Project Notification Form

Submitted Pursuant to Article 80 of the Boston Zoning Code

Frankfort + Gove Street Housing Project



Submitted to: Boston Planning & Development Agency

One City Hall Square Boston, MA 02201

Submitted by: Frankfort Gove, LLC 220 Boylston Street, Unit 1214 Boston, MA 02116 Prepared by:

Epsilon Associates, Inc. 3 Mill & Main Place, Suite 250 Maynard, MA 01754

In Association with:

Bruner + Cott Associates, Inc. BSC Group Drago & Toscano, LLP EBI Consulting Vanasse & Associates, Inc.

October 5, 2018



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Chapter 1

General Information

1.0 GENERAL INFORMATION

1.1 Introduction

Frankfort Gove, LLC (the Proponent) proposes the redevelopment of an approximately 49,125 square foot (sf) site in East Boston, Massachusetts. The redevelopment involves the renovation of Our Lady of Mt. Carmel Catholic Church and the creation of 112 residential units in a new building, along with 84 associated parking spaces, open space and public realm improvements (the Project). The redevelopment site includes four parcels and is generally bound by Frankfort Street to the west, Lubec Street to the east and existing residential and institutional properties to the north and south (the Project site). Our Lady of Mt. Carmel Catholic Church was ordered closed in 2004 and this redevelopment will bring the attractive buildings back to useful life. Certain buildings within the Church complex are historic resources (See Section 7.0).

The Project will blend with the predominantly residential neighborhood and will revitalize Our Lady of Mt. Carmel Catholic Church (Church Building) as a cornerstone of this development. The Church Building has long been a notable beacon of this East Boston neighborhood. The proposed massing and materials of the new building along Frankfort Street (Frankfort Street Building) will mirror those of the existing residences while providing distinct and contemporary design. Along Gove Street, the new building will be similar in height to the other buildings along the street and in materials which were inspired by the context and in contemporary design.

The Project site is less than one half mile from the Maverick Blue Line station as well as multiple routes of major bus lines, which will easily facilitate the commute for future residents of the Project. Additionally, the site is approximately two miles from Boston Logan International Airport.

This Expanded Project Notification Form (PNF) is being submitted to the BRA doing business as Boston Planning and Development Agency (herein, the BPDA), to initiate review of the Project under Article 80B, Large Project Review, of the Boston Zoning Code. The PNF offers a description of the Project, its minimal impacts and its benefits to the City of Boston. A Letter of Intent (LOI) was filed with the BPDA on February 23, 2018, in accordance with the Boston Zoning Code.

1.2 Project Identification and Team

Address/Location: Four parcels at the intersection of Frankfort and

Gove streets in East Boston, Massachusetts (115 Gove Street, 120 Gove Street, 128-134 Gove Street,

21-43 Frankfort Street)

Proponent: Frankfort Gove, LLC

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Andrew Bisbee

1.3 Public Benefits

The Proposed Project will provide substantial benefits to the City of Boston and the East Boston community. The Project will generate both direct and indirect economic and social benefits to the East Boston neighborhood. The Project provides for:

- ◆ Creating much needed market rate residential housing in the East Boston Neighborhood;
- Creating fifteen on-site affordable home ownership units, which will meet the BPDA's affordable housing standards;
- Revitalizing several underutilized parcels and replacing the previous church use and church associated uses with modern residential housing units;
- Replacing an underutilized vacant parking lot with residential housing use;
- Replacing vacant buildings and land with residential use;
- Maintaining and renovating the existing Church Building, allowing for the preservation of this iconic neighborhood structure;
- Removing the existing compromised structures that have fallen into a state of disrepair and don't meet current Building Code requirements;

- Constructing a building that will incorporate open space in the form of private and communal decking;
- Creating a reflection space adjacent to the former Church Building, providing the neighborhood with landscaped open space;
- Creating a multifaceted and detailed landscape plan to incorporate plants and trees into the reflection space, parking facilities, streetscapes and open spaces within the properties;
- Constructing an underground parking garage that will accommodate parking spaces for the unit owners;
- Constructing a ground level parking facility that will accommodate parking spaces for the unit owners in the Church Building and which integrates trees and landscaping to minimize visual detriments to the surrounding abutters;
- ♦ Additional revenue in the form of property taxes to the City of Boston;
- Creating 124 construction and labor jobs;
- Providing housing proximate to the MBTA station for middle income residents, as well as affordable housing in compliance with the City of Boston Inclusionary Development Policy to promote the Commonwealth's Transit-Oriented Development policy goals;
- ◆ Creating bicycle storage within the building to encourage bicycling as a mode of transportation, allowing for less vehicular traffic;
- Encouraging alternative modes of transportation such as use of bicycling and walking, due to the close proximity of the MBTA Maverick and Airport stations;
- New stormwater management strategies and infrastructure that will significantly improve the quality and decrease the quantity of stormwater generated by the site when compared to the existing conditions;
- Streetscape and urban design elements that will enhance the pedestrian experience and the surrounding neighborhood through lighting and landscaping; and
- ◆ The Project will be constructed to be certifiable under the Leadership in Energy and Environmental Design (LEED) rating system targeting the Silver level.

