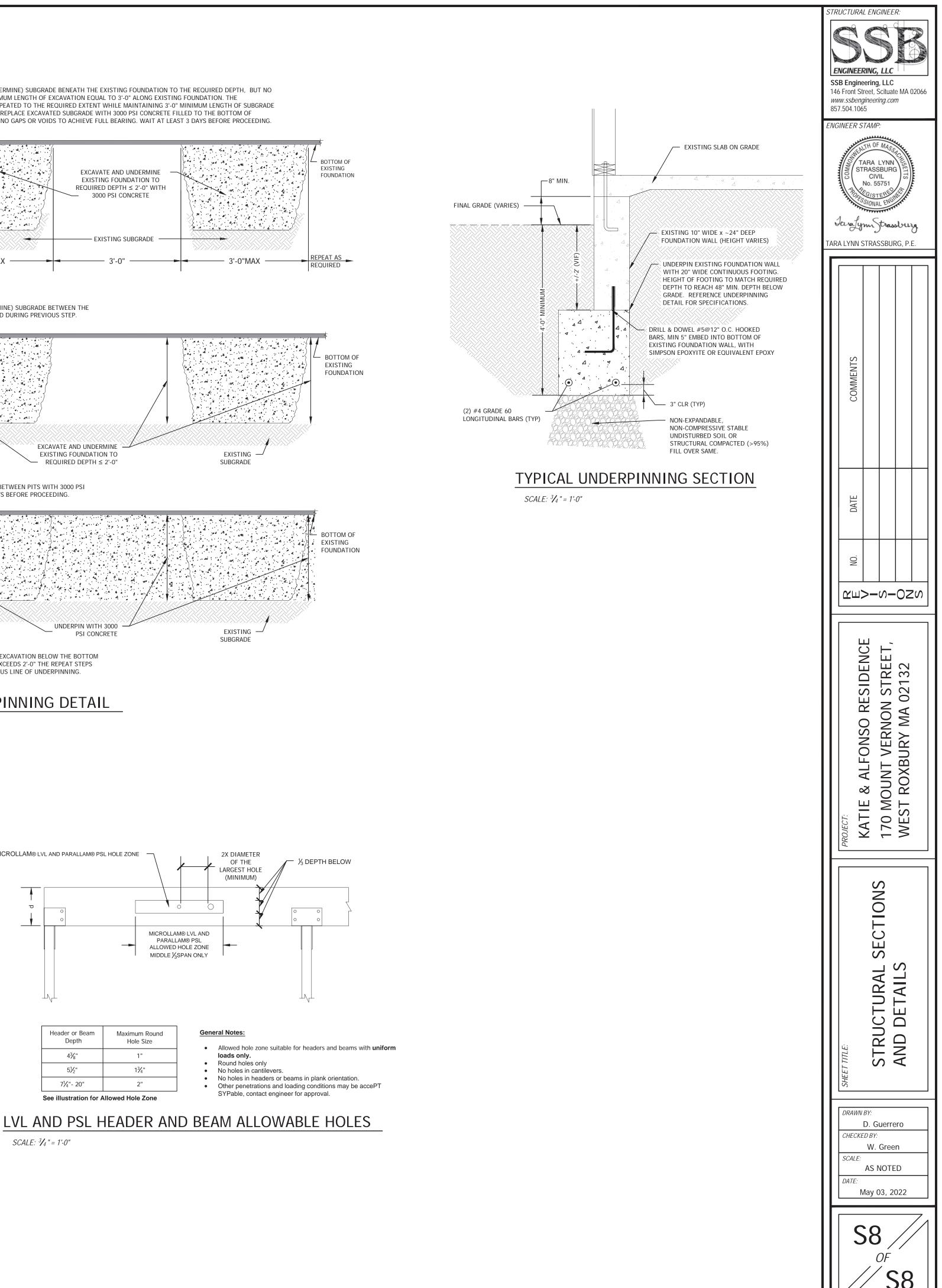
LVL NAILING SCHEDULE

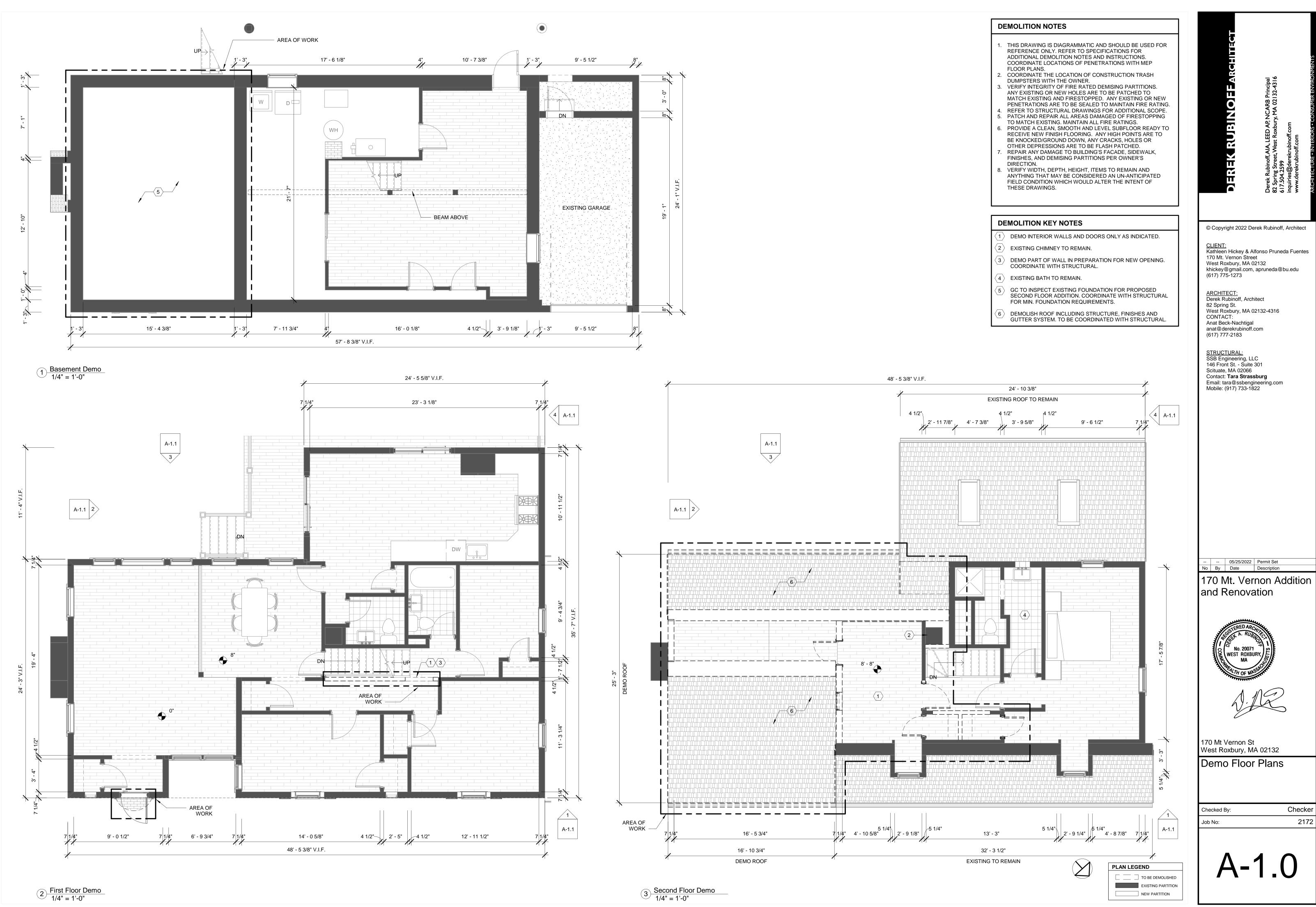
SCALE: 3⁄4 " = 1'-0"

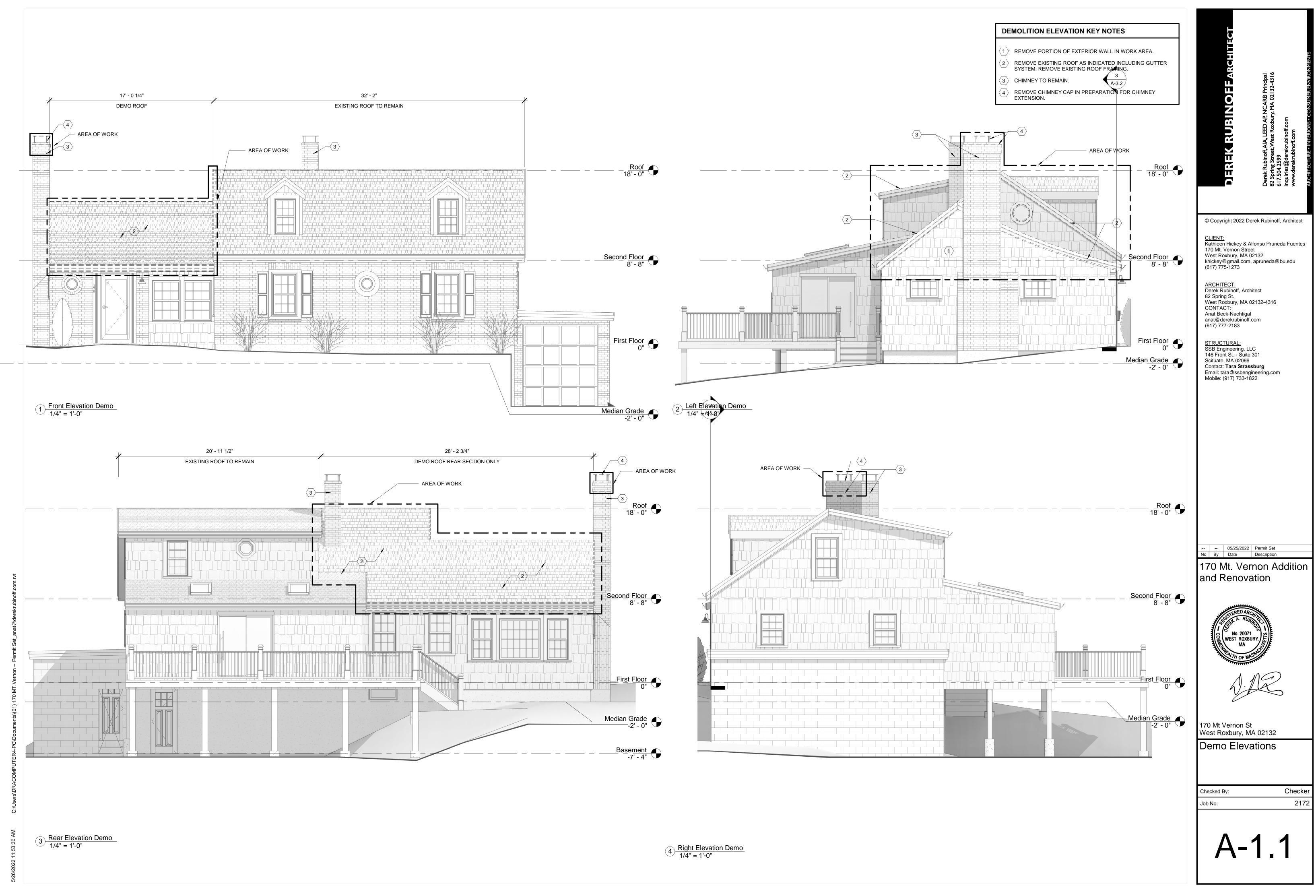


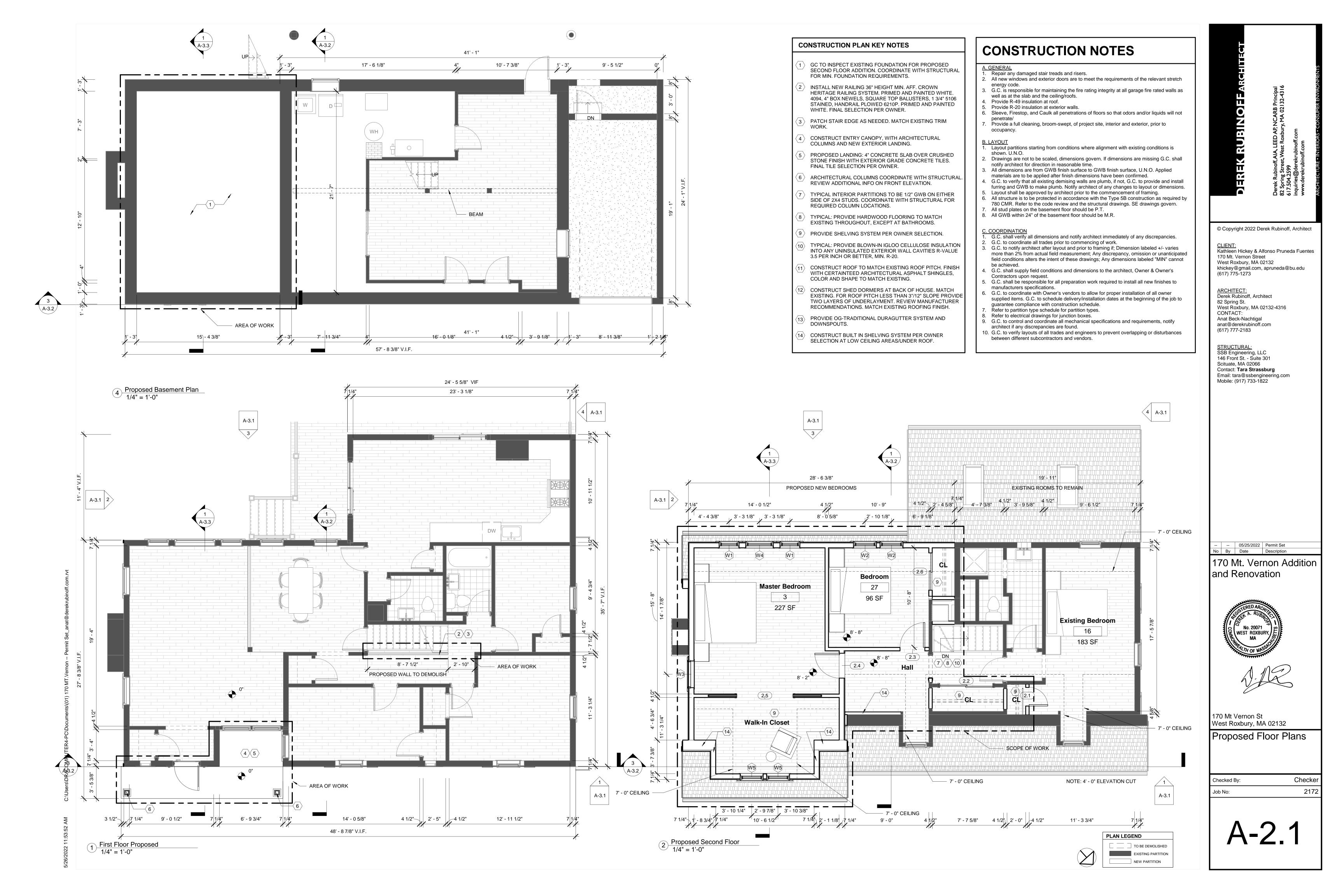








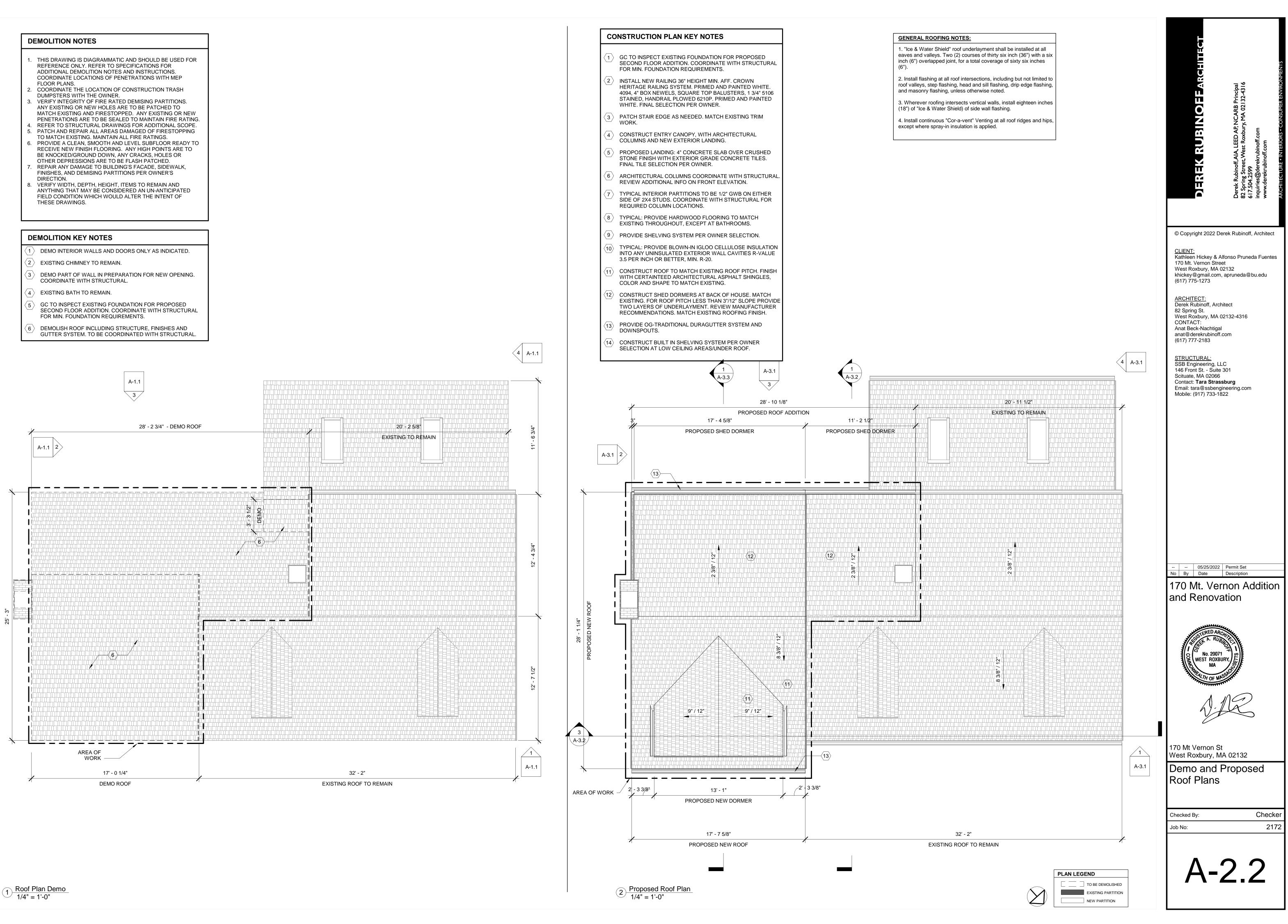




- REFERENCE ONLY. REFER TO SPECIFICATIONS FOR ADDITIONAL DEMOLITION NOTES AND INSTRUCTIONS. COORDINATE LOCATIONS OF PENETRATIONS WITH MEP
- ANY EXISTING OR NEW HOLES ARE TO BE PATCHED TO

- BE KNOCKED/GROUND DOWN, ANY CRACKS, HOLES OR OTHER DEPRESSIONS ARE TO BE FLASH PATCHED. REPAIR ANY DAMAGE TO BUILDING'S FACADE, SIDEWALK,
- DIRECTION.
- FIELD CONDITION WHICH WOULD ALTER THE INTENT OF THESE DRAWINGS.