1.4 Legal Information

1.4.1 Legal Judgements Adverse to the Proposed Project

There are no legal judgements or actions pending concerning the Proposed Project.

1.4.2 History of Tax Arrears on Property

All taxes due for the property have been timely paid by the Proponent, including the current FY taxes.

1.4.3 Site Control/Public Easements

The Project site, consisting of 1.13 acres, is under the control of the Proponent. On August 17, 2015, the Proponent acquired the four parcels which comprise the Project site, consisting of approximately 49,125 sf.

There are no public easements through the Project site.

1.5 Public Participation

The Project team has provided extensive community outreach efforts for the Proposed Project including community meetings in the East Boston neighborhood, and presentations before the elected officials. As part of the process, the Project team will hold an abutter's meeting to explain the Project to surrounding neighbors that will be directly impacted during and after construction. The Project team appeared three times before the Gove Street Association, as well as meeting with the Impact Advisory Group (IAG). The Proponent received feedback from the neighbors and members of the IAG and has made design changes accordingly.

Finally, the Project team has met individually with all of East Boston's elected officials and their staff members, including: Representative Adrian Madaro, City Councilor Lydia Edwards, and Mayor's Office of Neighborhood Services Liaison for East Boston, Jose Garcia-Mota. East Boston's elected officials have had input during the community outreach process and have had staff present at all community meetings.

The Proponent will continue to meet with public agencies, neighborhood representatives, local business organizations, abutting property owners, and other interested parties, and will follow the requirements of Article 80 pertaining to the public review process.

Chapter 2

Project Description

2.0 PROJECT DESCRIPTION

2.1 Project Description

2.1.1 Project Site

The Project site includes four parcels totaling approximately 49,125 square feet (sf). The Frankfort Street and 115 Gove Street parcels consist of approximately 32,390 sf and currently contain a Convent Building and a vacant lot. The 120 Gove Street and 128-134 Gove Street parcels total approximately 16,735 sf and currently include the closed Our Lady of Mt. Carmel Catholic Church and attached Rectory Building. See Figure 2-1 for an aerial locus map of the Project site and Figure 2-2 through Figure 2-6 for existing conditions on the site. Attachment A provides a site survey of the Frankfort and Gove Street parcels.

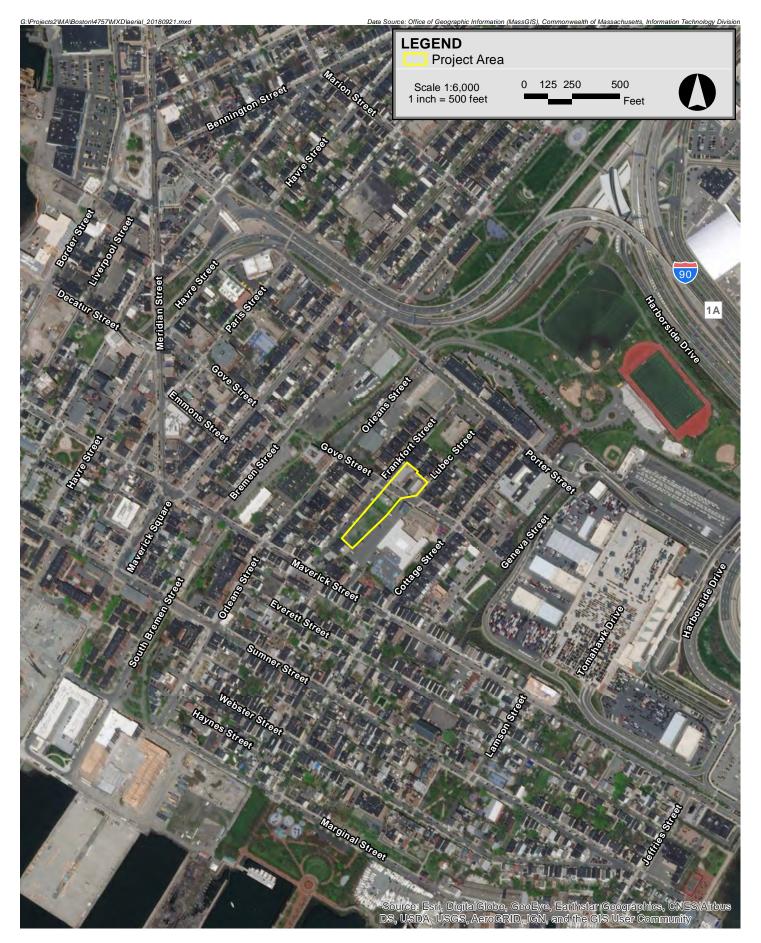
2.1.2 Area Context

The area surrounding the Project site includes a mix of residential, commercial and institutional uses. Existing residences are located to the north, west and south of the site. These residences are generally three-stories with brick facades, typical of East Boston. The Donald McKay School and East Boston Early Education Center are located directly to the southeast of the site. See Figure 2-7 for existing buildings in the vicinity of the Project site.

Maverick Square, located within a half mile from the Project site, includes numerous restaurants, shops, access to public transportation, parks and open spaces. Open spaces in the area include Lombardi Memorial Park, Lewis Mall, East Boston Greenway, Piers Park Sailing Center, Brophy Park, and Sumner and Lamson Street Playground. The Project site is located within one-half mile from the Maverick MBTA Blue Line station and is along the routes of multiple major bus lines. The Project site is also approximately two miles from Logan International Airport. The proximity to public transit makes the area an ideal location for transit-oriented development.

2.1.3 Proposed Project

The Project proposes approximately 120,430 gross sf of residential space, approximately 27,885 gross sf of below-grade parking and approximately 22,140 sf of open space. In total, the Project will include approximately 112 units and approximately 84 parking spaces, 71 of which are located in a below-grade garage. Thirteen parking spaces are off Lubec Street in a new landscaped lot. The Project will also include approximately 112 covered bicycle parking spaces and approximately 23 outdoor bicycle parking spaces. The parking garage will provide electrical vehicle charging capacity.



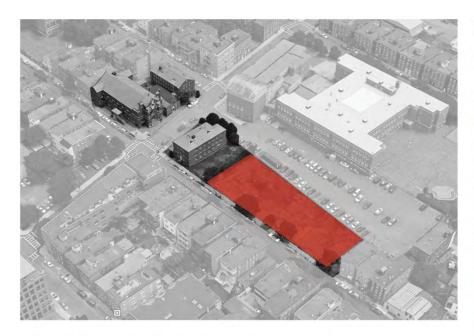
Frankfort Gove Street Housing Boston, Massachusetts