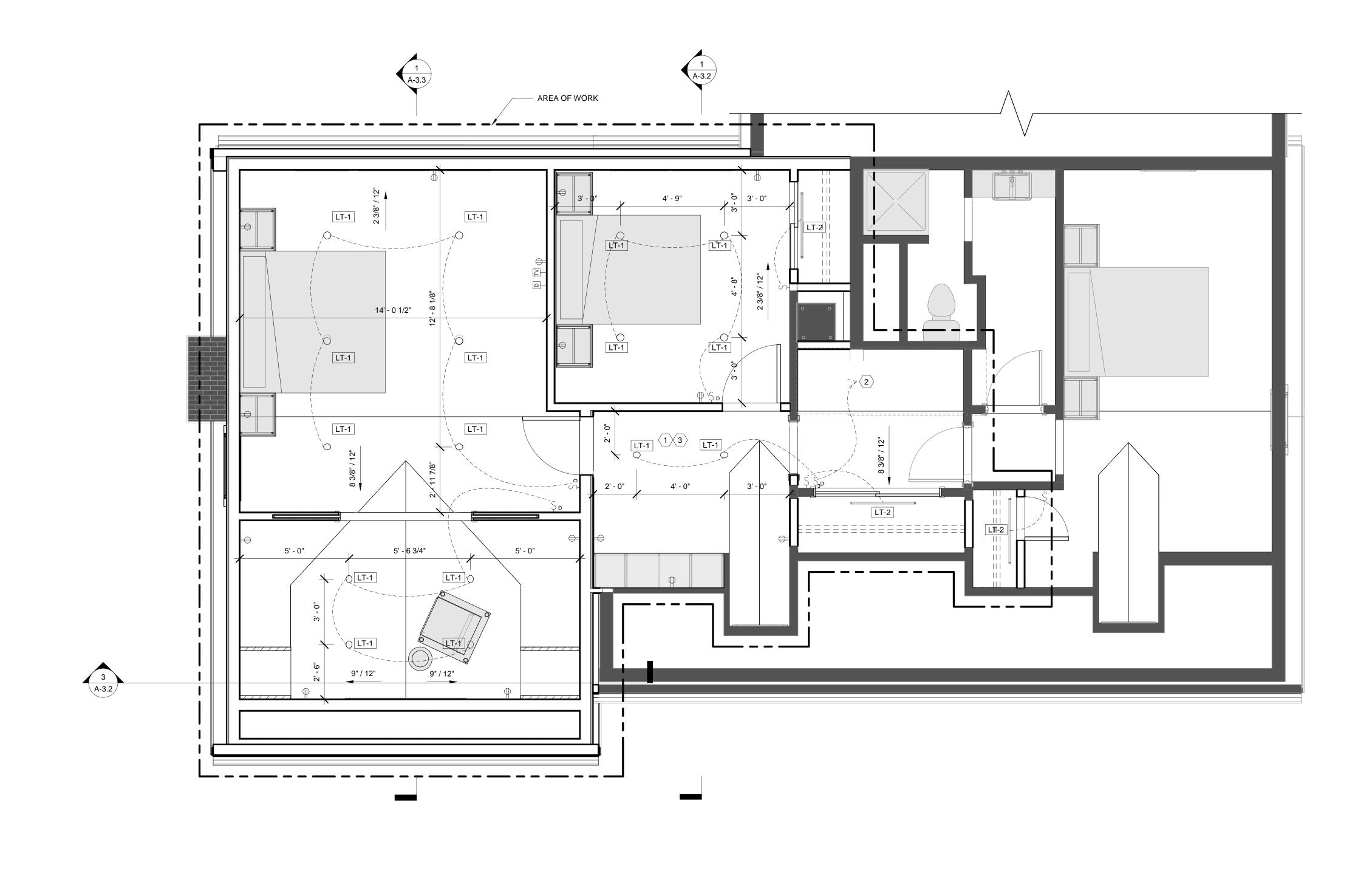
- GC TO INSPECT EXISTING FOUNDATION FOR PROPOSED FOR MIN. FOUNDATION REQUIREMENTS.
- DEMOLISH ROOF INCLUDING STRUCTURE, FINISHES AND



Family	Count	
Outlet-Communications	1	
Outlet-Communications-D	1	
Outlet-Duplex	15	
Switch-Dimmer	3	
Switch-Dimmer-Three Way	1	
Switch-Single	3	

Lighting Fixture Schedule (GC to Furnish and Install)										
Type Mark	Count	Description	Comments	Lamp						
LT-1	16	Downlight Recessed LED 4" Adjustable	IC-Rated	LED						
LT-2	3	36" Closet Wall Light	Hardwired LED							

NOTES: 1. LIGHT COLOR 2700K 2. LIGHTS ON DIMMABLE SWITCH TO BE FULLY DIMMABLE



2 Second Floor RCP 3/8" = 1'-0"

CONSTRUCTION CEILING PLAN KEY NOTES

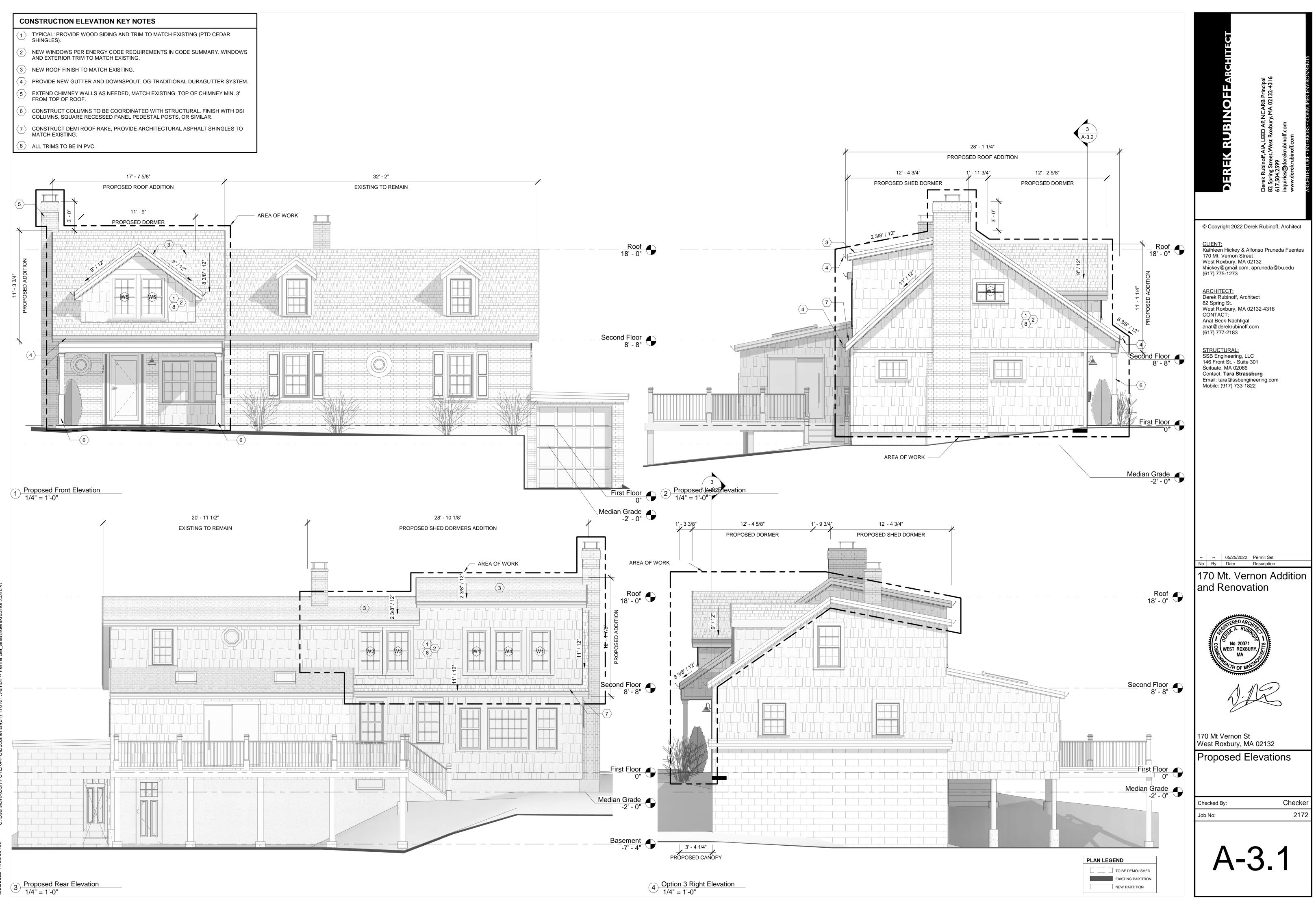
1 WALL OUTLETS SHOULD BE PLACED NO FURTHER THAN 12' APART, AT A HEIGHT OF BETWEEN 12"-18" AFF.

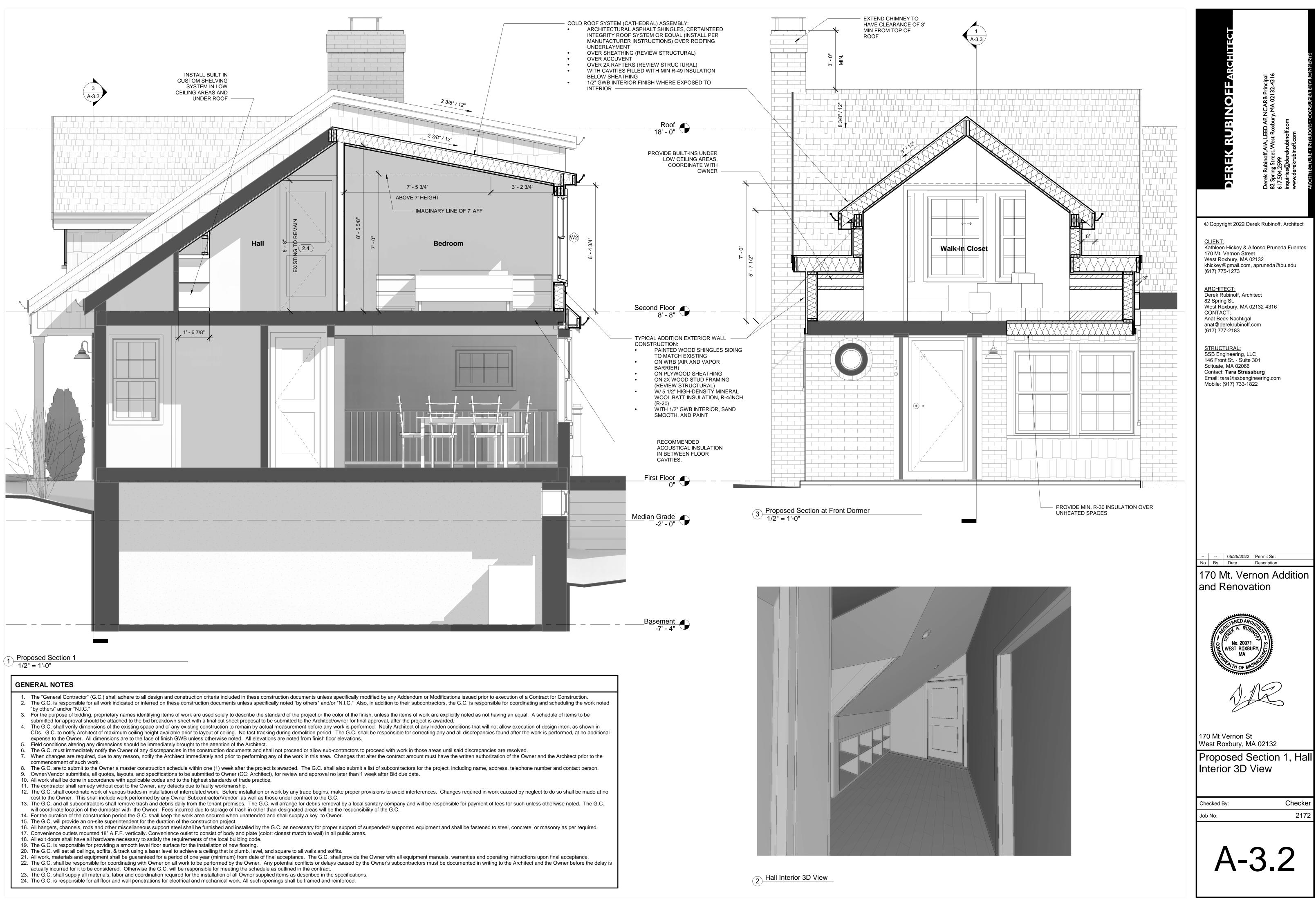
- $\langle 2 \rangle$ 3-WAY SWITCH TO FIRST/SECOND FLOOR.
- $\langle 3 \rangle$ Cathedral Ceiling on Second Floor. Provide "Cold Roof" insulation in this area. R-value 49 min.

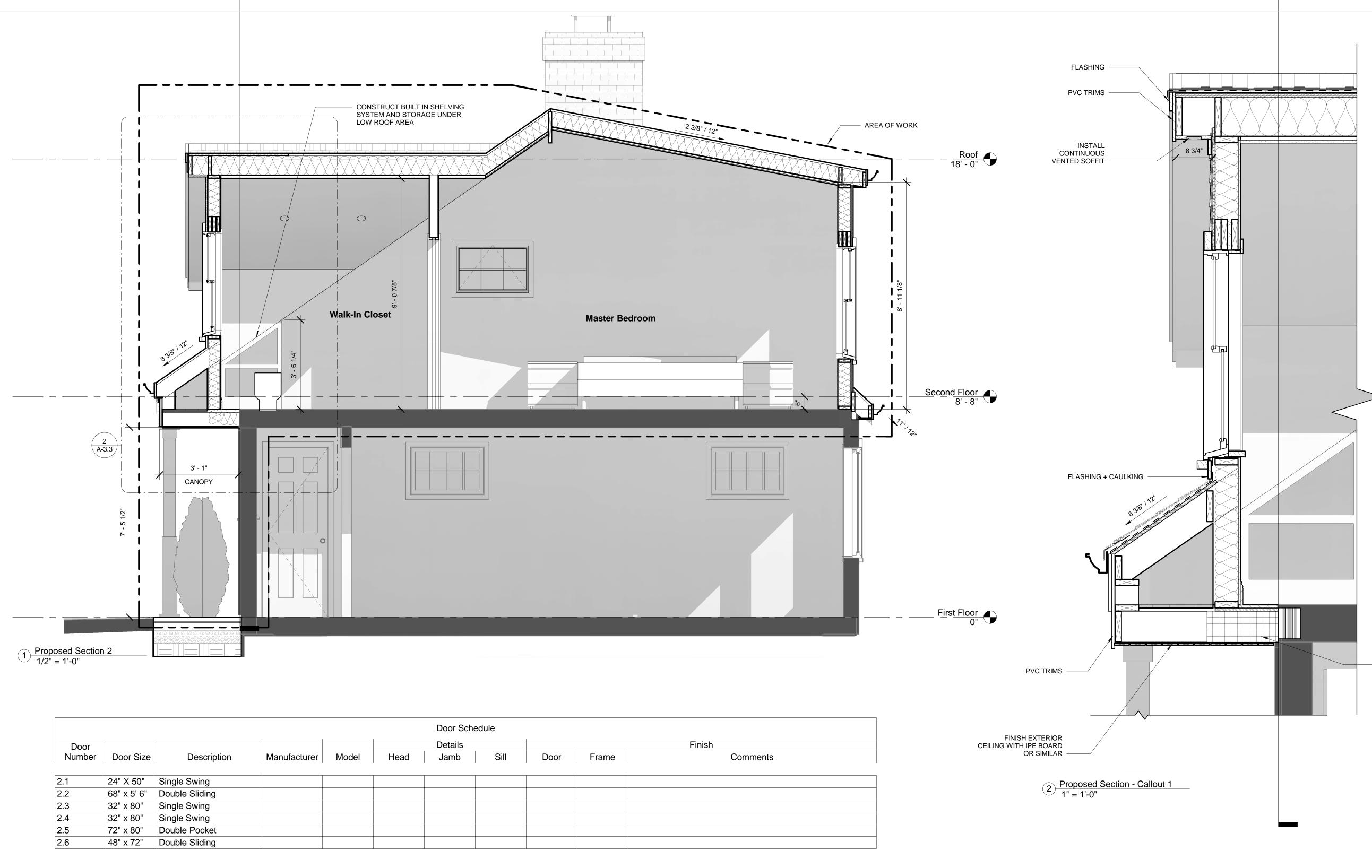
ELECTRICAL FIXTURE LEGEND

\$ ₃ S S 2	3-WAY SWITCH
5	SINGLE SWITCH
	DOUBLE SWITCH
Sd	SWITCH ON DIMMER
S _{3D}	3-WAY SWITCH ON DIMMER
TV	TV CONNECTOR
Þ	DATA
B	BELL CHIME AND RING
\square	OUTLET DUPLEX
\oplus_{GFI}	OUTLET GFI
\oplus_{R}	OUTLET RANGE
T P	THERMOSTAT
\bigoplus	DOUBLE-DUPLEX OUTLET
₩ex	OUTDOOR OUTLET
$\Psi_{\mathbf{D}}$	OUTLET DRYER

DEREK RUBINOFF ARCHITECT	Derek Rubinoff, AIA, LEED AP, NCARB Principal 82 Spring Street, West Roxbury, MA 02132-4316 617.504.2599 inquiries@derekrubinoff.com www.derekrubinoff.com AKCHIHECIURE-INNERIORS - CONSUMERENVIRONMENTS
© Copyright 2022 Dere	ek Rubinoff, Architect
CLIENT: Kathleen Hickey & Alfo 170 Mt. Vernon Street West Roxbury, MA 02 khickey@gmail.com, a (617) 775-1273	132
ARCHITECT: Derek Rubinoff, Archit 82 Spring St. West Roxbury, MA 02 CONTACT: Anat Beck-Nachtigal anat@derekrubinoff.cc	132-4316
(617) 777-2183 <u>STRUCTURAL:</u> SSB Engineering, LLC 146 Front St Suite 3 Scituate, MA 02066 Contact: Tara Strasst Email: tara@ssbengin Mobile: (917) 733-182) 01 burg eering.com
MODILE. (917) 733-162	2
	Permit Set Description
170 Mt. Verr and Renova	
STERED ARCH	
No. 20071 WEST ROXBUR MA	SILIS AN
D.J	R
170 Mt Vernon St West Roxbury, MA Proposed R (
	Ohashar
Checked By: Job No:	Checker 2172
A-2	72
	2.0







						Door Sche	edule			
Door						Details				Finish
Number	Door Size	Description	Manufacturer	Model	Head	Jamb	Sill	Door	Frame	Comments
2.1	24" X 50"	Single Swing								
2.2	68" x 5' 6"	Double Sliding								
2.3	32" x 80"	Single Swing								
2.4	32" x 80"	Single Swing								
2.5	72" x 80"	Double Pocket								
2.6	48" x 72"	Double Sliding								

DOORS NOTE

DOC	JRS NOTE:	<u>D00</u>	<u>IR HARDW</u>
1.	GC TO VERIFY ALL REPLACEMENT DOOR OPENING DIMENSIONS PRIOR TO PURCHASE.	1.	PROVI
2.	DOORS TO BE SOLID WOOD CORE, 2 PANEL TO MATCH EXISTING.		NICKE
3.	FINISH AND PAINT TO MATCH EXISTING.	2.	PROVI
4.	EXTERIOR DOORS TO BE INSULATED WITH LOW E GLASS. FINISH AND COLOR TO MATCH EXISTING.	3.	PROVI
5.	EXTERIOR PATIO SLIDING DOOR TO BE ANDERSON 400 SERIES (OR EQUAL)	4.	PROVI
6.	ALL FINISHES FINAL SELECTION, MANUFACTURER AND MATERIALS BY OWNER.	5.	PROVI

						Window Schedule									
		S	ize							Detail		Glaz	ing	Head	
Type Mark	Count	Width	Height	Туре	Manufacturer	Model	Material	Finish	Head	Jamb	Sill	Thickness	Туре	Height	Comments
								Γ						1	
W1	2	2' - 1 5/8"	4' - 4 7/8"	Window-Double_Hung-Andersen-400_Series-Tilt_Wash	Andersen Corporation	400-Series Tilt-Wash Double Hung								5' - 7 7/8"	
N2	2	2' - 5 5/8"	4' - 4 7/8"	Window-Double_Hung-Andersen-400_Series-Tilt_Wash	Andersen Corporation	400-Series Tilt-Wash Double Hung								5' - 7 7/8"	
W3	1	3' - 0"	2' - 0 1/8"	Window-Awning-Andersen-400_Series_Single	Andersen Corporation	400-Series Awning								6' - 0 1/8"	
W4	1	2' - 11 5/8"	4' - 4 7/8"	Window-Double_Hung-Andersen-400_Series-Tilt_Wash	Andersen Corporation	400-Series Tilt-Wash Double Hung								5' - 7 7/8"	
W5	2	2' - 5 5/8"	4' - 0 7/8"	Window-Double_Hung-Andersen-400_Series-Tilt_Wash	Andersen Corporation	400-Series Tilt-Wash Double Hung								6' - 2 7/8"	

WINDOWS NOTES:

GC TO VERIFY ALL REPLACEMENT WINDOW OPENING DIMENSION PRIOR TO PURCHASE.

WINDOWS TO MATCH EXISTING. PROVIDE SCREEN AND HARDWARE FOR THE OPERABLE WINDOWS. HARDWARE TO MATCH WINDOW FINISH.

LOW E GLASS.

ANY WINDOW WITH SILL HEIGHT LOWER THAN 24" AND ABOVE 60" FROM GRADE TO HAVE FALL PROTECTION DEVICE. ANY WINDOW NEAR SHOWER OR TUB ENCLOSURE TO HAVE TEMPERED GLASS.

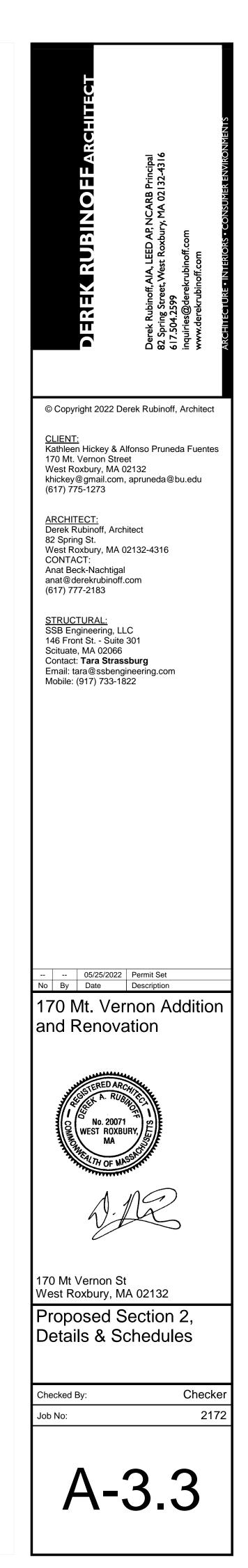
 DOOR HARDWARE NOTES:

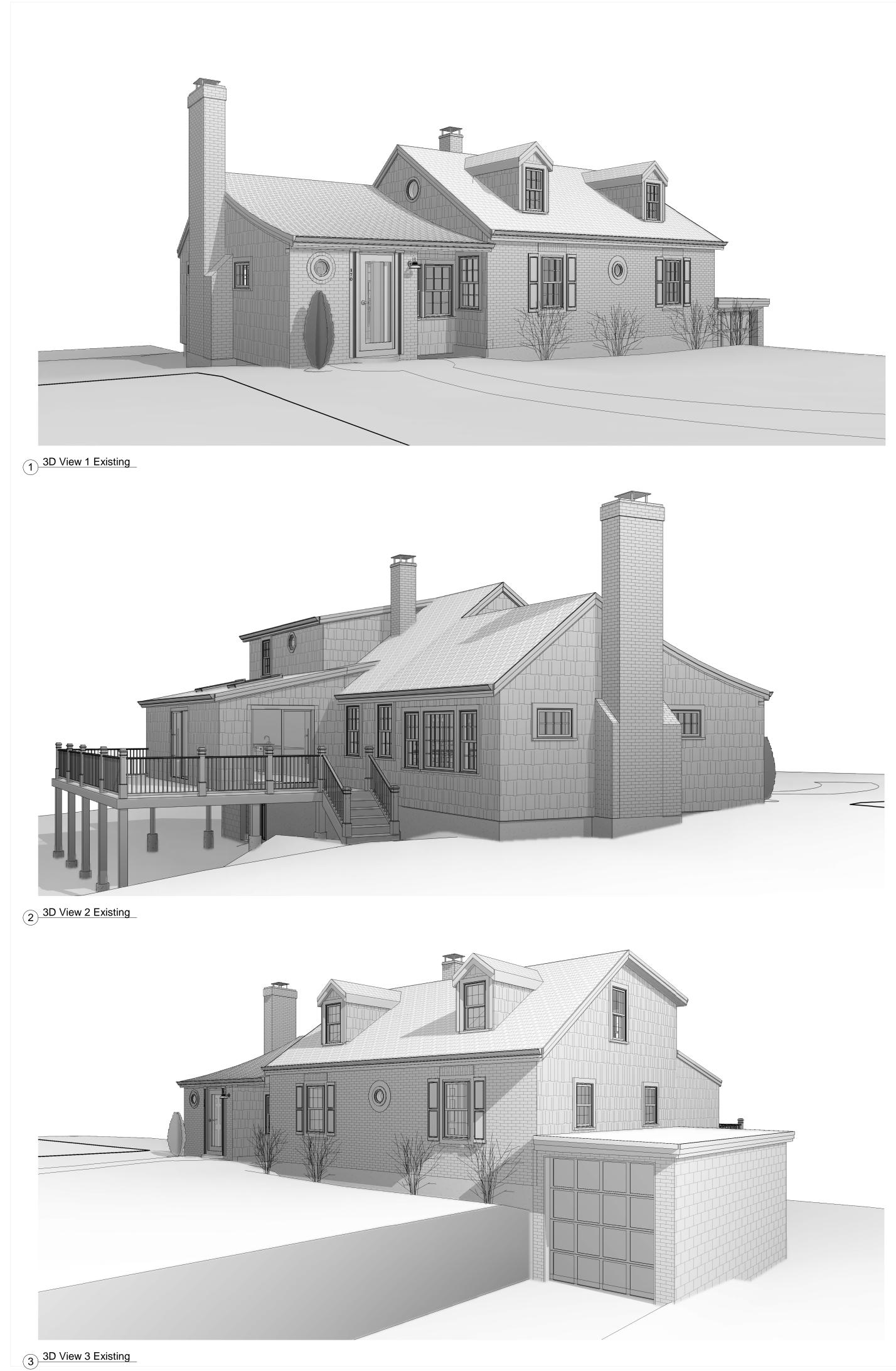
 1.
 PROVIDE SCHLAGE HARDWARE SETS, OR EQUAL, CUSTOM ALEXANDRIA GLASS KNOB WITH CAMELOT TRIM IN SATIN

 NICKEL FINISH (TO MATCH EXISTING HARDWARE THROUGHOUT HOUSE).

 OVIDE SCHLAGE, OR EQUAL, BED & BATH LOCK HARDWARE SET FOR ALL BEDROOMS AND BATHROOM DOORS. OVIDE SCHLAGE HALL & CLOSET HARDWARE SET FOR ALL CLOSETS AND STORAGE DOORS. OVIDE SCHLAGE HINGES (x3) 4" ROUND HINGE 5/8" RADIUS OVIDE DOOR OR WALL STOPS.

CLOSED CELL
INSULATION, MIN. R-30
OVER UNHEATED
SPACES



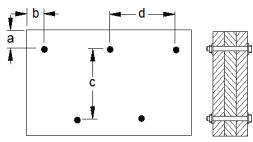








Boise Cascade	Trip	ole 1-3/4" >	c 7-1	/4" V	ER	SA-L	.AM	₿ L\	/L :	2.1E	3100	SP		P	ASSED
ENGINEERED WOOD PRODUCTS				2B0 ⁻	1 (FI	ush E	Beam	ı)						1.00	
BC CALC® Member Build 8381	Report					an N							Apri	l 25, 202	22 11:26:23
Job name:	Katie & Alfonso I						name		17	0 Mour	nt Verno	n Street,	West Ro	oxbury	
Address:	170 Mount Verno						criptio								
City, State, Zip:	West Roxbury, N	/IA, 02132					cifier:		De						
Customer: Code reports:	Derek Rubinoff ESR-1040						igner: npany			vid Gu B Engi	errero neering				
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\bowtie															
<u>k</u>					11	-06-00									
B1		-		• •											B2
Depotion Summ			otal Ho	orizonta	al Pro	duct L	ength	= 11-0	06-0	0					
Reaction Summ Bearing	Live	Dead			S	now			w	/ind		Roc	of Live		
B1, 3-1/2"	307 / 0	430 /	0			16/0									
B2, 3-1/2"	307 / 0	983 /	0		96	66 / 0									
												_			
Load Summary										Live	Dead	Snow	Wind	Roof Live	Tributary
Tag Description		d Type	Ref.				nd	Loc.		100%	90%	115%	1 60 %	125%	
0 Self-Weight 1 WALL		Lin. (lb/ft)	L	00-0		11-0	6-00	Top			11 0				00-00-00
1 WALL	Пар	bezoidal (lb/ft)	L	00-0	0-00	11-0	6-00	Тор			0 100				n\a
2 CB02	Con	c. Pt. (lbs)	L	09-0	3-00		3-00	Тор			558	1182			n\a
3 SECOND		Area (lb/ft ²)	L	00-0	0-00	11-0	6-00	Тор		40	10				01-04-00
Controls Summ	ary Value	%	Allow	ahle	г	Duratio	n	C	ase	Loc	ation				
Pos. Moment	3592 ft-lb		5.5%			15%			3)2-07				
End Shear	1898 lbs	22	2.8%		1	15%			2	10-0	07-04				
Total Load Deflection	(,).7%			ı∖a			3)2-15				
Live Load Deflection	L/999 (0.1	-				ı∖a			6)2-15				
Max Defl. Span / Depth	0.225" 18.3	22	2.5%		n	n∖a			3	06-0)2-15				
opan / Depin	10.5														
Notes															
Design meets Code															
Design meets Code Design meets arbitra															
Minimum bearing ler			1000101	i onton	u.										
Minimum bearing ler	•														
Design based on Dry															
BC CALC® analysis															
Calculations assume	member is brace	ed at all suppo	rts. Se	ee engi	neeri	ing rep	oort fo	or the	unbi	aced le	ength.				
Connection Dia	gram: Full Le	ngth of Me	mbe	r											





(23)	Boise Cascade	
VDV	ENGINEERED WOOD PRODUCTS	_

BC CALC® Member Report

Triple 1-3/4" x 7-1/4" VERSA-LAM® LVL 2.1E 3100 SP



2B01 (Flush Beam) Dry | 1 span | No cant.

Company:

April 25, 2022 11:26:23

Build 8381	
Job name:	Katie & Alfonso Residence
Address:	170 Mount Vernon Street
City, State, Zip:	West Roxbury, MA, 02132
Customer:	Derek Rubinoff
Code reports:	ESR-1040

File name: 170 Mount Vernon Street, West Roxbury Description: Specifier: Designer: David Guerrero

SSB Engineering

Connection Diagram: Full Length of Member

a minimum = 2" c = 3-1/4"b minimum = 2-1/2" d = 24"

Calculated Side Load = 0.0 lb/ftBolts are assumed to be Grade A307 or Grade 2 or higher. Connectors are: 1/2 in. Staggered Through Bolt



Disclosure

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BC CALC®, BC FRAMER®, AJS™, ALLJOIST®, BC RIM BOARD™, BCI®, BOISE GLULAM™, BC FloorValue®, VERSA-LAM®, VERSA-RIM PLUS®,

Boise Cascade	Triple 1	-3/4" x 7-′	1/4" VER	SA-LAM	R LVL :	2.1E 3	3100 S	SP		P	ASSED
ENGINEERED WOOD PRODUCTS			2B02 (F	lush Beam)						
BC CALC® Member Build 8381	Report		Dry 1 sp	oan No can	t.				Apri	1 25, 202	2 11:26:23
Job name:	Katie & Alfonso Resid	lence		File name	e: 17	0 Mount	t Vernor	Street,	West Ro	oxbury	
Address:	170 Mount Vernon St	reet		Description	on:						
City, State, Zip:	West Roxbury, MA, 0	2132		Specifier:							
Customer:	Derek Rubinoff			Designer		ivid Gue	errero				
Code reports:	ESR-1040			Company	r: SS	B Engir	neering				
					\downarrow \downarrow \downarrow	↓ ·	↓ ↓ ↓ ↓	↓ ↓ ↓ ↓	\downarrow \downarrow	+ $+$ $+$ $+$	↓ ↓ ↓ ↓
<i>∤</i> B1			11	1-09-00							 B2
Ы		Total H	lorizontal Pro	oduct Length	= 11-09-0	0					DZ
Reaction Summa	ary (Down / Uplif _{Live}	t) (Ibs) _{Dead}	s	inow	w	/ind		Roo	f Live		
B1, 2"	310 / 0	3458 / 0		208 / 0					LIVE		
B2, 3-1/2"	317 / 0	187 / 0	9	2/0							
Load Summary						Live	Dead	Snow	Wind	Roof Live	Tributary
Tag Description	Load Typ			End	Loc.	100%	90%	115%	1 60 %	125%	
0 Self-Weight	Unf. Lin.		00-00-00		Тор		11				00-00-00
1 SECOND	Unf. Area	. ,	00-00-00		Тор	40	10				01-04-00
2 CB02	Conc. Pt	. (Ibs) L	00-03-00	00-03-00	Front		3359	7300			n\a
Controls Summa		% Allov		Duration	Case	Loca	tion				
Pos. Moment	1783 ft-lbs	18.6%		115%	3	04-0					
End Shear	1458 lbs	17.5%		115%	2	00-0					
Total Load Deflection	· (-	-		n\a	3	05-0					
Live Load Deflection	L/999 (0.079")			n\a	6	05-0					
Max Defl.	0.127"	12.7%	1	n\a	3	05-0	5-00				
Span / Depth	18.9										
Bearing Support		Value	% Allow Support	% Allow Member	Materia						
B1 Hanger	2" x 5-1/4"	n\a	n\a	n∖a	Hange	r					
Cautions											
Concentrated side loa representative or Pro	fessional of Record.			Please con	sult a tech	nnical					
Header for the hange	r Hanger is a Triple 1	-3/4" x 7-1/4" l	_VL beam.								
Notes											
Design meets Code r	ninimum (L/240) Tota	I load deflection	on criteria.								
Design meets Code r											
Design meets arbitra											
Minimum bearing leng	gth for B1 is 2-11/16".										
Minimum bearing leng	gth for B2 is 1-1/2".										
Hanger Manufacturer	: Simpson Strong-Tie	. Inc.									

Hanger Manufacturer: Simpson Strong-I Design based on Dry Service Condition.

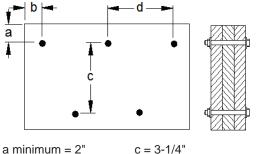
BC CALC® analysis is based on IBC 2018.

Calculations assume member is braced at all supports. See engineering report for the unbraced length.



Boise Cascade	Triple 1-3/4" x	7-1/4" VERSA-LAM® L	VL 2.1E 3100 SP	PASSED
ENGINEERED WOOD PRODUC		2B02 (Flush Beam)		
BC CALC® Memb	er Report	Dry 1 span No cant.	April	25, 2022 11:26:23
Build 8381				
Job name:	Katie & Alfonso Residence	File name:	170 Mount Vernon Street, West Ro	xbury
Address:	170 Mount Vernon Street	Description:		
City, State, Zip:	West Roxbury, MA, 02132	Specifier:		
Customer:	Derek Rubinoff	Designer:	David Guerrero	
Code reports:	ESR-1040	Company:	SSB Engineering	

Connection Diagram: Full Length of Member



b minimum = 2-1/2"d = 12"

Calculated Side Load = 0.0 lb/ft Bolts are assumed to be Grade A307 or Grade 2 or higher. Connectors are: 1/2 in. Staggered Through Bolt

Jym trasburg

Disclosure

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Boise Cascade	Quadruple 1-3	8/4" x 1 ⁻	1-1/4" VI	ERSA-LA	AM® L	VL 2.1	E 310	0 SP		P	ASSED
ENGINEERED WOOD PRODUCTS			2B03 (Fl	ush Beam)						
BC CALC® Member Build 8381	Report		•	an No cant	-				Apri	l 25, 202	22 11:26:23
Job name:	Katie & Alfonso Residence			File name	e: 1 [°]	70 Mount	Vernor	Street,	West Ro	oxbury	
Address:	170 Mount Vernon Street			Descriptio							
City, State, Zip:	West Roxbury, MA, 02132			Specifier:							
Customer:	Derek Rubinoff			Designer:		avid Gue					
Code reports:	ESR-1040			Company	: 5	SB Engir	ieering				
											3
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\bowtie											\sim
<u>/</u>											
B1			17-	03-00							B2
			rizontal Pro	duct Length	= 17-03-	00					
	ary (Down / Uplift) (Ik		6.		,	Win d		Dee	flive		
Bearing B1, 3-1/2"		ead 436 / 0		10w 75 / 0		Wind		ROO	f Live		
B2, 3-1/2"		469 / 0		799 / 0							
,											
Load Summary						Live	Dead	Snow	Wind	Roof Live	Tributary
Tag Description	Load Type	Ref.	Start	End	Loc.	100%	90%	115%	160%	125%	
0 Self-Weight	Unf. Lin. (lb/ft)		00-00-00	17-03-00	Тор Тор	40	23				00-00-00
1 SECOND 2 2B01	Unf. Area (lb/f Conc. Pt. (lbs)		00-00-00 01-00-00	17-03-00 01-00-00	Top Front	40 310	10 3458	7208			12-00-00 n∖a
2 2B01 3 2B02	Conc. Pt. (lbs) Conc. Pt. (lbs)		01-00-00	01-00-00	Front	307	3456 983	7208 966			n\a n\a
5 2002		IX.	01-00-00	01-00-00	TIOII	307	900	900			ma
Controls Summ	ary Value	% Allowa	able D	uration	Case	e Loca	tion				
Pos. Moment	23942 ft-lbs	62.3%	1	00%	1	09-0	1-01				
End Shear	11549 lbs	67.1%		15%	3	16-0					
Total Load Deflection	. (-)	88.1%		\a	1	08-1					
Live Load Deflection				10	4	08-0	7-08				
	· · · · · · · · · · · · · · · · · · ·	93.7%		\a	-						
Max Defl.	0.74"	93.7% 74.0%		\a \a	4	08-1					
Span / Depth	· · · · · · · · · · · · · · · · · · ·				-						
Span / Depth Cautions	0.74" 17.9	74.0%	n	\a	1	08-10					
Span / Depth Cautions Concentrated side lo	0.74"	74.0%	n	\a	1	08-10					
Span / Depth Cautions Concentrated side lo	0.74" 17.9 ad(s) 2,3 are closer than 18	74.0%	n	\a	1	08-10					
Span / Depth Cautions Concentrated side lo representative or Pro- Notes	0.74" 17.9 pad(s) 2,3 are closer than 18 ofessional of Record.	74.0%	n d of membe	\a	1	08-10					
Span / Depth Cautions Concentrated side lo representative or Pro Notes Design meets Code	0.74" 17.9 bad(s) 2,3 are closer than 18 ofessional of Record. minimum (L/240) Total load	74.0% " from end	n d of membe criteria.	\a	1	08-10					
Span / Depth Cautions Concentrated side lo representative or Pro Notes Design meets Code Design meets Code	0.74" 17.9 pad(s) 2,3 are closer than 18 ofessional of Record.	74.0% " from end deflection deflection	n d of membe criteria. criteria.	\a	1	08-10					

PASSED

Minimum bearing length for B2 is 2-13/16". Design based on Dry Service Condition.