Frankfort Gove Street Housing Boston, Massachusetts



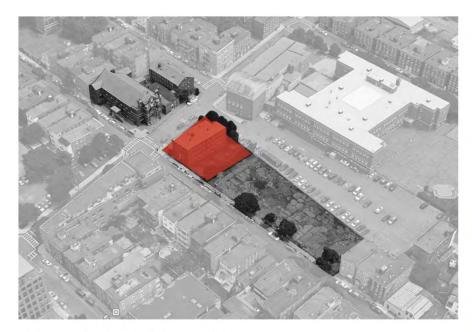


FRANKFORT STREET PARCEL

EXISTING VACANT LOT



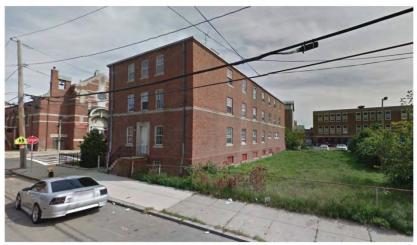


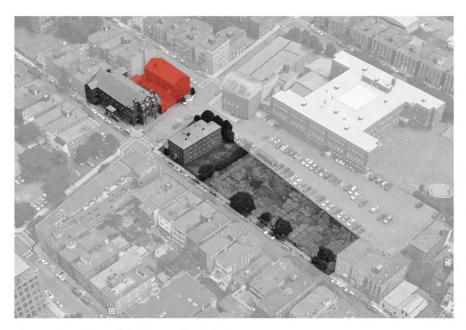




EXISTING CONVENT BUILDING - NOT SUITABLE FOR HOUSING





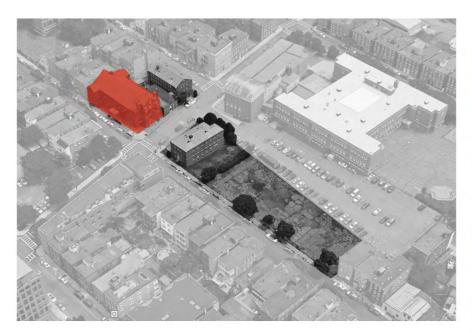


128 - 134 GOVE STREET PARCEL

• EXISTING RECTORY BUILDING - NOT SUITABLE FOR HOUSING









- EXISTING CHURCH BUILDING
- 1907-1920







48 FRANKFORT STINTERSECTION OF FRANKFORT & GOVE ST



THE GUMBALL FACTORY
150 ORLEANS ST

The renovated Church Building will include approximately 14 residential units. The existing Rectory and Convent buildings will be removed. The vacant Frankfort Street parcel will include the construction of a new building including approximately 98 condominium units (identified as the Frankfort Street Building, which will encompass the 115 Gove Street parcel and the Frankfort Street parcel).

The Project will provide market-rate and affordable units with a variety of unit sizes and styles to accommodate East Boston's diverse and growing population. The unit designs will vary and include apartments and lofts. The unit mix for the approximately 112 units will comprise a mix of studio units, one-bedroom or one-bedroom plus den units, and two-bedroom units. Approximately 13% of the units will be designated as affordable in accordance with the Mayor's Inclusionary Development Policy. There will be a number of private decks as well as a common amenity roof deck to provide ample outdoor space for the residents. Table 2-1 includes the Project program. Figures 2-8 to 2-26 at the end of this section show site plans, floor plans and sections of the Project.

Table 2-1 Proposed Program

Project Element	Approximate Dimension	
Church Building		
1-bedroom / 1-bedroom with den	10	
2-bedroom	4	
Frankfort Street Building		
1-bedroom / 1-bedroom with den	68	
2-bedroom	30	
Total Units	112 units	
Parking	71 below-grade spaces, 13 surface spaces	
Total Parking	84 spaces	
Zoning Height*	Up to 65 feet	
Parcel Area	49,125 square feet (1.13 acres)	
FAR	2.45	

^{*}The As-of-Right height is 35 feet; the Project is seeking a variance of up to 65 feet.

2.1.4 Evolution of Design

During the conceptual design and planning of the Project, the Project team outlined several goals:

- Respectfully reposition the existing Church Building as a cornerstone of the neighborhood;
- Create a modern residential building that is woven into the surrounding context, reflecting its size, shape, and materiality;

- Bring additional vibrancy to the streets of this high-quality, urban neighborhood by activating the street and introducing beautiful landscaped spaces along Gove Street; and
- Be a model for resilient planning and sustainable design in 21st century Boston.

Repositioning the Existing Church Building

The Church Building will be the cornerstone of this development, respectfully reused and converted into spacious loft-style living units that capture the soaring interior spaces and volume of the building. There will be three levels of residential units with the top floor units capturing the currently hidden truss space above the vaulted ceiling. The exterior of the building, including its brick and stone façade, will be restored. It will be sensitively repaired where religious iconography was removed. The front door and side window openings will be lowered to the ground to better connect the building to the street and surrounding landscape. A reflective outdoor space that honors the church's history will be crafted along the widened sidewalks at the corner of Frankfort and Gove streets. A new corner green space will be built at the corner of Gove and Lubec Streets to conceal parking for the Church Building units, while also creating a landscaped edge along Gove Street bringing much needed green space to the neighborhood.

Weaving into the Gove Street Neighborhood

<u>Frankfort Street Row Houses:</u> Mirroring the brick row houses along Frankfort Street will be a four-story structure extending along Frankfort Street and articulated as individual row houses with separate, raised entrances and planter boxes. The Project will serve to activate the street and add individual character including urban landscape and street trees. The fourth floor will contain setbacks for private deck space while reducing the massing along Frankfort Street. One row house will serve as an accessible main entrance to the condominiums and six-story building at the corner of Frankfort and Gove streets. The rear of the row houses will house a raised green space with fencing running along the adjacent School parking area. The primary material will be brick, with stone and ironwork accents, recalling the materials and details used around the neighborhood. A fifth story roof deck will be set back approximately 30 feet from the property line along Frankfort Street.