BC CALC® analysis is based on IBC 2018.

Calculations assume member is fully braced.



Roise Caseado

Boise Cascade	Quadruple 1-3/4" x 11-1/4" VERSA-LAM® LVL 2.1E 3100 SP
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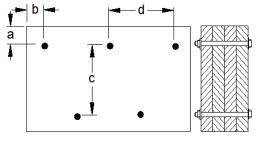
PASSED

2B03 (Flush Beam) Dry 1 1 anon | No cont

April 25 2022 11:26:23

BC CALC® Member Build 8381	er Report	Dry 1 span No cant.	April 25, 2022 11:2
Job name:	Katie & Alfonso Residence	File name:	170 Mount Vernon Street, West Roxbury
Address:	170 Mount Vernon Street	Description:	
City, State, Zip:	West Roxbury, MA, 02132	Specifier:	
Customer:	Derek Rubinoff	Designer:	David Guerrero
Code reports:	ESR-1040	Company:	SSB Engineering

Connection Diagram: Full Length of Member



a minimum = 2"c = 7 - 1/4" b minimum = 2-1/2"d = 24"

Calculated Side Load = 0.0 lb/ft Bolts are assumed to be Grade A307 or Grade 2 or higher. Connectors are: 1/2 in. Staggered Through Bolt

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	Bo	ise Cas	cade"				Do	ub	le 1	-3/4	" х	11	-1/4	" V	/EF	SA	-LA	M®) L\	۷L	2.11	E 310	0 8	SP				PA	SS	SED
	ENG	ANEEKED WY	DOD PRODU	CIS									21	B04	(Dr	op	Bear	n)												
BC Buil		LC® 3381	Mem	ber	Rep	oort									-	-	No ca									Apr	il 25,	2022	2 11	:26:23
		ime:			Kat	ie & A	lfons	so F	Reside	ence						Fil	e nan	ne:	1	170	Mou	nt Vern	on S	Street,	W	est R	oxbur	y		
Add	res	SS:			170) Mour	nt Ve	ernc	on Stro	eet						De	scrip	tion:												
City	, S	tate, 2	Zip:		We	st Rox	bury	/, M	IA, 02	132						Sp	ecifie	r:												
Cus						rek Ru		ff									esigne					ierrero								
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B1, B2,						326/					335 /					204 /														
DΖ,	3-	1/2				3207	0			10	5557	0			32	.04 /	0													
Loa	ad	Sun	nma	ry																	Live	Dea	d	Snow	,	Wind	Ro Liv		Tr	ibutary
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2		B03							c. Pt.	• •	(6.)	L		1-06·		04-	06-00		•		500	200								n∖a
3	V	VALL					I	rap	ezoid	al (Ib	o/ft)	L	00	0-00	-00	00	~~ ~	To	р			0								n∖a
4	v	VALL					т	ron	ezoid		(/f +)	D	00)-00-	00	03-	06-00		20			100 0								n∖a
4	v	VALL					1	Tap	ezoiu	ai (ir	<i>////)</i>	К	00	J-00·	-00	03	06-00	To	γ			100								II\d
																05-	-00-00)				100								
Со	nt	rols	Sun	nm	ary	Va	lue				%	Allow	able		D	urati	on		Cas	se	Loc	ation	_							
Pos	. N	1omer	nt			12	499	ft-lk	os		67.	4%			1	15%			2		04-	03-00								
	-	hear					23 lk				58.	4%			1	15%			2			09-04								
		load [٦		999 (-			n∖a				'n	∖a			2		-	00-12								
		oad D	eflec	tion			999 ((0.0)77")		n∖a					a			5			00-12								
Max							12"				n∖a				'n	∖a			2		04-	00-12								
Spa	n /	Dept	h			7.0)																							
No	te	S																												
	_		ts Co	de	mini	mum (L/24	10)	Total	load	defle	ectio	n crit	teria									-							
	-					mum (•																							
	-					1") Ma																								
Min	imu	um be	aring	ler	gth	for B1	is 1·	-1/2	2".																					
			-		-	for B2																								
			-		-	rvice C																								
			-			ased c																								
Cal	cul	ations	assu	ume	me	mber i	s bra	ace	d at a	ll su	opor	s. S	ee ei	ngin	eeri	ng re	eport	for th	e ur	nbra	iced I	ength.								



Boise Cascade		Double 1-3/4" x 11-1/4" VERSA-LAM® LVL 2.1E 3100 SP
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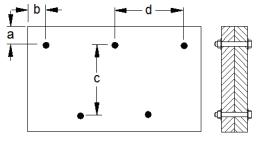


2B04 (Drop Beam) . . .

ril 25 2022 11:26:23

BC CALC® Memb Build 8381	er Report	Dry 1 span No cant.	April 25, 2022 11:2
Job name:	Katie & Alfonso Residence	File name:	170 Mount Vernon Street, West Roxbury
Address:	170 Mount Vernon Street	Description:	
City, State, Zip:	West Roxbury, MA, 02132	Specifier:	
Customer:	Derek Rubinoff	Designer:	David Guerrero
Code reports:	ESR-1040	Company:	SSB Engineering

Connection Diagram: Full Length of Member



a minimum = 2"c = 7 - 1/4" b minimum = 2-1/2"d = 24"

Calculated Side Load = 0.0 lb/ft Bolts are assumed to be Grade A307 or Grade 2 or higher. Connectors are: 1/2 in. Staggered Through Bolt



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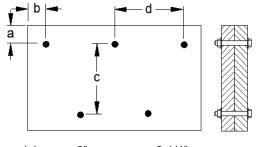
3	Boise Cascade"	Double	e 1-3/4" x 7∙				. 2.1E	3100	SP		P	ASSED
	CALC® Member d 8381	Report		•	u sh Beam an No cant					Apri	l 25, 202	2 11:26:23
Job Add	a 8381 name: ress: , State, Zip:	Katie & Alfonso Res 170 Mount Vernon S West Roxbury, MA,	Street		File name Descriptic Specifier:	on:	'0 Mount	t Vernor	n Street, '	West Ro	oxbury	
	tomer:	Derek Rubinoff	02102		Designer:		avid Gue	errero				
Cod	le reports:	ESR-1040			Company		SB Engir	neering				
	• • • •	••••••	• • • •	• • •		<u> </u>	<u> </u>	<u> </u>	<u> </u>	<u> </u>	• •	• •
\times	1											
∦— B1	1			07-	-06-00							⊀ B2
Ы			Total H	lorizontal Pro	duct Length	= 07-06-0	00					BZ
		ary (Down / Upli	, , ,	e.			Nin d		Dee	flive		
Bear B1.	3-1/2"	Live 1077 / 0	Dead 1058 / 0		now 13/0	V	Vind		K00	f Live		
	3-1/2"	1302 / 0	1191/0	-	79/0							
							Live	Dead	Snow	Wind	Roof	Tributary
LUG	ad Summary						LIVE	Deau	01101	wind		,, ,
Tag	Description	Load Ty			End	Loc.	100%	90%	115%	160%	Live 125%	-
Tag 0	Description Self-Weight	Unf. Lin	. (lb/ft) L	00-00-00	07-06-00	Тор		90% 7			Live	00-00-00
Tag 0 1	Description Self-Weight WALL	Unf. Lin Unf. Lin	. (lb/ft) L . (lb/ft) L	00-00-00 00-00-00	07-06-00 07-06-00	Top Front	100%	90% 7 100			Live	00-00-00 n\a
Tag 0 1 2	Description Self-Weight WALL SECOND	Unf. Lin Unf. Lin Unf. Are	. (Ib/ft) L . (Ib/ft) L ea (Ib/ft²) L	00-00-00 00-00-00 00-00-00	07-06-00 07-06-00 07-06-00	Top Front Top	100% 40	90% 7 100 10			Live	00-00-00 n\a 06-00-00
<u>Tag</u> 0 1 2 3	Description Self-Weight WALL SECOND CEILING	Unf. Lin Unf. Lin Unf. Are Unf. Are	. (Ib/ft) L . (Ib/ft) L ea (Ib/ft ²) L ea (Ib/ft ²) L	00-00-00 00-00-00 00-00-00 00-00-00	07-06-00 07-06-00 07-06-00 07-06-00	Top Front Top Top	100%	90% 7 100 10 5	115%		Live	00-00-00 n\a 06-00-00 03-06-00
Tag 0 1 2	Description Self-Weight WALL SECOND	Unf. Lin Unf. Lin Unf. Are Unf. Are	. (lb/ft) L . (lb/ft) L ea (lb/ft ²) L ea (lb/ft ²) L ea (lb/ft ²) L	00-00-00 00-00-00 00-00-00	07-06-00 07-06-00 07-06-00	Top Front Top	100% 40	90% 7 100 10			Live	00-00-00 n\a 06-00-00
Tag 0 1 2 3 4 5	Description Self-Weight WALL SECOND CEILING ROOF 2B02	Unf. Lin Unf. Lin Unf. Are Unf. Are Unf. Are Conc. P	. (lb/ft) L . (lb/ft) L ea (lb/ft ²) L ea (lb/ft ²) L ea (lb/ft ²) L tt. (lbs) R	00-00-00 00-00-00 00-00-00 00-00-00 00-00-	07-06-00 07-06-00 07-06-00 07-06-00 07-06-00 01-03-00	Top Front Top Top Front	100% 40 10 317	90% 7 100 10 5 15 187	<u>115%</u> 40		Live	00-00-00 n\a 06-00-00 03-06-00 06-00-00
Tag 0 1 2 3 4 5	Description Self-Weight WALL SECOND CEILING ROOF 2B02 ntrols Summ	Unf. Lin Unf. Lin Unf. Are Unf. Are Unf. Are Conc. P ary Value	. (Ib/ft) L . (Ib/ft) L ea (Ib/ft ²) L ea (Ib/ft ²) L ea (Ib/ft ²) L rt. (Ibs) R % Allow	00-00-00 00-00-00 00-00-00 00-00-00 01-03-00 vable E	07-06-00 07-06-00 07-06-00 07-06-00 07-06-00 01-03-00	Top Front Top Top Front Case	100% 40 10 317 Loca	90% 7 100 10 5 15 187 tion	<u>115%</u> 40		Live	00-00-00 n\a 06-00-00 03-06-00 06-00-00
Tag 0 1 2 3 4 5 Co Pos	Description Self-Weight WALL SECOND CEILING ROOF 2B02 ntrols Summ . Moment	Unf. Lin Unf. Lin Unf. Are Unf. Are Unf. Are Conc. P ary Value 4353 ft-lbs	. (Ib/ft) L . (Ib/ft) L ea (Ib/ft ²) L ea (Ib/ft ²) L ea (Ib/ft ²) L rt. (Ibs) R <u>% Allow</u> 45.2%	00-00-00 00-00-00 00-00-00 00-00-00 01-03-00 vable [2] 1	07-06-00 07-06-00 07-06-00 07-06-00 07-06-00 01-03-00 Duration	Top Front Top Top Front Case 3	100% 40 10 317 <u>Loca</u> 03-0	90% 7 100 10 5 15 187 tion 9-14	<u>115%</u> 40		Live	00-00-00 n\a 06-00-00 03-06-00 06-00-00
Tag 0 1 2 3 4 5 Co Pos End	Description Self-Weight WALL SECOND CEILING ROOF 2B02 ntrols Summ Moment Shear	Unf. Lin Unf. Lin Unf. Are Unf. Are Unf. Are Conc. P ary Value 4353 ft-lbs 2309 lbs	. (Ib/ft) L . (Ib/ft) L ea (Ib/ft ²) L ea (Ib/ft ²) L ea (Ib/ft ²) L t. (Ibs) R <u>% Allov</u> 45.2% 41.6%	00-00-00 00-00-00 00-00-00 00-00-00 01-03-00 vable E 1 1	07-06-00 07-06-00 07-06-00 07-06-00 07-06-00 01-03-00 01-03-00 0uration 15%	Top Front Top Top Front Case 3 3	40 10 317 Loca 03-0 06-0	90% 7 100 5 15 187 tion 9-14 7-04	<u>115%</u> 40		Live	00-00-00 n\a 06-00-00 03-06-00 06-00-00
Tag 0 1 2 3 4 5 Co Pos End Total	Description Self-Weight WALL SECOND CEILING ROOF 2B02 ntrols Summ . Moment	Unf. Lin Unf. Lin Unf. Are Unf. Are Unf. Are Conc. P ary Value 4353 ft-lbs 2309 lbs n L/452 (0.187 ⁴)	. (Ib/ft) L . (Ib/ft) L ea (Ib/ft ²) L ea (Ib/ft ²) L ea (Ib/ft ²) L t. (Ibs) R <u>% Allow</u> 45.2% 41.6% ') 53.0%	00-00-00 00-00-00 00-00-00 00-00-00 01-03-00 vable E 1 1	07-06-00 07-06-00 07-06-00 07-06-00 07-06-00 01-03-00 Duration	Top Front Top Top Front Case 3 3 3 3	100% 40 10 317 Loca 03-0 06-0 03-0	90% 7 100 5 15 187 tion 9-14 9-14 9-14	<u>115%</u> 40		Live	00-00-00 n\a 06-00-00 03-06-00 06-00-00
Tag 0 1 2 3 4 5 Co Pos End Tota Live	Description Self-Weight WALL SECOND CEILING ROOF 2B02 ntrols Summ Moment Shear al Load Deflection	Unf. Lin Unf. Lin Unf. Are Unf. Are Unf. Are Conc. P ary Value 4353 ft-lbs 2309 lbs	. (Ib/ft) L . (Ib/ft) L ea (Ib/ft ²) L ea (Ib/ft ²) L ea (Ib/ft ²) L t. (Ibs) R <u>% Allov</u> 45.2% 41.6%	00-00-00 00-00-00 00-00-00 00-00-00 01-03-00 vable E 1 1 n	07-06-00 07-06-00 07-06-00 07-06-00 07-06-00 01-03-00 01-03-00 0uration 15% 15%	Top Front Top Top Front Case 3 3	40 10 317 Loca 03-0 06-0	90% 7 100 5 15 187 tion 9-14 9-14 9-14 9-14	<u>115%</u> 40		Live	00-00-00 n\a 06-00-00 03-06-00 06-00-00
Tag 0 1 2 3 4 5 CO Pos End Tota Live Max	Description Self-Weight WALL SECOND CEILING ROOF 2B02 ntrols Summ Moment Shear al Load Deflection	Unf. Lin Unf. Lin Unf. Are Unf. Are Unf. Are Conc. P 4353 ft-lbs 2309 lbs n L/452 (0.187' L/999 (0.11")	. (Ib/ft) L . (Ib/ft) L ea (Ib/ft ²) L ea (Ib/ft ²) L ea (Ib/ft ²) L t. (Ibs) R <u>% Allov</u> 45.2% 41.6% ') 53.0% n\a	00-00-00 00-00-00 00-00-00 00-00-00 01-03-00 vable E 1 1 n	07-06-00 07-06-00 07-06-00 07-06-00 07-06-00 01-03-00 0uration 15% 15% \a	Top Front Top Top Front Case 3 3 3 6	100% 40 10 317 Loca 03-0 06-0 03-0 03-0 03-0	90% 7 100 5 15 187 tion 9-14 9-14 9-14 9-14	<u>115%</u> 40		Live	00-00-00 n\a 06-00-00 03-06-00 06-00-00
Tag 0 1 2 3 4 5 Co Pos End Tota Live Max Spa	Description Self-Weight WALL SECOND CEILING ROOF 2B02 ntrols Summ Moment Shear al Load Deflection Load Deflection Defl. n / Depth	Unf. Lin Unf. Lin Unf. Are Unf. Are Unf. Are Conc. P 4353 ft-lbs 2309 lbs L/452 (0.187' L/999 (0.11") 0.187"	. (Ib/ft) L . (Ib/ft) L ea (Ib/ft ²) L ea (Ib/ft ²) L ea (Ib/ft ²) L t. (Ibs) R <u>% Allov</u> 45.2% 41.6% ') 53.0% n\a	00-00-00 00-00-00 00-00-00 00-00-00 01-03-00 vable E 1 1 n	07-06-00 07-06-00 07-06-00 07-06-00 07-06-00 01-03-00 0uration 15% 15% \a	Top Front Top Top Front Case 3 3 3 6	100% 40 10 317 Loca 03-0 06-0 03-0 03-0 03-0	90% 7 100 5 15 187 tion 9-14 9-14 9-14 9-14	<u>115%</u> 40		Live	00-00-00 n\a 06-00-00 03-06-00 06-00-00
Tag 0 1 2 3 4 5 Co Poss End Tota Live Max Spa	Description Self-Weight WALL SECOND CEILING ROOF 2B02 ntrols Summ Moment Shear al Load Deflection Load Deflection Defl. n / Depth	Unf. Lin Unf. Lin Unf. Are Unf. Are Unf. Are Conc. P 4353 ft-lbs 2309 lbs L/452 (0.187' L/999 (0.11'') 0.187'' 11.7	. (Ib/ft) L . (Ib/ft) L ea (Ib/ft ²) L ea (Ib/ft ²) L ea (Ib/ft ²) L t. (Ibs) R <u>% Allov</u> 45.2% 41.6% ') 53.0% n\a 18.7%	00-00-00 00-00-00 00-00-00 00-00-00 01-03-00 vable E 1 1 n n	07-06-00 07-06-00 07-06-00 07-06-00 07-06-00 01-03-00 0uration 15% 15% \a	Top Front Top Top Front Case 3 3 3 6	100% 40 10 317 Loca 03-0 06-0 03-0 03-0 03-0	90% 7 100 5 15 187 tion 9-14 9-14 9-14 9-14	<u>115%</u> 40		Live	00-00-00 n\a 06-00-00 03-06-00 06-00-00
Tag 0 1 2 3 4 5 Co Pos End Tota Live Max Spa No Des	Description Self-Weight WALL SECOND CEILING ROOF 2B02 ntrols Summ Moment Shear al Load Deflection bead Deflection coefl. n / Depth tes ign meets Code	Unf. Lin Unf. Lin Unf. Are Unf. Are Unf. Are Conc. P 4353 ft-lbs 2309 lbs L/452 (0.187' L/999 (0.11'') 0.187'' 11.7	. (lb/ft) L . (lb/ft) L ea (lb/ft ²) L ea (lb/ft ²) L ea (lb/ft ²) L t. (lbs) R % Allov 45.2% 41.6% ') 53.0% n\a 18.7% al load deflection	00-00-00 00-00-00 00-00-00 00-00-00 01-03-00 vable E 1 1 n n n	07-06-00 07-06-00 07-06-00 07-06-00 07-06-00 01-03-00 0uration 15% 15% \a	Top Front Top Top Front Case 3 3 3 6	100% 40 10 317 Loca 03-0 06-0 03-0 03-0 03-0	90% 7 100 5 15 187 tion 9-14 9-14 9-14 9-14	<u>115%</u> 40		Live	00-00-00 n\a 06-00-00 03-06-00 06-00-00
Tag 0 1 2 3 4 5 Co Pos End Tota Spa No Des Des Des	Description Self-Weight WALL SECOND CEILING ROOF 2B02 ntrols Summ Moment Shear al Load Deflection be Load Deflection c Defl. n / Depth tes ign meets Code ign meets Code	Unf. Lin Unf. Lin Unf. Are Unf. Are Unf. Are Conc. P 4353 ft-lbs 2309 lbs L/452 (0.187' L/999 (0.11'') 0.187'' 11.7	. (lb/ft) L . (lb/ft) L ea (lb/ft ²) L ea (lb/ft ²) L ea (lb/ft ²) L t. (lbs) R % Allov 45.2% 41.6% ') 53.0% n\a 18.7% al load deflection	00-00-00 00-00-00 00-00-00 00-00-00 01-03-00 vable E 1 1 1 n n n	07-06-00 07-06-00 07-06-00 07-06-00 07-06-00 01-03-00 0uration 15% 15% \a	Top Front Top Top Front Case 3 3 3 6	100% 40 10 317 Loca 03-0 06-0 03-0 03-0 03-0	90% 7 100 5 15 187 tion 9-14 9-14 9-14 9-14	<u>115%</u> 40		Live	00-00-00 n\a 06-00-00 03-06-00 06-00-00
Tag 0 1 2 3 4 5 Co Pos End Tota Live Max Spa Des Des Des Des	Description Self-Weight WALL SECOND CEILING ROOF 2B02 ntrols Summ . Moment Shear al Load Deflection c Defl. n / Depth tes ign meets Code ign meets arbitra	Unf. Lin Unf. Lin Unf. Are Unf. Are Unf. Are Conc. P 4353 ft-lbs 2309 lbs L/452 (0.187' L/999 (0.11') 0.187'' 11.7 minimum (L/240) Tota minimum (L/260) Live ry (1'') Maximum Tota	. (lb/ft) L . (lb/ft) L ea (lb/ft ²) L ea (lb/ft ²) L ea (lb/ft ²) L t. (lbs) R % Allov 45.2% 41.6% ') 53.0% n\a 18.7% al load deflection	00-00-00 00-00-00 00-00-00 00-00-00 01-03-00 vable E 1 1 1 n n n	07-06-00 07-06-00 07-06-00 07-06-00 07-06-00 01-03-00 0uration 15% 15% \a	Top Front Top Top Front Case 3 3 3 6	100% 40 10 317 Loca 03-0 06-0 03-0 03-0 03-0	90% 7 100 5 15 187 tion 9-14 9-14 9-14 9-14	<u>115%</u> 40		Live	00-00-00 n\a 06-00-00 03-06-00 06-00-00
Tag 0 1 2 3 4 5 Co Pos End Tota Live Max Spa Des Des Des Mini	Description Self-Weight WALL SECOND CEILING ROOF 2B02 ntrols Summ Moment Shear al Load Deflection Load Deflection c Defl. n / Depth tes ign meets Code ign meets arbitra imum bearing ler	Unf. Lin Unf. Lin Unf. Are Unf. Are Unf. Are Conc. P 4353 ft-lbs 2309 lbs L/452 (0.187' L/999 (0.11'') 0.187'' 11.7 minimum (L/240) Tota minimum (L/240) Tota minimum (L/360) Live ry (1'') Maximum Tota gth for B1 is 1-1/2''.	. (lb/ft) L . (lb/ft) L ea (lb/ft ²) L ea (lb/ft ²) L ea (lb/ft ²) L t. (lbs) R % Allov 45.2% 41.6% ') 53.0% n\a 18.7% al load deflection	00-00-00 00-00-00 00-00-00 00-00-00 01-03-00 vable E 1 1 1 n n n	07-06-00 07-06-00 07-06-00 07-06-00 07-06-00 01-03-00 0uration 15% 15% \a	Top Front Top Top Front Case 3 3 3 6	100% 40 10 317 Loca 03-0 06-0 03-0 03-0 03-0	90% 7 100 5 15 187 tion 9-14 9-14 9-14 9-14	<u>115%</u> 40		Live	00-00-00 n\a 06-00-00 03-06-00 06-00-00
Tag 0 1 2 3 4 5 Co Pos End Tota Live Max Spa Des Des Des Mini Mini	Description Self-Weight WALL SECOND CEILING ROOF 2B02 ntrols Summ Moment Shear al Load Deflection Load Deflection Load Deflection Defl. n / Depth tes ign meets Code ign meets arbitra imum bearing len imum bearing len	Unf. Lin Unf. Lin Unf. Are Unf. Are Unf. Are Conc. P 4353 ft-lbs 2309 lbs L/452 (0.187' L/999 (0.11'') 0.187'' 11.7 minimum (L/240) Tote minimum (L/360) Live ry (1'') Maximum Tote gth for B1 is 1-1/2''. gth for B2 is 1-1/2''.	. (lb/ft) L . (lb/ft) L ea (lb/ft ²) L ea (lb/ft ²) L ea (lb/ft ²) L t. (lbs) R % Allov 45.2% 41.6% ') 53.0% n\a 18.7% al load deflection	00-00-00 00-00-00 00-00-00 00-00-00 01-03-00 vable E 1 1 1 n n n	07-06-00 07-06-00 07-06-00 07-06-00 07-06-00 01-03-00 0uration 15% 15% \a	Top Front Top Top Front Case 3 3 3 6	100% 40 10 317 Loca 03-0 06-0 03-0 03-0 03-0	90% 7 100 5 15 187 tion 9-14 9-14 9-14 9-14	<u>115%</u> 40		Live	00-00-00 n\a 06-00-00 03-06-00 06-00-00
Tag 0 1 2 3 4 5 Co Pos End Tota Live Max Spa Des Des Des Mini Des	Description Self-Weight WALL SECOND CEILING ROOF 2B02 ntrols Summ Moment Shear al Load Deflection Cod Deflection Defl. n / Depth tes ign meets Code ign meets arbitra imum bearing ler ign based on Dry	Unf. Lin Unf. Lin Unf. Are Unf. Are Unf. Are Conc. P 4353 ft-lbs 2309 lbs L/452 (0.187' L/999 (0.11'') 0.187'' 11.7 minimum (L/240) Tota minimum (L/240) Tota minimum (L/360) Live ry (1'') Maximum Tota gth for B1 is 1-1/2''.	. (lb/ft) L . (lb/ft) L ea (lb/ft ²) L ea (lb/ft ²) L ea (lb/ft ²) L ea (lb/ft ²) L t. (lbs) R <u>% Allow</u> 45.2% 41.6% ') 53.0% n\a 18.7% al load deflection al load deflection	00-00-00 00-00-00 00-00-00 00-00-00 01-03-00 vable E 1 1 1 n n n	07-06-00 07-06-00 07-06-00 07-06-00 07-06-00 01-03-00 0uration 15% 15% \a	Top Front Top Top Front Case 3 3 3 6	100% 40 10 317 Loca 03-0 06-0 03-0 03-0 03-0	90% 7 100 5 15 187 tion 9-14 9-14 9-14 9-14	<u>115%</u> 40		Live	00-00-00 n\a 06-00-00 03-06-00 06-00-00



Boise Cascade	Double 1-3/4" x	7-1/4" VERSA-LAM® L	VL 2.1E 3100 SP PASSED
CHOINEERED 110007R00	Judie	2B05 (Flush Beam)	
BC CALC® Men	nber Report	Dry 1 span No cant.	April 25, 2022 11:26:23
Build 8381			
Job name:	Katie & Alfonso Residence	File name:	170 Mount Vernon Street, West Roxbury
Address:	170 Mount Vernon Street	Description:	

Job name:	Katie & Alfonso Residence	File name:	170 Mount Vernon Street, West Roxbury
Address:	170 Mount Vernon Street	Description:	
City, State, Zip:	West Roxbury, MA, 02132	Specifier:	
Customer:	Derek Rubinoff	Designer:	David Guerrero
Code reports:	ESR-1040	Company:	SSB Engineering

Connection Diagram: Full Length of Member



a minimum = 2" c = 3-1/4" b minimum = 2-1/2" d = 24"

Calculated Side Load = 252.0 lb/ftBolts are assumed to be Grade A307 or Grade 2 or higher. Connectors are: 1/2 in. Staggered Through Bolt

> TARA LYNN STRASBURG CIVIL NO. 5575 STRASBURG STRASB

Disclosure

Use of the Boise Cascade Software is subject to the terms of the End User License Agreement (EULA). Completeness and accuracy of input must be reviewed and verified by a qualified engineer or other appropriate expert to assure its adequacy, prior to anyone relying on such output as evidence of suitability for a particular application. The output here is based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call (800)232-0788 before installation.

BC CALC®, BC FRAMER® , AJSTM, ALLJOIST® , BC RIM BOARDTM, BCI® , BOISE GLULAMTM, BC FloorValue® , VERSA-LAM®, VERSA-RIM PLUS® ,

	Boise Ca	ascade"							т	•			SPF Bean								P	ASSED
		Membe	r Re	port						Dry	1 sp	an N	No car	nt.						Apr	il 25, 202	22 11:26:23
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End S		Deflectio			624 lb			55.3%	%			100%			1		09-04					
		Deflection				(0.007")		n∖a n∖a				n∖a n∖a			3 6		03-05 03-05					
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Bea	ring	Suppo	rts	Dim.	(LxW)		Valu	е	-	Allov			Allow	Ма	teria	al						
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B2		Hanger		2" x 4	4-1/2"		n\a		n	∖a		n∖a		Ha	inge	er						
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BC CALC®, BC FRAMER®, AJS™, ALLJOIST®, BC RIM BOARD™, BCI®, BOISE GLULAM™, BC FloorValue®, VERSA-LAM®, VERSA-RIM PLUS®,

Boise Cascade				S	Single 2	x 8 SPF	#2					PA	SSED
ENVIRENCE TOWNTRANGE					2J01	(Joist)							
BC CALC® Member Build 8381	Report	Dry	1 spar	n No (cant. 16 O	CS Repeti	tive Gl	ued & na	iled		Apri	l 25, 2022	13:19:05
Job name:	Katie & A	lfonso Residen	се			File name	e: 1	70 Moun	t Vernor	n Street, '	West Ro	oxbury	
Address:	170 Mour	nt Vernon Stree	t			Descriptio						,	
City, State, Zip:	West Rox	bury, MA, 0213	32			Specifier:							
Customer:	Derek Ru	-				Designer		avid Gue	errero				
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B1					03-	-00-00							B2
51			Т	otal Ho	rizontal Pro	duct Length	= 03-00-	00					DL
Reaction Summ	narv (Dov	wn / Uplift) ((lbs)			_							
Bearing	Live	1 7	Dead		Si	now		Wind		Roo	f Live		
B1, 3-1/2"	80 / 0		163 /			0 / 0							
B2, 3-1/2"	80 / 0		163 /	0	20	0 / 0							
Load Summary								Live	Dead	Snow	Wind	Roof	OCS
-												Live	
TagDescription1SECOND		Load Type Unf. Area (It	∩/ft2)	Ref.	Start 00-00-00	End 03-00-00	Loc. Top	100% 40	90% 10	115%	160%	125%	16
2 WALL		Conc. Lin. (I		L	01-06-00	01-06-00	Тор	40	100				16
3 ROOF		Conc. Lin. (I Conc. Lin. (I	,	L	01-06-00	01-06-00	Тор		115	300			16
	OIN (_	-	000			10
Controls Summ		lue 7 ft-lbs		Allowa		ouration 15%	<u>Case</u> 2	<u>Loca</u> 01-0					
End Shear		2 lbs		1.2%		15%	2	01-0					
Total Load Deflectio		999 (0.006")	n\			\a	2	01-0					
Live Load Deflection		999 (0.000) 999 (0.004")	n\			\a	5	01-0					
Max Defl.		0.004) 006"	n\			\a	2	01-0					
Span / Depth	4.2		111	a			2	01-0	0-00				
opan / Depin	7.2	-											
BC FloorValue®	Summa	ary											
BC FloorValue®:					,	Glue + Nai	I						
		nhanced Premiu	ım Sı	ubfloor	Rating: Pre	emium							
Controlling Location	: 01-08-12												
Notoo										<u>Disc</u> l	osure		
Notes	noining	(1/040) Tetel	od -!- (oritoria					Use of t	he Boise	Cascade S	
Design meets Code												ms of the E	nd User
Design meets User												ent (EULA). nd accuracy	of input
Design meets arbitra			au uei	lection	cillena.					must be	reviewe	d and verifie	ed by a
Minimum bearing ler	•											er or other a	
Minimum bearing ler	-		abaath	المع مار			hor					its adequac n such outp	
Composite El value			sneatr	ning git	led and hai	led to memi	ber.					bility for a p	
Design based on Dr	-				a with the N		line it e el te		4	applicat	ion. The	output here	is based on
The analysis of solic												cepted desig	
shown above. All oth connections, installa												nalysis meth ise Cascade	
professional of reco			5.001			Pointionity C		.,	9··	enginee	red wood	d products n	nust be in
BC CALC® analysis		on IBC 2018.								accorda	nce with	current Inst	allation
Calculations assume												able buildin: n Guide or a	g codes. To
		, <u></u>				and a state of the						e call (800)2	
						ALTH OF MASSIC					nstallatio		



BC CALC®, BC FRAMER® , AJS™, ALLJOIST® , BC RIM BOARD™, BCI® , BOISE GLULAM™, BC FloorValue® , VERSA-LAM®, VERSA-RIM PLUS® ,

*	Boise Cascade	
J D	ENGINEERED WOOD PRODUCTS	

BC CALC® Member Report

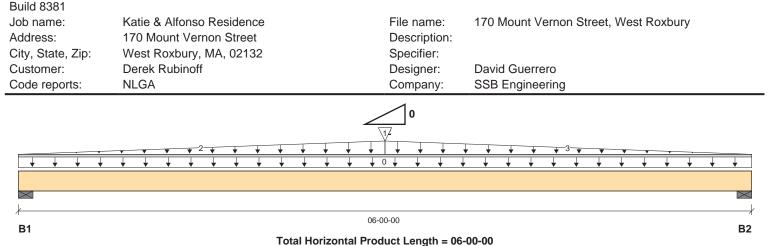
Triple 2 x 8 SPF #2



Dry | 1 span | No cant.

April 25, 2022 11:26:23

PASSED



Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead	Snow	Wind	Roof Live	
B1, 3-1/2"		362 / 0	632 / 0			
B2, 3-1/2"		363 / 0	632 / 0			

Lo	ad Summary						Live	Dead	Snow	Wind	Roof Live	Tributary
Tag	Description	Load Type	Ref.	Start	End	Loc.	100%	90%	115%	1 60 %	125%	
0	Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	06-00-00	Тор		7				00-00-00
1	RB01	Conc. Pt. (lbs)	L	03-00-00	03-00-00	Тор		530	1264			n\a
2	WALL	Trapezoidal (lb/ft)	L	00-00-00		Тор		0				n\a
					03-00-00			50				
3	WALL	Trapezoidal (lb/ft)	R	00-00-00		Тор		0				n\a
					03-00-00	·		50				

Controls Summary	Value	% Allowable	Duration	Case	Location
Pos. Moment	2646 ft-lbs	70.5%	115%	4	03-00-00
End Shear	988 lbs	29.2%	115%	4	05-01-04
Total Load Deflection	L/999 (0.059")	n\a	n\a	4	03-00-00
Live Load Deflection	L/999 (0.039")	n\a	n\a	5	03-00-00
Max Defl.	0.059"	n\a	n\a	4	03-00-00
Span / Depth	9.2				

Cautions

For roof members with slope (1/4)/12 or less final design must ensure that ponding instability will not occur.

For roof members with slope (1/2)/12 or less final design must account for Rain-on-Snow surcharge load.

Notes

Design meets Code minimum (L/180) Total load deflection criteria.

Design meets Code minimum (L/240) Live load deflection criteria.

Design meets arbitrary (1") Maximum Total load deflection criteria.

Minimum bearing length for B1 is 1-1/2".

Minimum bearing length for B2 is 1-1/2".

Design based on Dry Service Condition.

The analysis of solid sawn wood members is in accordance with the NDS and is limited to the output shown above. All other support and design for these products, including but not limited to notching, connections, installation, and engineer/architect certification is the responsibility of the project's design professional of record.

BC CALC® analysis is based on IBC 2018.

Calculations assume member is fully braced.



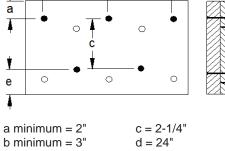
Disclosure

Use of the Boise Cascade Software is subject to the terms of the End User License Agreement (EULA). Completeness and accuracy of input must be reviewed and verified by a qualified engineer or other appropriate expert to assure its adequacy, prior to anyone relying on such output as evidence of suitability for a particular application. The output here is based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call (800)232-0788 before installation.