Frankfort Street Condominium Building: At the corner of Frankfort and Gove streets, there will be a six-story building condominium building. The existing street corridor along Gove Street includes taller structures such as the Gumball Factory Loft Building, the adjacent school building, the Church Building, and the former School Building that was converted to residential living. This six-story building will contain one- and two-bedroom condominium units with access to the main common roof deck amenity space through the main elevator lobby. The sixth floor will be set back approximately 25 feet from the Gove Street property line and approximately 12 feet from the Frankfort Street property line to reduce massing and shadows. The upper floor materials will be lighter in color and texture.

Materials - New Building

The primary exterior materials have been chosen with respect to the existing patterns, scale, and materials present within the adjacent Gove Street neighborhood. A modern mixture of brick and terra-cotta façade materials will complement the existing neighborhood brick in color and texture. Modern windows and Juliette balconies will complement the new facades in color and scale. The Project will feature cast iron railings and stone accents along the streetscape at Frankfort Street, as well as a modern interpretation of the existing ornamental cornices along Frankfort Street. The façade will be broken down in sections that respond to the scale of the row houses along Frankfort Street, reflecting their masonry facades and detail. The facades located above the fifth floor will set back from the street and be lighter in color. The ground floor of the six-story element will feature new glazing at the lobby and amenity spaces at the intersection of Frankfort and Gove streets. This glazed façade will wrap along Gove Street allowing view of the existing Church Building from the amenity spaces.

Landscape and Street Vibrancy

The Project will feature a new landscaped lot adjacent to the existing Church Building at Gove and Lubec streets. The landscaped lot will integrate approximately 13 parking spaces for the renovated Church Building, concealing them from the street through plantings and topography. The parking spaces will be integrated into the landscape with grated trees and coordinated pavers and plantings. New street trees along both sides of Gove Street will serve to activate a section of the neighborhood and suggest a link between the Church Building and the new Frankfort Street Building across Gove Street. The Project is also proposing a widened streetscape with landscaping and hardscape material selections in accordance with the Boston Complete Street guidelines where applicable.

2.2 City of Boston Zoning

2.2.1 Site Zoning

The Project site is located in a Multi-Family Resident Subdistrict (MFR) of the East Boston Neighborhood District, Article 53 of the Boston Zoning Code (the "Code"). (See below Table 2-2 Gove Street - Zoning Compliance and Table 2-3 Frankfort Street - Zoning Compliance).

The Project site is comprised of four separate City of Boston parcels. The parcel ID's for these parcels are Parcel ID 0104015000, which has an address of 120 Gove Street, Parcel ID 0104010000, which has an address of 128-134 Gove Street, Parcel ID 0103988002, which is known as Frankfort Street, and Parcel ID 0103988001, which is a vacant lot that is also only known as Frankfort Street. The Gove Street site will consist of the Gove Street parcels and will contain approximately 16,735 sf of land. The Frankfort Street site will consist of the two Frankfort Street parcels and will contain approximately 32,390 sf. Multi-

family dwellings are an allowed use in Multi-Family Residential Subdistricts under Article 53, Table A. Therefore, since both Project site are located within a Multi-Family Residential Subdistrict a use variance will not be necessary.

The proposed Project will seek relief from several dimensional regulation requirements of the existing zoning outlined in Article 53. One or both of the proposed buildings will require variances from the Zoning Board of Appeal, will likely include: floor-area-ratio ("FAR"), height, additional lot area per additional dwelling unit, open space, and rear yard setback. Depending on the final design and location of the buildings, other dimensional regulations may result in noncompliance with Article 53, Section 9, triggering furthering zoning exceptions, which would also require variances.

The proposed Project will not seek any relief regarding Off-Street Parking requirements or Off-Street Loading requirements. Typically, under to Table N of Article 53, which governs the parking requirements within East Boston, 2.0 parking spaces must be provided per unit when ten or more units are being proposed. However, footnote number one to Table N states that the provisions of Table N do not apply to proposed projects that are subject to Large Project Review. Similarly, according to Table O of Article 53, Off-Street Loading requirements do not apply to projects subject to Large Project Review. Therefore, no variances or relief will be required regarding the proposed parking or loading spaces relating to the Project.

The site is located in an area that contains primarily residential uses. The abutting structures are a variety of residential dwellings and apartment buildings. Although some of the two and three-family structures in the neighborhood are smaller than the proposed Project, the Donald McKay School and the Gumball Factory Residential Building, as well as several other four-story residential buildings are similar to the proposed Project in size and scope. Overall, the Project team feels that given this location and the immense size of the lots, and the structures influencing the design, that the proposed building's height, mass and scale are appropriate for this location.

Table 2-2 Gove Street Zoning Compliance

Categories	Multi-Family Residential Subdistrict	Proposed Project*
Minimum Lot Area (Square Feet)	2,000 sf For First 2 Units	16,737 sf
Lot Area for Each Additional Dwelling Unit (Square Feet)	1,000 sf per DU (12,000)	14,737 sf
Floor Area Ratio	1.0	
Minimum Lot Width	40 Feet	152 Feet
Minimum Lot Frontage	40 Feet	130 Feet

Table 2-2 Gove Street Zoning Compliance (Continued)

Categories	Multi-Family Residential Subdistrict	Proposed Project*
Minimum Front Yard	5 Feet	5 Feet
Minimum Side Yard	5 Feet	7-35 Feet
Minimum Rear Yard	30 Feet	0-5 Feet
Maximum Building Height	3 Stories/35 Feet	Up to 55 Feet, 0 Inches
Minimum Useable Open Space Per Dwelling Unit (Square Feet)	200 sf / Unit (2,800)	750 – 1325 sf / Unit
Off-Street Parking Spaces	2 Space per Dwelling Unit (28 Spaces) *Negated by Article 80 Large Project Review	13 Spaces

^{*}The proposed Church Building renovation includes an existing, non-conforming condition for building height and front/side/rear yard dimensions.

Table 2-3 Frankfort Street – Zoning Compliance

Categories	Multi-Family Residential Subdistrict	Proposed Project*
Minimum Lot Area (Square Feet)	2,000 sf For First 2 Units	32,403 sf
Lot Area for Each Additional Dwelling Unit (Square Feet)	1,000 sf per DU (96,000)	30,403 sf
Floor Area Ratio	1.0	
Minimum Lot Width	40 Feet	350 Feet
Minimum Lot Frontage	40 Feet	350 Feet
Minimum Front Yard	5 Feet	5 Feet
Minimum Side Yard	5 Feet	7-35 Feet
Minimum Rear Yard	30 Feet	0 to5 Feet
Maximum Building Height	3 Stories/35 Feet	Up to 65 Feet, 0 Inches
Minimum Useable Open Space Per Dwelling Unit (Square Feet)	200 sf / Unit (19,600)	660 – 1320 sf / Unit
Off-Street Parking Spaces	2 Space per Dwelling Unit (196 Spaces) *Negated by Article 80 Large Project Review	71 Spaces

^{*}The proposed Church Building renovation includes an existing, non-conforming condition for building height and front/side/rear yard dimensions.

2.2.2 Article 80 – Large Project Review

The proposed Project is subject to review by the BPDA pursuant to Article 80B, Large Project Review of the Code. The Project will require Zoning Relief in the form of variances, conditional use permits or other form of Zoning Relief as to be determined through the development review process of Article 80 of the Code.

2.3 Anticipated Permits

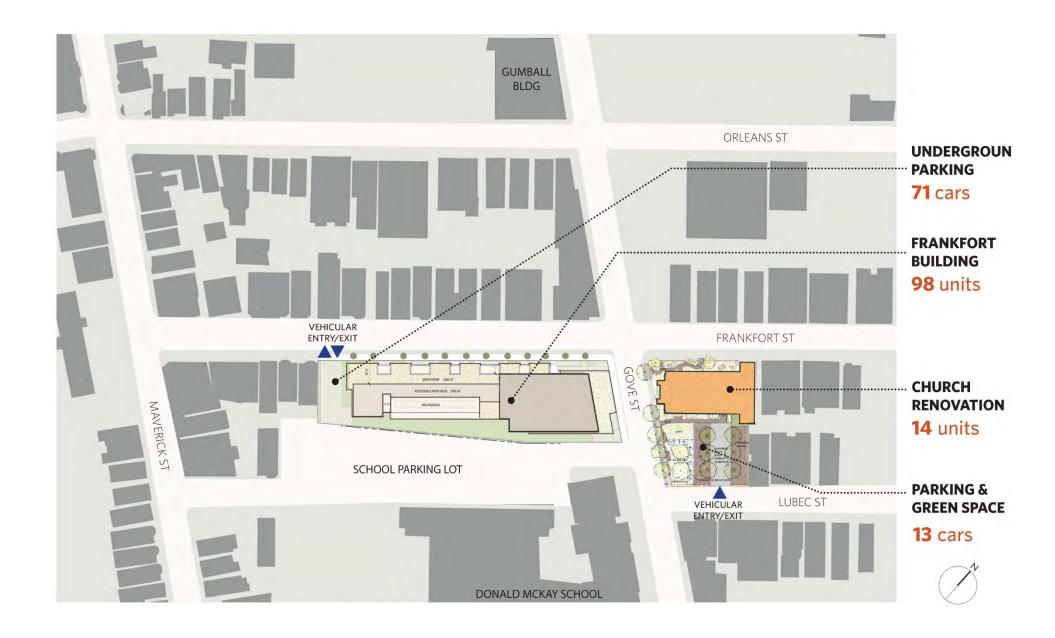
Table 2-4 represents a preliminary list of permits and approvals from governmental agencies that are expected to be required for the Project, based on currently available information. It is possible that only some of these permits or actions will be required, or that additional permits or actions will be required.