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Tax Lynn Strassburg

				CB02 (FI	oor Beam)						
BC CALC® Member F Build 8381	Report			Dry 1 spa	an No can	t.				April	25, 202	2 11:26:23
	Katie & Alfonso Residence File name: 170 Mount Vernor						t Vernor	Street.	West Ro	xburv		
		Vernon Street			Descriptio		o moun		, ou oou,		, and any	
		ury, MA, 02132			Specifier:							
	Derek Rubi				Designer		vid Gue	errero				
Code reports:	ESR-1040				Company		B Engir	neering				
		1/										
	+ +	+ + + +	+ +	+ + +	0 🖌 🗍 .	+ + +		+ +	+ +	+ +	+ +	+ +
B1				03-	02-00							B2
51			Total Ho	orizontal Pro	duct Length	= 03-02-0	0					DL
Reaction Summa	ry (Dow	n / Uplift) (lt	os)									
Bearing	Live		ead		now	V	/ind		Roo	of Live		
31, 5-1/2"			359/0		300 / 0							
32, 3-1/2"		5	58 / 0	11	182/0							
_oad Summary							Live	Dead	Snow	Wind	Roof	Tributar
ag Description		Load Type	Ref.	Start	End	Loc.	100%	90%	115%	160%	Live 125%	
) Self-Weight		Unf. Lin. (lb/ft)		00-00-00	03-02-00	Тор	10070	11	11070	10070	12070	00-00-0
RB03		Conc. Pt. (lbs)		00-09-00	00-09-00	Тор		3882	8482			n\a
Controls Summa	ry Valu	e	% Allowa	able D	uration	Case	Loca	tion				
Pos. Moment		6 ft-lbs	26.6%		15%	1	00-0					
Ind Shear		2 lbs	51.1%		15%	1	01-0					
otal Load Deflection		99 (0.017")	n\a		\a	1	01-0					
ive Load Deflection		9 (0.012")	n∖a		\a	2	01-0	-				
Max Defl.	0.01	7"	n\a	n	\a	1	01-0	1-11				
Span / Depth	4.2											
Notes												
Design meets Code m												
Design meets Code m		,										
Design meets arbitrar			deflection	o criteria.								
Ainimum bearing leng												
Minimum bearing leng Design based on Dry												
BC CALC® analysis is												
Calculations assume			3-02-00, B	ottom: 03-02	2-00.							
	_			_								
Sammaatian Di												
Connection Diag	ram: Fu	I Length of	Member	ſ								



e minimum = 3"



		333
(23)	Boise Cascade	
V	ENGINEERED WOOD PRODUCTS	

BC CALC® Member Report

Triple 1-3/4" x 7-1/4" VERSA-LAM® LVL 2.1E 3100 SP



CB02 (Floor Beam)

Dry | 1 span | No cant.

April 25, 2022 11:26:23

Build 8381	
Job name:	Katie & Alfonso Residence
Address:	170 Mount Vernon Street
City, State, Zip:	West Roxbury, MA, 02132
Customer:	Derek Rubinoff
Code reports:	ESR-1040

File name:170 Mount Vernon Street, West RoxburyDescription:Specifier:Designer:David GuerreroCompany:SSB Engineering

Connection Diagram: Full Length of Member

Calculated Side Load = 0.0 lb/ft Nailing applies to both sides of the member Connectors are: 3-1/4 in. Pneumatic Gun Nails



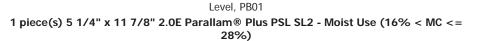
Disclosure

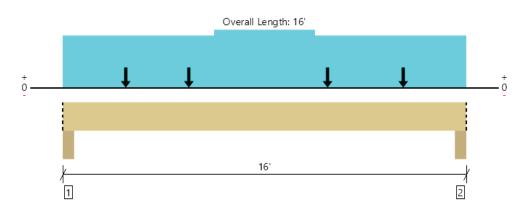
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MEMBER REPORT





All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	3159 @ 4"	9745 (5.50")	Passed (32%)		1.0 D + 0.75 L + 0.75 S (All Spans)
Shear (lbs)	2755 @ 1' 5 3/8"	8196	Passed (34%)	1.00	1.0 D + 1.0 L (All Spans)
Moment (Ft-Ibs)	10884 @ 8' 1 3/4"	18808	Passed (58%)	1.00	1.0 D + 1.0 L (All Spans)
Live Load Defl. (in)	0.253 @ 7' 11 15/16"	0.767	Passed (L/727)		1.0 D + 0.75 L + 0.75 S (All Spans)
Total Load Defl. (in)	0.693 @ 7' 11 15/16"	1.022	Passed (L/266)		1.0 D + 0.75 L + 0.75 S (All Spans)

• Deflection criteria: LL (L/240) and TL (L/180).

· Allowed moment does not reflect the adjustment for the beam stability factor.

	Bearing Length			L	oads to Sup			
Supports	Total	Available	Required	Dead	Floor Live	Snow	Total	Accessories
1 - Column - SPF	5.50"	5.50"	1.78"	1481	1448	789	3718	Blocking
2 - Column - SPF	5.50"	5.50"	1.75"	1454	1416	775	3645	Blocking
Blocking Panels are assumed to carry no loads applied directly above them and the full load is applied to the member being designed.								

Lateral Bracing	Bracing Intervals	Comments					
Top Edge (Lu)	16' o/c						
Bottom Edge (Lu)	16' o/c						
Maximum allowable burging intervals beard on analised band							

•Maximum allowable bracing intervals based on applied load.

			Dead	Floor Live	Snow	
Vertical Loads	Location (Side)	Tributary Width	(0.90)	(1.00)	(1.15)	Comments
0 - Self Weight (PLF)	0 to 16'	N/A	22.6			
1 - Uniform (PSF)	0 to 16' (Top)	1' 6"	10.0	40.0	-	SECOND
2 - Point (lb)	2' 6" (Top)	N/A	416	476	211	2B07
3 - Point (lb)	13' 6" (Top)	N/A	416	476	211	2B07
4 - Point (lb)	5' (Top)	N/A	416	476	211	2B07 BELOW CB01
5 - Point (lb)	10' 6" (Top)	N/A	416	476	211	2B07 BELOW CB01
6 - Uniform (PSF)	0 to 6' (Top)	1' 6"	15.0	-	40.0	ROOF
7 - Uniform (PSF)	10' to 16' (Top)	1' 6"	15.0	-	40.0	ROOF
8 - Uniform (PLF)	6' to 10' (Top)	N/A	100.0	-	-	DORMER WALL

Weyerhaeuser Notes

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The product application, input design loads, dimensions and support information have been provided by ForteWEB Software Operator

ForteWEB Software Operator William Green SSB Engineering (781) 689-6133 william@ssbengineering.com Job Notes Ceiling beam without lateral bracing



Weyerhaeuser

4/25/2022 1:53:14 PM UTC ForteWEB v3.2, Engine: V8.2.0.17, Data: V8.1.0.16 File Name: 170 Mount Vernon Street, Boston MA Page 1 / 1

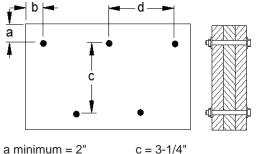
System : Roof Member Type : Drop Beam Building Use : Residential Building Code : IBC 2018 Design Methodology : ASD Member Pitch : 0/12

BC CALC® Member	Report	Drv l	1 span I		(Rafter) 6 OCS Re	petitive	8.37/12			Apri	25, 202	2 11:26:23
Build 8381	toport	2.91	i opan j		0000110	pounto	10.01712			7.01	20, 202	
Job name:	Katie & Alf	onso Residence			File name	ə: 1	70 Mount	Verno	n Street,	West Ro	oxbury	
	170 Mount	Vernon Street			Description	on:						
•		oury, MA, 02132			Specifier:							
	Derek Rub				Designer		David Gue					
Code reports:	ESR-1040				Company	/: 8	SSB Engir	neering				
				3	1							
+ + + +	5-	•		`	8.37					F	7	
	,		$ \begin{array}{c} \downarrow \\ \downarrow \\ \downarrow \\ \end{array} $	 ↓ ↓ ↓	1 ↓ ↓	↓ ↓	↓ ↓ ·	↓ ↓	+ +	↓ ↓	\downarrow \downarrow \downarrow	$\begin{array}{c} 4 \downarrow \downarrow \\ \downarrow \downarrow \\ \end{array}$
		· · · · ·			· ·	<u> </u>				<u> </u>		
<u>/</u>				13	-00-00							/
B1			Total Ho	orizontal Pro	duct Length	= 13-00	-00					B2
Reaction Summa	ary (Dow	/n / Uplift) (lb	s)		· ·							
Bearing	Live	De			now		Wind		Roo	f Live		
B1, 3-1/2"			6/0		398 / 0							
B2, 3-1/2"		95	8/0	10	889 / 0							
Load Summary							Live	Dead	Snow	Wind	Roof	Tributar
Tag Description		Load Type	Ref.	Start	End	Loc.	100%	90%	115%	160%	Live 125%	
1 ROOF		Unf. Area (lb/ft ²		00-00-00	13-00-00	Тор	10070	15	40	100 /0	12370	01-04-0
2 RB02		Conc. Pt. (lbs)	Ŕ	01-08-00		Тор		556	1116			n\a
3 VB01		Conc. Pt. (lbs)	R	07-07-00	07-07-00	Тор		302	582			n\a
4 ROOF		Unf. Lin. (lb/ft)	R	00-00-00	01-08-00	Тор		39	105			n\a
5 WALL		Trapezoidal (Ib	/ft) R	07-07-00		Тор		0				n\a
6 ROOF		Unf. Lin. (lb/ft)	R	07-07-00	13-00-00 13-00-00	Top		100 50	130			n\a
			i c	01 01 00	10 00 00	rop		00	100			
Controls Summa			% Allowa		Duration	Cas						
Pos. Moment	-	4 ft-lbs	46.9%		15%	4	05-0					
End Shear Total Load Deflection		2 lbs 19 (0.839")	33.3% 82.3%		∣15% ì\a	4 4	12-0 06-0					
Live Load Deflection		40 (0.54")	70.6%		n\a	5	06-0					
Max Defl.	0.83		83.9%		n\a	4	06-0					
Span / Depth	20.8											
Slope and Cut Lope Plumb Cut with Hang		p plate 8.37/1			oriz. Length 3-00-00		oduct Leng -03-04	lth				
Piump Cul with Hand		p plate 6.37/1	2 0-13	/10 1.	3-00-00	10-	-03-04					
J												
C		/180) Total load	deflection	critoria								
Notes	unimum (I											
Notes Design meets Code r		,		ontoniai								
Notes Design meets Code r Design meets Code r	ninimum (L	/240) Live load d		criteria.								
Notes Design meets Code r Design meets Code r Design meets arbitra	ninimum (L y (1") Maxi	/240) Live load d imum Total load		criteria.								
Notes Design meets Code r Design meets Code r Design meets arbitrar Minimum bearing leng	ninimum (L y (1") Maxi gth for B1 is	/240) Live load d imum Total load o s 1-1/2".		criteria.								
Notes Design meets Code r Design meets Code r Design meets arbitra Minimum bearing leng Minimum bearing leng	ninimum (L y (1") Maxi gth for B1 is gth for B2 is	/240) Live load d imum Total load d s 1-1/2". s 1-1/2".		criteria.								
Notes Design meets Code r Design meets Code r Design meets arbitrat Minimum bearing leng Minimum bearing leng Design based on Dry BC CALC® analysis i Calculations assume	ninimum (L y (1") Maxi gth for B1 is gth for B2 is Service Co s based on	/240) Live load d imum Total load d s 1-1/2". s 1-1/2". ondition. n IBC 2018.		criteria.								



Boise Cascade	Triple 1-3/4 "	x 7-1/4" VERSA-LAM® L\	/L 2.1E 3100 SP	PASSED
Envirence (loss neevels		R01 (Rafter)		
BC CALC® Member	Report Dry 1	span No cant. 16 OCS Repetitiv	/e 8.37/12	April 25, 2022 11:26:23
Build 8381				
Job name:	Katie & Alfonso Residence	File name:	170 Mount Vernon Street	t, West Roxbury
Address:	170 Mount Vernon Street	Description:		
City, State, Zip:	West Roxbury, MA, 02132	Specifier:		
Customer:	Derek Rubinoff	Designer:	David Guerrero	
Code reports:	ESR-1040	Company:	SSB Engineering	

Connection Diagram: Full Length of Member



a minimum = 2 C = 3-1/2b minimum = 2-1/2" d = 24"

Calculated Side Load = 0.0 lb/ftBolts are assumed to be Grade A307 or Grade 2 or higher. Connectors are: 1/2 in. Staggered Through Bolt



Disclosure

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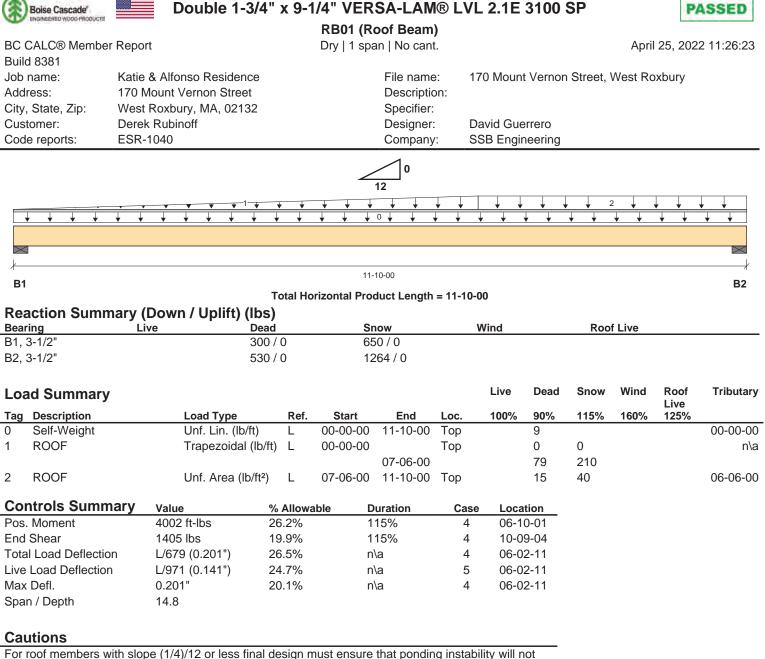
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Boise Cascade					:	Single			⁼ #2						P	ASSED
BC CALC® Member	Repor	t		Dry	/ 1 span			after) 6 OCS	Repeti	tive	4/12			Apri	l 25, 202	22 11:26:23
Build 8381 Job name: Address:		& Alfonso Iount Ver						File nar Descrip				t Vernoi AFTER	n Street, '	West Ro	oxbury	
City, State, Zip: Customer:		Roxbury, Rubinof						vid Gue	errero							
Code reports:	NLGA							Compa	ny:	SS	B Engir	neering				
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<u>}</u>							13-00	-00								/
B1						orizontal	Produ	ict Leng	th = 13-	00-00	0					B2
Reaction Summ Bearing	nary (I Liv		Upli	<i>,</i> ,	ead		Sno	w		W	/ind		Roo	f Live		
B1, 3-1/2" B2, 3-1/2"					37 / 0 37 / 0		347 347									
Load Summary											Live	Dead	Snow	Wind	Roof Live	Tributary
TagDescription1ROOF			ad Ty	be a (lb/ft	2) L	Star 00-00-		End 13-00-0	Loc 0 Top		100%	90% 15	115% 40	160%	125%	01-04-00
		01	II. AIC		-) L	00-00-	.00	13-00-0	o rop			15	40			01-04-00
Controls Summ	ary	Value			% Allow	able		ration	C	ase	Loca					
Pos. Moment End Shear		1463 ft- 462 lbs	IDS		96.2%		115 115			4	06-0					
Total Load Deflection	n				41.0% 78.3%		n\a			4 4	00-0 06-0					
Live Load Deflection		L/230 ((L/321 ((-	`	78.3 <i>%</i> 74.8%		n\a			4 5	06-0					
Max Defl.	1	0.69"	J.49J)	69.0%		n\a			4	06-0					
Span / Depth		20.8			00.070		ma			-	00 0	0.00				
Slope and Cut L	_enat	h		Slope	Fase	cia Depth	Hori	iz. Lengi	h I	Produ	uct Leng	nth				
Plumb Cut with Hang			ate	4/12	7-5/			00-00		13-10		<u>, , , , , , , , , , , , , , , , , , , </u>				
Notes																
Design meets Code																
Design meets Code			-													
Design meets arbitra	• • •			al load	deflection	n criteria.							Discl	osure	è	
Minimum bearing ler	•												Use of t	he Boise	Cascade	e Software is
Minimum bearing ler	-															End User
Design based on Dry								0	. 1	-1 4 - 4	(l				ent (EUL/ nd accura	A). Acy of input
The analysis of solid																ified by a
shown above. All oth connections, installa													qualified	d enginee	er or othe	r appropriate
professional of recor			, ci/ait	moot	oonmoali		icopt	, i Sibility		Pioj	0013 00	Sign			its adequ n such ou	acy, prior to
BC CALC® analysis		ed on IBC	2018	3.												a particular
Calculations assume													applicat	ion. The		ere is based on



expert to assure its adequacy, prior to anyone relying on such output as evidence of suitability for a particular application. The output here is based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call (800)232-0788 before installation.

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For root members with slope (1/4)/12 or less final design must ensure that ponding instability will not occur. For root members with slope (1/2)/12 or less final design must account for Pain-on-Snow surcharge

For roof members with slope (1/2)/12 or less final design must account for Rain-on-Snow surcharge load.

Notes

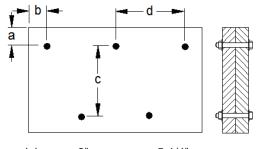
Design meets Code minimum (L/180) Total load deflection criteria. Design meets Code minimum (L/240) Live load deflection criteria. Design meets arbitrary (1") Maximum Total load deflection criteria. Minimum bearing length for B1 is 1-1/2". Minimum bearing length for B2 is 1-1/2". Design based on Dry Service Condition. BC CALC® analysis is based on IBC 2018. Calculations assume member is fully braced.



Boise Cascade	Double 1-3/4" x 9-1/4" VERSA-LAM® LVL 2.1E 3100 SP	PASSED
	RB01 (Roof Beam)	
BC CALC® Member Report	Dry 1 span No cant.	April 25, 2022 11:26:23
Build 8381		

Job name:	Katie & Alfonso Residence	File name:	170 Mount Vernon Street, West Roxbury
Address:	170 Mount Vernon Street	Description:	
City, State, Zip:	West Roxbury, MA, 02132	Specifier:	
Customer:	Derek Rubinoff	Designer:	David Guerrero
Code reports:	ESR-1040	Company:	SSB Engineering

Connection Diagram: Full Length of Member



a minimum = 2"c = 5 - 1/4" b minimum = 2 - 1/2" d = 24"

Calculated Side Load = 0.0 lb/ft Bolts are assumed to be Grade A307 or Grade 2 or higher. Connectors are: 1/2 in. Staggered Through Bolt

Jym Strassburg

Disclosure

Use of the Boise Cascade Software is subject to the terms of the End User License Agreement (EULA). Completeness and accuracy of input must be reviewed and verified by a qualified engineer or other appropriate expert to assure its adequacy, prior to anyone relying on such output as evidence of suitability for a particular application. The output here is based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call (800)232-0788 before installation.