Table 2-4 Anticipated Permits and Approvals

Agency	Permit / Approval	
Local		
Boston Planning & Development Agency	Article 80 Review and Execution of Related Agreements; Section 80B-6 Certificate of Compliance	
Boston Transportation Department	Transportation Access Plan Agreement; Construction Management Plan	
Boston Department of Public Works, Public Improvement Commission	Possible Sidewalk Repair Plan; Curb-Cut Permit; Street/Sidewalk Occupancy Permit; Other	
Boston Zoning Board of Appeals	Possible Variances and Dimensional Relief from Existing Zoning Code Requirements	
Boston Public Safety Commission, Committee on Licenses	Permit for Storage of Fuel in (Emergency Storage) Tanks; Garage Licenses	
Boston Fire Department	Approval of Fire Safety Equipment	
Boston Water and Sewer	Approval for Sewer and Water Connections; Construction Site Dewatering; and Storm Drainage	
Boston Parks Department	Approval for Site Location in Relation to Nearby Parks	
Boston Department of Inspection Services	Building Permits; Certificates of Occupancy; Other Construction-Related Permits	
Interagency Green Building Committee	Zoning Article 37 Compliance	
State		
Department of Environmental Protection	Notification of Demolition and Construction	
Federal		
U.S. Environmental Protection Agency	NPDES Notice of Intent for Construction	

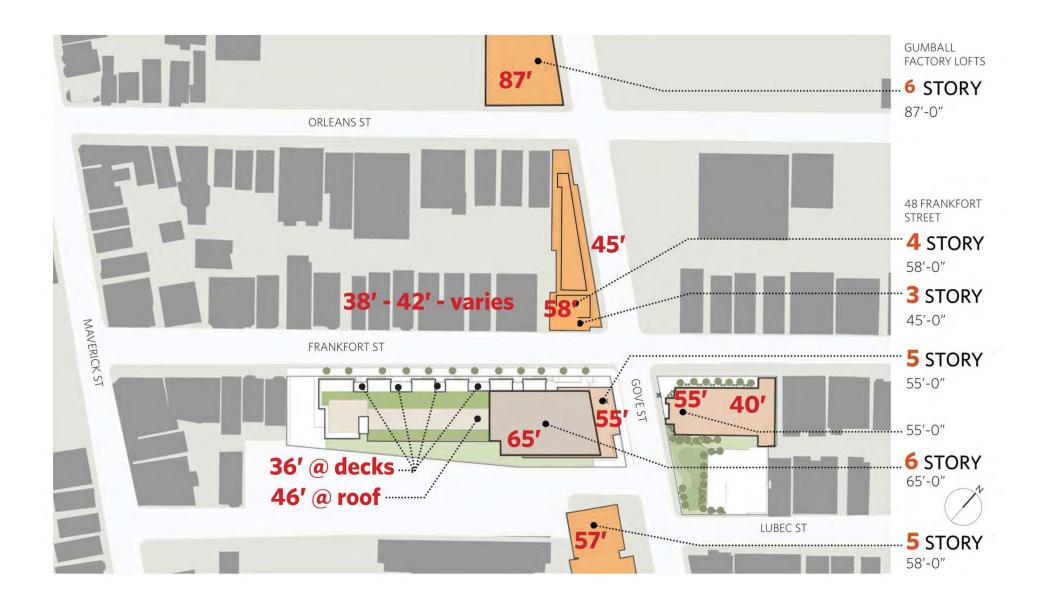
2.4 Schedule

Construction of the Project is expected to commence in the third quarter of 2019 and is expected to be completed in the first quarter of 2021.