BC CALC®, BC FRAMER® , AJS™, ALLJOIST®, BC RIM BOARD™, BCI®, BOISE GLULAM™, BC FloorValue®, VERSA-LAM®, VERSA-RIM PLUS®,

			1/4" VER RB02 (R	oof Beam							ASSED
BC CALC® Member Build 8381	Report		Dry 1 spa	an No cant					Apri	l 25, 202	2 11:26:23
Job name:	Katie & Alfonso Residend	ce		File name	e: 17	'0 Moun	t Vernor	Street,	West Ro	oxbury	
	170 Mount Vernon Street	t		Descriptio	on:						
	West Roxbury, MA, 0213	2		Specifier:							
	Derek Rubinoff			Designer:		avid Gue					
Code reports:	ESR-1040			Company	: 58	SB Engii	neering				
				3 2 0							
	↓ ↓ ↓ ⁵ ↓ ↓			1/2			▼ ▼ 6	* *	+ +	+ +	+ +
		\downarrow \downarrow \downarrow \downarrow \downarrow \downarrow	$\begin{array}{c c} \downarrow & \downarrow & \downarrow \\ \hline \downarrow & \downarrow & \downarrow \end{array}$				$\frac{\downarrow}{\downarrow}$	$\frac{\downarrow}{\downarrow}$	$\frac{\downarrow}{\downarrow}$	\downarrow \downarrow \downarrow	\downarrow \downarrow
			10	10.00							
B1				-10-00							B2
Reaction Summ	ary (Down / Uplift) (orizontal Pro	duct Length	= 10-10-0	00					
Bearing		Dead	Si	now	V	Vind		Roo	f Live		
31, 3-1/2"		556 / 0	11	116/0							
32, 3-1/2"		552 / 0	11	108 / 0							
Load Summary						Live	Dead	Snow	Wind	Roof	Tributary
Tag Description	Load Type	Ref.	Start	End	Loc.	100%	90%	115%	160%	Live 125%	
) Self-Weight	Unf. Lin. (lb/l		00-00-00	10-10-00	Тор		9				00-00-00
1 RB01	Conc. Pt. (Ib	s) L	05-04-08	05-04-08	Тор		300	650			n\a
2 VB01	Conc. Pt. (Ib	s) L	05-04-08	05-04-08	Тор		161	272			n\a
		,		05 04 00							n\a
	Conc. Pt. (lb	,	05-04-08	05-04-08	Тор		161	272			01-00-00
3 VB01 4 ROOF	Conc. Pt. (lb Unf. Area (lb	/ft²) L	05-04-08 00-00-00	05-04-08 10-10-00	Тор Тор		161 15	272 40			
3 VB01	Conc. Pt. (lb	/ft²) L						40 110			
3 VB01 4 ROOF 5 ROOF	Conc. Pt. (lb Unf. Area (lb Trapezoidal	/ft²) L (lb/ft) L	00-00-00 00-00-00		Тор Тор		15 41 0	40 110 0			n\a
3 VB01 4 ROOF	Conc. Pt. (lb Unf. Area (lb	/ft²) L (lb/ft) L	00-00-00	10-10-00 05-04-08	Тор		15 41 0 0	40 110 0 0			n\a n∖a
3 VB01 4 ROOF 5 ROOF 6 ROOF	Conc. Pt. (lb Unf. Area (lb Trapezoidal Trapezoidal	/ft²) L (lb/ft) L (lb/ft) L	00-00-00 00-00-00 05-04-08	10-10-00	Тор Тор		15 41 0 0 41	40 110 0			n\a
3 VB01 4 ROOF 5 ROOF 6 ROOF Controls Summa	Conc. Pt. (lb Unf. Area (lb Trapezoidal Trapezoidal Trapezoidal	/ft²) L (lb/ft) L (lb/ft) L <u>% Allow</u>	00-00-00 00-00-00 05-04-08	10-10-00 05-04-08 10-10-00 Duration	Top Top Top Case	Loca	15 41 0 41 41	40 110 0 0			n\a
3 VB01 4 ROOF 5 ROOF 6 ROOF 6 ROOF Controls Summa	Conc. Pt. (lb Unf. Area (lb Trapezoidal Trapezoidal Iry Value 6225 ft-lbs	/ft ²) L (lb/ft) L (lb/ft) L <u>% Allow</u> 40.8%	00-00-00 00-00-00 05-04-08 rable D	10-10-00 05-04-08 10-10-00 <u>Duration</u> 15%	Top Top Top <u>Case</u> 4	05-0	15 41 0 41 41 tion 4-08	40 110 0 0			n\a
3 VB01 4 ROOF 5 ROOF 6 ROOF 6 ROOF Controls Summa Pos. Moment End Shear	Conc. Pt. (lb Unf. Area (lb Trapezoidal Trapezoidal Ary Value 6225 ft-lbs 1561 lbs	/ft²) L (lb/ft) L (lb/ft) L <u>% Allow</u> 40.8% 22.1%	00-00-00 00-00-00 05-04-08 rable D 1 1	10-10-00 05-04-08 10-10-00 0uration 15% 15%	Top Top Top Case 4 4	05-0 01-0	15 41 0 41 41 ttion 4-08 0-12	40 110 0 0			n\a
 3 VB01 4 ROOF 5 ROOF 6 ROOF 6 ROOF Controls Summa Pos. Moment End Shear Total Load Deflection 	Conc. Pt. (lb Unf. Area (lb Trapezoidal Trapezoidal ary Value 6225 ft-lbs 1561 lbs L/531 (0.234")	/ft ²) L (lb/ft) L (lb/ft) L <u>% Allow</u> 40.8% 22.1% 33.9%	00-00-00 00-00-00 05-04-08 rable D 1 1 n	10-10-00 05-04-08 10-10-00 Duration 15% 15%	Top Top Top <u>Case</u> 4 4 4	05-0 01-0 05-0	15 41 0 41 41 4-08 0-12 4-08	40 110 0 0			n\a
 3 VB01 4 ROOF 5 ROOF 6 ROOF 6 ROOF Controls Summa Pos. Moment End Shear Total Load Deflection Live Load Deflection 	Conc. Pt. (lb Unf. Area (lb Trapezoidal Trapezoidal 6225 ft-lbs 1561 lbs L/531 (0.234") L/804 (0.155")	/ft ²) L (lb/ft) L (lb/ft) L <u>% Allow</u> 40.8% 22.1% 33.9% 29.8%	00-00-00 00-00-00 05-04-08 rable D 1 1 n n	10-10-00 05-04-08 10-10-00 0uration 15% 15% Na	Top Top Top Case 4 4 4 5	05-0 01-0 05-0 05-0	15 41 0 41 tion 4-08 0-12 4-08 4-08	40 110 0 0			n\a
 3 VB01 4 ROOF 5 ROOF 6 ROOF 7 ROOF 7 ROOF 8 ROOF 9 ROO	Conc. Pt. (lb Unf. Area (lb Trapezoidal Trapezoidal 6225 ft-lbs 1561 lbs L/531 (0.234") L/804 (0.155") 0.234"	/ft ²) L (lb/ft) L (lb/ft) L <u>% Allow</u> 40.8% 22.1% 33.9%	00-00-00 00-00-00 05-04-08 rable D 1 1 n n	10-10-00 05-04-08 10-10-00 Duration 15% 15%	Top Top Top <u>Case</u> 4 4 4	05-0 01-0 05-0	15 41 0 41 tion 4-08 0-12 4-08 4-08	40 110 0 0			n\a
 3 VB01 4 ROOF 5 ROOF 6 ROOF 6 ROOF Controls Summa Pos. Moment End Shear Total Load Deflection Live Load Deflection 	Conc. Pt. (lb Unf. Area (lb Trapezoidal Trapezoidal 6225 ft-lbs 1561 lbs L/531 (0.234") L/804 (0.155")	/ft ²) L (lb/ft) L (lb/ft) L <u>% Allow</u> 40.8% 22.1% 33.9% 29.8%	00-00-00 00-00-00 05-04-08 rable D 1 1 n n	10-10-00 05-04-08 10-10-00 0uration 15% 15% Na	Top Top Top Case 4 4 4 5	05-0 01-0 05-0 05-0	15 41 0 41 tion 4-08 0-12 4-08 4-08	40 110 0 0			n\a
 3 VB01 4 ROOF 5 ROOF 6 ROOF 6 ROOF Controls Summa Pos. Moment End Shear Total Load Deflection Live Load Deflection Max Defl. Span / Depth Cautions 	Conc. Pt. (lb Unf. Area (lb Trapezoidal Trapezoidal 6225 ft-lbs 1561 lbs L/531 (0.234") L/804 (0.155") 0.234" 13.5	/ft ²) L (lb/ft) L (lb/ft) L <u>% Allow</u> 40.8% 22.1% 33.9% 29.8% 23.4%	00-00-00 00-00-00 05-04-08 1 1 1 n n n	10-10-00 05-04-08 10-10-00 0uration 15% 15% Na Na	Top Top Top <u>Case</u> 4 4 4 5 4	05-0 01-0 05-0 05-0 05-0	15 41 0 41 4-08 0-12 4-08 4-08 4-08	40 110 0 0			n\a
 3 VB01 4 ROOF 5 ROOF 6 ROOF 6 ROOF 6 ROOF Controls Summa Pos. Moment End Shear Total Load Deflection Live Load Deflection Max Defl. Span / Depth Cautions For roof members with 	Conc. Pt. (lb Unf. Area (lb Trapezoidal Trapezoidal 6225 ft-lbs 1561 lbs L/531 (0.234") L/804 (0.155") 0.234"	/ft ²) L (lb/ft) L (lb/ft) L <u>% Allow</u> 40.8% 22.1% 33.9% 29.8% 23.4%	00-00-00 00-00-00 05-04-08 1 1 1 n n n	10-10-00 05-04-08 10-10-00 0uration 15% 15% Na Na	Top Top Top <u>Case</u> 4 4 4 5 4	05-0 01-0 05-0 05-0 05-0	15 41 0 41 4-08 0-12 4-08 4-08 4-08	40 110 0 0			n\a
 3 VB01 4 ROOF 5 ROOF 6 ROOF 7 ROOF 8 ROOF 9 ROF 9 ROF<	Conc. Pt. (lb Unf. Area (lb Trapezoidal Trapezoidal 6225 ft-lbs 1561 lbs L/531 (0.234") L/804 (0.155") 0.234" 13.5 h slope (1/4)/12 or less fit	v/ft²) L (lb/ft) L (lb/ft) L (lb/ft) L (lb/ft) L 40.8% 22.1% 33.9% 29.8% 23.4%	00-00-00 00-00-00 05-04-08 1 1 1 1 1 n n n must ensure	10-10-00 05-04-08 10-10-00 0uration 15% 15% 15% 1\a 1\a 1\a	Top Top Top 4 4 4 5 4 9 instabi	05-0 01-0 05-0 05-0 05-0	15 41 0 41 4-08 0-12 4-08 4-08 4-08 4-08	40 110 0 0			n\a
 3 VB01 4 ROOF 5 ROOF 6 ROOF 7 ROOF 8 ROOF 9 ROF 9 ROF<	Conc. Pt. (lb Unf. Area (lb Trapezoidal Trapezoidal 6225 ft-lbs 1561 lbs L/531 (0.234") L/804 (0.155") 0.234" 13.5	v/ft²) L (lb/ft) L (lb/ft) L (lb/ft) L (lb/ft) L 40.8% 22.1% 33.9% 29.8% 23.4%	00-00-00 00-00-00 05-04-08 1 1 1 1 1 n n n must ensure	10-10-00 05-04-08 10-10-00 0uration 15% 15% 15% 1\a 1\a 1\a	Top Top Top 4 4 4 5 4 9 instabi	05-0 01-0 05-0 05-0 05-0	15 41 0 41 4-08 0-12 4-08 4-08 4-08 4-08	40 110 0 0			n\a



Boise Cascade"		Double 1-3/4" x 9-1/4" VERSA-LAM® LVL 2.1E 3100 SP
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RB02 (Roof Beam)

Dry | 1 span | No cant.

April 25, 2022 11:26:23

		Корон
E	Build 8381	
J	ob name:	Katie & Alfonso Residence
A	Address:	170 Mount Vernon Street
C	City, State, Zip:	West Roxbury, MA, 02132
C	Customer:	Derek Rubinoff
C	Code reports:	ESR-1040

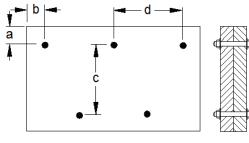
BC CALC® Member Report

File name: 170 Mount Vernon Street, West Roxbury Description: Specifier: Designer: David Guerrero Company: SSB Engineering

Notes

Design meets Code minimum (L/180) Total load deflection criteria. Design meets Code minimum (L/240) Live load deflection criteria. Design meets arbitrary (1") Maximum Total load deflection criteria. Minimum bearing length for B1 is 1-1/2". Minimum bearing length for B2 is 1-1/2". Design based on Dry Service Condition. BC CALC® analysis is based on IBC 2018. Calculations assume member is fully braced.

Connection Diagram: Full Length of Member



a minimum = 2" c = 5-1/4" b minimum = 2-1/2" d = 24"

Calculated Side Load = 0.0 lb/ftBolts are assumed to be Grade A307 or Grade 2 or higher. Connectors are: 1/2 in. Staggered Through Bolt



Disclosure

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BC CALC®, BC FRAMER®, AJS™, ALLJOIST®, BC RIM BOARD™, BCI®, BOISE GLULAM™, BC FloorValue®, VERSA-LAM®, VERSA-RIM PLUS®,

Boise Cascade	Triple 1-3/4	" x 11-1	/4" VER	SA-LAM	® LVL	. 2.1E	3100 \$	SP		P	ASSED
ENGINEERED WOOD PRODUCTS			RB03 (R	oof Beam)						
BC CALC® Member Build 8381	Report		•	an No cant					April	25, 202	2 11:26:23
Job name:	Katie & Alfonso Residence	•		File name	e: 17	70 Mount	t Vernon	Street,	West Ro	xbury	
Address:	170 Mount Vernon Street			Descriptio				,		,	
City, State, Zip:	West Roxbury, MA, 02132			Specifier:							
Customer:	Derek Rubinoff			Designer:	Da	avid Gue	errero				
Code reports:	ESR-1040			Company	: SS	SB Engir	neering				
									3∕ ↓ ↓ ↓ ↓	 6 ↓ ↓ 	
\times											
<u>/</u>			16-	06-00							
B1		Total Ho	rizontal Pro		- 16-06-0	0					B2
Reaction Summ	ary (Down / Uplift) (Ik		112011121110	adot Eengin	- 10-00-0						
Bearing		ead	Sr	now	<u>\</u>	Wind		Roo	f Live		
B1, 3-1/2"	24	422 / 0	52	210/0							
B2, 3-1/2"	23	369 / 0	51	23 / 0							
Load Summary						Live	Dead	Snow	Wind	Roof Live	Tributary
Tag Description	Load Type	Ref.	Start	End	Loc.	Live 100%	90%	Snow 115%	Wind 160%	Roof Live 125%	
TagDescription0Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	16-06-00	Тор		90% 17	115%		Live	00-00-00
TagDescription0Self-Weight1ROOF	Unf. Lin. (lb/ft) Unf. Area (lb/f	L t²) L	00-00-00 00-00-00	16-06-00 16-06-00	Тор Тор		90% 17 15	115% 40		Live	00-00-00 07-00-00
TagDescription0Self-Weight1ROOF2R01	Unf. Lin. (lb/ft) Unf. Area (lb/f Conc. Pt. (lbs)	L t²) L L	00-00-00 00-00-00 02-06-00	16-06-00 16-06-00 02-06-00	Тор Тор Тор		90% 17 15 1110	115% 40 2115		Live	00-00-00 07-00-00 n\a
TagDescription0Self-Weight1ROOF2R013R01	Unf. Lin. (lb/ft) Unf. Area (lb/f Conc. Pt. (lbs) Conc. Pt. (lbs)	L t²) L L L	00-00-00 00-00-00 02-06-00 13-06-00	16-06-00 16-06-00 02-06-00 13-06-00	Тор Тор Тор Тор		90% 17 15 1110 1110	115% 40 2115 2115		Live	00-00-00 07-00-00 n\a n\a
TagDescription0Self-Weight1ROOF2R013R014ROOF	Unf. Lin. (lb/ft) Unf. Area (lb/f Conc. Pt. (lbs) Conc. Pt. (lbs) Unf. Area (lb/f	L t ²) L L L t ²) L	00-00-00 00-00-00 02-06-00 13-06-00 00-00-00	16-06-00 16-06-00 02-06-00 13-06-00 02-05-00	Тор Тор Тор Тор Тор Тор		90% 17 15 1110 1110 15	115% 40 2115 2115 40		Live	00-00-00 07-00-00 n\a n\a 05-00-00
TagDescription0Self-Weight1ROOF2R013R014ROOF5ROOF	Unf. Lin. (lb/ft) Unf. Area (lb/f Conc. Pt. (lbs) Conc. Pt. (lbs) Unf. Area (lb/f Unf. Area (lb/f	L t ²) L L L t ²) L t ²) L	00-00-00 00-00-00 02-06-00 13-06-00 00-00-00 02-07-00	16-06-00 16-06-00 02-06-00 13-06-00 02-05-00 13-05-00	Тор Тор Тор Тор Тор Тор Тор		90% 17 15 1110 1110 15 15	115% 40 2115 2115 40 40		Live	00-00-00 07-00-00 n\a n\a 05-00-00 01-00-00
TagDescription0Self-Weight1ROOF2R013R014ROOF	Unf. Lin. (lb/ft) Unf. Area (lb/f Conc. Pt. (lbs) Conc. Pt. (lbs) Unf. Area (lb/f	L t ²) L L L t ²) L t ²) L	00-00-00 00-00-00 02-06-00 13-06-00 00-00-00	16-06-00 16-06-00 02-06-00 13-06-00 02-05-00	Тор Тор Тор Тор Тор Тор		90% 17 15 1110 1110 15	115% 40 2115 2115 40		Live	00-00-00 07-00-00 n\a n\a 05-00-00
TagDescription0Self-Weight1ROOF2R013R014ROOF5ROOF	Unf. Lin. (lb/ft) Unf. Area (lb/f Conc. Pt. (lbs) Conc. Pt. (lbs) Unf. Area (lb/f Unf. Area (lb/f	L t ²) L L L t ²) L t ²) L	00-00-00 00-00-00 02-06-00 13-06-00 00-00-00 02-07-00 13-08-00	16-06-00 16-06-00 02-06-00 13-06-00 02-05-00 13-05-00	Тор Тор Тор Тор Тор Тор Тор	100%	90% 17 15 1110 1110 15 15 15	115% 40 2115 2115 40 40		Live	00-00-00 07-00-00 n\a n\a 05-00-00 01-00-00
TagDescription0Self-Weight1ROOF2R013R014ROOF5ROOF6ROOF	Unf. Lin. (lb/ft) Unf. Area (lb/f Conc. Pt. (lbs) Conc. Pt. (lbs) Unf. Area (lb/f Unf. Area (lb/f	L t ²) L L t ²) L t ²) L t ²) L	00-00-00 00-00-00 02-06-00 13-06-00 00-00-00 02-07-00 13-08-00 able D	16-06-00 16-06-00 02-06-00 13-06-00 02-05-00 13-05-00 16-06-00	Тор Тор Тор Тор Тор Тор Тор	100%	90% 17 15 1110 1110 15 15 15 15	115% 40 2115 2115 40 40		Live	00-00-00 07-00-00 n\a n\a 05-00-00 01-00-00
TagDescription0Self-Weight1ROOF2R013R014ROOF5ROOF6ROOFControls Summa	Unf. Lin. (lb/ft) Unf. Area (lb/f Conc. Pt. (lbs) Conc. Pt. (lbs) Unf. Area (lb/f Unf. Area (lb/f Unf. Area (lb/f ary Value	L t ²) L L t ²) L t ²) L t ²) L	00-00-00 00-00-00 02-06-00 13-06-00 00-00-00 02-07-00 13-08-00 able D	16-06-00 16-06-00 02-06-00 13-06-00 02-05-00 13-05-00 16-06-00 uration	Top Top Top Top Top Top Case	<u>100%</u>	90% 17 15 1110 1110 15 15 15 15 tion 6-11	115% 40 2115 2115 40 40		Live	00-00-00 07-00-00 n\a n\a 05-00-00 01-00-00
TagDescription0Self-Weight1ROOF2R013R014ROOF5ROOF6ROOFControls SummaPos. Moment	Unf. Lin. (lb/ft) Unf. Area (lb/f Conc. Pt. (lbs) Conc. Pt. (lbs) Unf. Area (lb/f Unf. Area (lb/f Unf. Area (lb/f 23452 ft-lbs 6800 lbs	L t ²) L L t ²) L t ²) L t ²) L % Allowa 70.8%	00-00-00 00-00-00 13-06-00 00-00-00 02-07-00 13-08-00 able D 1 1	16-06-00 16-06-00 02-06-00 13-06-00 02-05-00 13-05-00 16-06-00 uration 15%	Top Top Top Top Top Top Top Case 4	100% Loca 08-0	90% 17 15 1110 1110 15 15 15 15 tion 6-11 2-12	115% 40 2115 2115 40 40		Live	00-00-00 07-00-00 n\a n\a 05-00-00 01-00-00
TagDescription0Self-Weight1ROOF2R013R014ROOF5ROOF6ROOFControls SummaPos. MomentEnd Shear	Unf. Lin. (lb/ft) Unf. Area (lb/f Conc. Pt. (lbs) Conc. Pt. (lbs) Unf. Area (lb/f Unf. Area (lb/f Unf. Area (lb/f 23452 ft-lbs 6800 lbs	L t ²) L L t ²) L t ²) L t ²) L % Allowa 52.7%	00-00-00 00-00-00 13-06-00 00-00-00 02-07-00 13-08-00 13-08-00 1 1 1 1	16-06-00 16-06-00 02-06-00 13-06-00 02-05-00 13-05-00 16-06-00 uration 15%	Top Top Top Top Top Top Top Case 4 4	Loca 08-0 01-0	90% 17 15 1110 15 15 15 tion 6-11 2-12 3-00	115% 40 2115 2115 40 40		Live	00-00-00 07-00-00 n\a n\a 05-00-00 01-00-00
TagDescription0Self-Weight1ROOF2R013R014ROOF5ROOF6ROOFControls SummaPos. MomentEnd ShearTotal Load Deflection	Unf. Lin. (lb/ft) Unf. Area (lb/f Conc. Pt. (lbs) Conc. Pt. (lbs) Unf. Area (lb/f Unf. Area (lb/f Unf. Area (lb/f 23452 ft-lbs 6800 lbs h L/208 (0.923")	L t ²) L L t ²) L t ²) L t ²) L % Allowa 52.7% 86.3%	00-00-00 00-00-00 13-06-00 00-00-00 02-07-00 13-08-00 13-08-00 1 1 1 1 1 1	16-06-00 16-06-00 02-06-00 13-06-00 02-05-00 13-05-00 16-06-00 uration 15% 15%	Top Top Top Top Top Top Case 4 4 4	Loca 08-0 01-0 08-0	90% 17 15 1110 15 15 15 tion 6-11 2-12 3-00 3-00	115% 40 2115 2115 40 40		Live	00-00-00 07-00-00 n\a n\a 05-00-00 01-00-00
TagDescription0Self-Weight1ROOF2R013R014ROOF5ROOF6ROOFControls SummaPos. MomentEnd ShearTotal Load DeflectionLive Load Deflection	Unf. Lin. (lb/ft) Unf. Area (lb/ft) Conc. Pt. (lbs) Conc. Pt. (lbs) Unf. Area (lb/f Unf. Area (lb/f Unf. Area (lb/f 23452 ft-lbs 6800 lbs h L/208 (0.923") L/305 (0.632")	L t ²) L L t ²) L t ²) L t ²) L <u>% Allowa</u> 70.8% 52.7% 86.3% 78.8%	00-00-00 00-00-00 13-06-00 00-00-00 02-07-00 13-08-00 13-08-00 1 1 1 1 1 1	16-06-00 16-06-00 02-06-00 13-06-00 02-05-00 13-05-00 16-06-00 uration 15% 15% \a	Top Top Top Top Top Top Top Case 4 4 4 5	Loca 08-0 01-0 08-0 08-0 08-0	90% 17 15 1110 15 15 15 tion 6-11 2-12 3-00 3-00	115% 40 2115 2115 40 40		Live	00-00-00 07-00-00 n\a n\a 05-00-00 01-00-00
TagDescription0Self-Weight1ROOF2R013R014ROOF5ROOF6ROOFControls SummaPos. MomentEnd ShearTotal Load DeflectionLive Load DeflectionMax Defl.Span / DepthCautions	Unf. Lin. (lb/ft) Unf. Area (lb/ft) Conc. Pt. (lbs) Conc. Pt. (lbs) Unf. Area (lb/f Unf. Area (lb/f Unf. Area (lb/f Unf. Area (lb/f 23452 ft-lbs 6800 lbs L/208 (0.923") L/305 (0.632") 0.923" 17.1	L t ²) L <u>% Allowa</u> 52.7% 86.3% 78.8% 92.3%	00-00-00 00-00-00 13-06-00 00-00-00 02-07-00 13-08-00 13-08-00 1 1 1 1 1 1 1	16-06-00 16-06-00 02-06-00 13-06-00 13-05-00 13-05-00 16-06-00 uration 15% \a \a \a	Top Top Top Top Top Top Case 4 4 4 5 4	Loca 08-0 01-0 08-0 08-0 08-0	90% 17 15 1110 15 15 15 tion 6-11 2-12 3-00 3-00 3-00	115% 40 2115 2115 40 40		Live	00-00-00 07-00-00 n\a n\a 05-00-00 01-00-00
TagDescription0Self-Weight1ROOF2R013R014ROOF5ROOF6ROOFControls SummaPos. MomentEnd ShearTotal Load DeflectionLive Load DeflectionMax Defl.Span / DepthCautionsFor roof members with	Unf. Lin. (lb/ft) Unf. Area (lb/ft) Conc. Pt. (lbs) Conc. Pt. (lbs) Unf. Area (lb/f Unf. Area (lb/f Unf. Area (lb/f 23452 ft-lbs 6800 lbs L/208 (0.923") L/305 (0.632") 0.923"	L t ²) L <u>% Allowa</u> 52.7% 86.3% 78.8% 92.3%	00-00-00 00-00-00 13-06-00 00-00-00 02-07-00 13-08-00 13-08-00 1 1 1 1 1 1 1	16-06-00 16-06-00 02-06-00 13-06-00 13-05-00 13-05-00 16-06-00 uration 15% \a \a \a	Top Top Top Top Top Top Case 4 4 4 5 4	Loca 08-0 01-0 08-0 08-0 08-0	90% 17 15 1110 15 15 15 tion 6-11 2-12 3-00 3-00 3-00	115% 40 2115 2115 40 40		Live	00-00-00 07-00-00 n\a n\a 05-00-00 01-00-00
Tag Description 0 Self-Weight 1 ROOF 2 R01 3 R01 4 ROOF 5 ROOF 6 ROOF Pos. Moment End Shear Total Load Deflection Max Defl. Span / Depth Cautions For roof members wir occur.	Unf. Lin. (lb/ft) Unf. Area (lb/ft) Conc. Pt. (lbs) Conc. Pt. (lbs) Unf. Area (lb/f Unf. Area (lb/f Unf. Area (lb/f Unf. Area (lb/f 23452 ft-lbs 6800 lbs L/208 (0.923") L/305 (0.632") 0.923" 17.1	L t ²) L L t ²) L t ²) L t ²) L % Allowa 70.8% 52.7% 86.3% 78.8% 92.3% al design n	00-00-00 00-00-00 13-06-00 00-00-00 02-07-00 13-08-00 13-08-00 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	16-06-00 16-06-00 02-06-00 13-06-00 02-05-00 13-05-00 16-06-00 uration 15% \a \a \a	Top Top Top Top Top Top Case 4 4 4 5 4 9 instabi	Loca 08-0 01-0 08-0 08-0 08-0	90% 17 15 1110 15 15 15 15 tion 6-11 2-12 3-00 3-00 3-00 3-00	115% 40 2115 2115 40 40		Live	00-00-00 07-00-00 n\a n\a 05-00-00 01-00-00
Tag Description 0 Self-Weight 1 ROOF 2 R01 3 R01 4 ROOF 5 ROOF 6 ROOF Pos. Moment End Shear Total Load Deflection Max Defl. Span / Depth Cautions For roof members wir occur.	Unf. Lin. (lb/ft) Unf. Area (lb/ft) Conc. Pt. (lbs) Conc. Pt. (lbs) Unf. Area (lb/f Unf. Area (lb/f Unf. Area (lb/f Unf. Area (lb/f 23452 ft-lbs 6800 lbs L/208 (0.923") L/305 (0.632") 0.923" 17.1	L t ²) L L t ²) L t ²) L t ²) L % Allowa 70.8% 52.7% 86.3% 78.8% 92.3% al design n	00-00-00 00-00-00 13-06-00 00-00-00 02-07-00 13-08-00 13-08-00 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	16-06-00 16-06-00 02-06-00 13-06-00 02-05-00 13-05-00 16-06-00 uration 15% \a \a \a	Top Top Top Top Top Top Case 4 4 4 5 4 9 instabi	Loca 08-0 01-0 08-0 08-0 08-0	90% 17 15 1110 15 15 15 15 tion 6-11 2-12 3-00 3-00 3-00 3-00	115% 40 2115 2115 40 40		Live	00-00-00 07-00-00 n\a n\a 05-00-00 01-00-00

Design meets Code minimum (L/180) Total load deflection criteria. Design meets Code minimum (L/240) Live load deflection criteria. Design meets arbitrary (1") Maximum Total load deflection criteria. Minimum bearing length for B1 is 1-15/16". Minimum bearing length for B2 is 1-7/8". Design based on Dry Service Condition. BC CALC® analysis is based on IBC 2018.

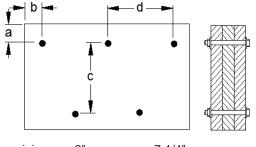
Calculations assume member is fully braced.



Boise Cascade	Triple 1-3/4" x 1	1-1/4" VERSA-LAM® I	_VL 2.1E 3100 SP	PASSED
ENGINEERED TOODPRO	00012	RB03 (Roof Beam)		
BC CALC® Mer	nber Report	Dry 1 span No cant.		April 25, 2022 11:26:23
Build 8381				
Job name:	Katie & Alfonso Residence	File name:	170 Mount Vernon Street, Wes	st Roxbury
Address:	170 Mount Vernon Street	Description:		

170 Mount Vernon Street Description: City, State, Zip: West Roxbury, MA, 02132 Specifier: Customer: Derek Rubinoff Designer: David Guerrero Code reports: ESR-1040 Company: SSB Engineering

Connection Diagram: Full Length of Member



a minimum = 2"c = 7 - 1/4" b minimum = 2-1/2"d = 24"

Calculated Side Load = 0.0 lb/ft Bolts are assumed to be Grade A307 or Grade 2 or higher. Connectors are: 1/2 in. Staggered Through Bolt



Disclosure

Use of the Boise Cascade Software is subject to the terms of the End User License Agreement (EULA). Completeness and accuracy of input must be reviewed and verified by a qualified engineer or other appropriate expert to assure its adequacy, prior to anyone relying on such output as evidence of suitability for a particular application. The output here is based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call (800)232-0788 before installation.

BC CALC®, BC FRAMER® , AJS™, ALLJOIST®, BC RIM BOARD™, BCI®, BOISE GLULAM[™], BC FloorValue®, VERSA-LAM®, VERSA-RIM PLUS®,

Boise Cascade	Tr	iple 1-3/4	" x 9-1	/4" VER	SA-LAM	® LVL	2.1E :	3100 \$	SP		P	ASSED
ENGINEERED WOOD PRODUCTS			R	804 (Roo	f Drop Bea	am)						
BC CALC® Member Build 8381	Report			Dry 1 sp	an No can	t.				Apri	25, 202	2 11:26:23
Job name:	Katie & Alfons	o Residence			File name	e: 17	70 Moun	t Vernor	n Street,	West Ro	oxbury	
Address:	170 Mount Ver	rnon Street			Descriptio		OWER R				,	
City, State, Zip:	West Roxbury	, MA, 02132			Specifier:							
Customer:	Derek Rubinof	f			Designer	: D	avid Gue	errero				
Code reports:	ESR-1040				Company	/: S	SB Engii	neering				
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\bowtie												
k				09	-06-00							
B1			Total Ho	orizontal Pro	duct Length	= 09-06-0	00					B2
Reaction Summ	ary (Down /	Uplift) (lb			U							
Bearing	Live	De			now		Wind		Roo	f Live		
B1, 3-1/2"		-	2/0		280/0							
B2, 3-1/2"		92	2/0	22	280 / 0							
Load Summary							Live	Dead	Snow	Wind	Roof Live	Tributary
Tag Description	Lo	oad Type	Ref.	Start	End	Loc.	100%	90%	115%	1 60 %	125%	
0 Self-Weight	Ui	nf. Lin. (Ib/ft)	L	00-00-00	09-06-00	Тор		14				00-00-00
1 ROOF	Ui	nf. Area (lb/ft ²	²) L	00-00-00	09-06-00	Тор		15	40			12-00-00
Controls Summ	ary Value		% Allow	able D	Duration	Case	Loca	ition				
Pos. Moment	6888 ft-	lbs	30.1%	1	15%	4	04-0	9-00				
End Shear	2486 lb	S	23.4%	1	15%	4	01-0	0-12				
Total Load Deflectio	n L/700 (0.155")	25.7%	r	n\a	4	04-0	9-00				
Live Load Deflection	L/999 (0.11")	n∖a	r	n\a	5	04-0	9-00				
Max Defl.	0.155"		15.5%	r	n∖a	4	04-0	9-00				
Span / Depth	11.7											
Cautions												
For roof members w	ith slope $(1/4)/1$	2 or less fina	l design r	nust ansure	that nondir	na instah	ility will r					
occur.			, acaigir i			19 11 3 140	inty will I					
For roof members w load.	ith slope (1/2)/1	2 or less fina	l design r	nust accour	nt for Rain-o	n-Snow	surcharg	je				
Notes												
Design meets Code)) Total load	deflection	criteria								

Design meets Code minimum (L/240) Live load deflection criteria. Design meets arbitrary (1") Maximum Total load deflection criteria. Minimum bearing length for B1 is 1-1/2". Minimum bearing length for B2 is 1-1/2". Design based on Dry Service Condition. BC CALC® analysis is based on IBC 2018. Calculations assume member is fully braced.



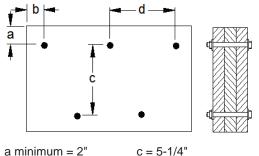
Boise Cascade Triple 1-3/4" x 9-1/4" VERSA-LAM® LVL 2.1E 3100 SP	PASSED
RB04 (Roof Drop Beam)	
BC CALC® Member ReportDry 1 span No cant.April 25,	2022 11:26:23
Build 8381	
Job name: Katie & Alfonso Residence File name: 170 Mount Vernon Street, West Roxbu	ſy
Address: 170 Mount Vernon Street Description: LOWER RIDGE	
City, State, Zip: West Roxbury, MA, 02132 Specifier:	
Customer: Derek Rubinoff Designer: David Guerrero	

Company:

SSB Engineering

Connection Diagram: Full Length of Member

ESR-1040



b minimum = 2-1/2"d = 24"

Code reports:

Calculated Side Load = 0.0 lb/ft Bolts are assumed to be Grade A307 or Grade 2 or higher. Connectors are: 1/2 in. Staggered Through Bolt



Disclosure

Use of the Boise Cascade Software is subject to the terms of the End User License Agreement (EULA). Completeness and accuracy of input must be reviewed and verified by a qualified engineer or other appropriate expert to assure its adequacy, prior to anyone relying on such output as evidence of suitability for a particular application. The output here is based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call (800)232-0788 before installation.

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100000

Double 2 x 10 SPF #2

PASSED

questions, please call (800)232-0788 before installation.

BC CALC®, BC FRAMER® , AJS™, ALLJOIST® , BC RIM BOARD™, BCI® , BOISE GLULAM™, BC FloorValue® , VERSA-LAM®, VERSA-RIM PLUS® ,

BC CALC® Member Rep	oort		•	oof Beam an No cant					Apri	25, 202	2 11:26:23
Address:170City, State, Zip:WeCustomer:De	tie & Alfonso Residence 0 Mount Vernon Street est Roxbury, MA, 02132 rek Rubinoff GA			File name Descriptio Specifier: Designer Company	on: : D	70 Mount avid Gue SB Engir	errero	n Street, '	West Ro	oxbury	
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X											
₩ }											
B1		Terette		10-00	07.40						B2
Reaction Summary	/ (Down / Uplift) (Ib:		orizontal Pro	duct Length	= 07-10-	00					
Bearing	Live De		Si	างพ		Wind		Roo	f Live		
B1, 3-1/2"		2/0		32 / 0							
B2, 3-1/2"	16	1/0	27	73 / 0							
							Deed	0	Martin d	Deef	T
Load Summary						Live	Dead	Snow	Wind	Roof Live	Tributary
Tag Description	Load Type	Ref.	Start	End	Loc.	100%	90%	115%	160%	125%	
0 Self-Weight	Unf. Lin. (lb/ft)	L	00-00-00	07-10-00	Тор		8				00-00-00
1 ROOF	Trapezoidal (lb/	/ft) R	00-00-00		Тор		0	0			n\a
				07-10-00			42	110			
2 ROOF	Trapezoidal (lb/	/ft) R	00-00-00	07 40 00	Тор		0	0			n∖a
				07-10-00			39	105			
Controls Summary	Value	% Allow	able D	uration	Case	Loca	tion				
Pos. Moment	1157 ft-lbs	29.3%	1	15%	4	03-0	5-06				
End Shear	778 lbs	27.1%	1	15%	4	00-0	3-08				
Total Load Deflection	L/999 (0.064")	n∖a	n	\a	4	03-0					
Live Load Deflection	L/999 (0.041")	n∖a		\a	5	03-0					
Max Defl.	0.064"	n∖a	n	\a	4	03-0	9-08				
Span / Depth	9.6										
Slope and Cut Len	gth Slope	Faco	ia Depth Ho	oriz Longth	Pro	duct Leng	uth				
Plumb Cut with Hanger t				7-10-00)5-13		Discl	losure	•	
<u>j</u>					_						Software is
Notes											End User
	imum (L/180) Total load o	deflection	criteria.							ent (EULA	 cy of input
-	imum (L/240) Live load d									d and veri	
	1") Maximum Total load o								0		appropriate acy, prior to
Minimum bearing length	for B1 is 1-1/2".									n such ou	
Minimum bearing length								evidenc	e of suita	bility for a	particular
Design based on Dry Se	rvice Condition.									output hei cepted de	re is based of
	vn wood members is in a									nalysis me	
	support and design for the							Installat	ion of Bo	ise Casca	ide
connections, installation, professional of record.	, and engineer/architect c	entificatio	on is the res	ponsibility c	n the pro	ject's de	sign			d products current In	s must be in
BC CALC® analysis is b	ased on IBC 2018										ling codes. T
Calculations assume me								obtain li	nstallatio	n Guide o	r ask
				A DESCRIPTION OF THE OWNER OWNER OF THE OWNER OWNE				question	IS. DIEAS	e call (800))232-0788



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