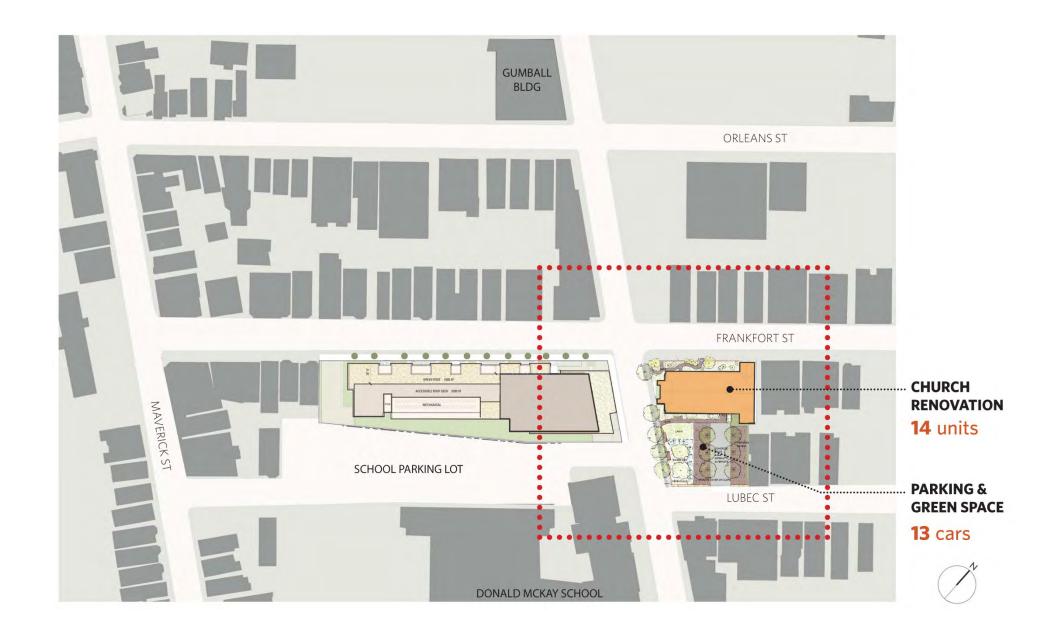
Frankfort Gove Street Housing Bo

Boston, Massachusetts



Frankfort Gove Street Housing

Boston, Massachusetts

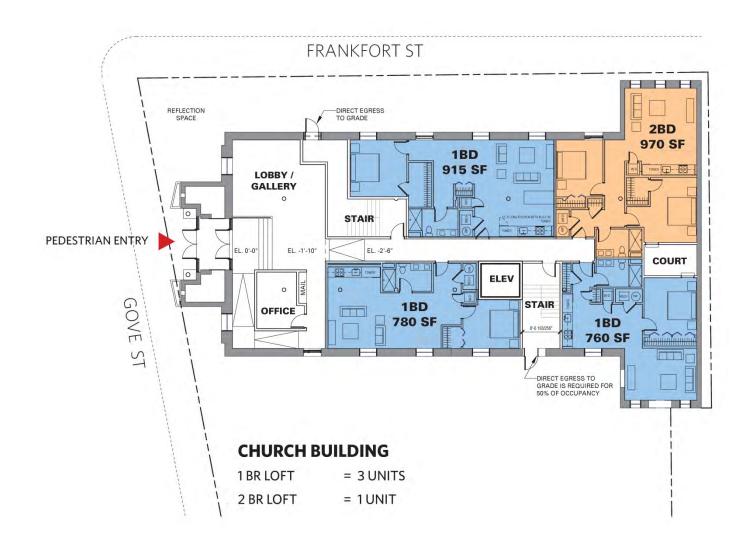


Frankfort Gove Street Housing

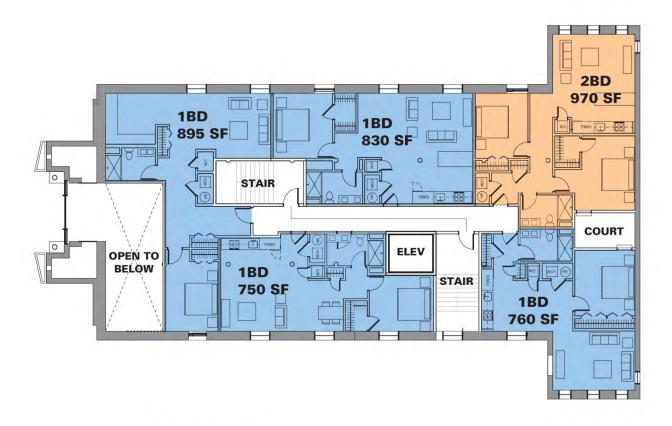
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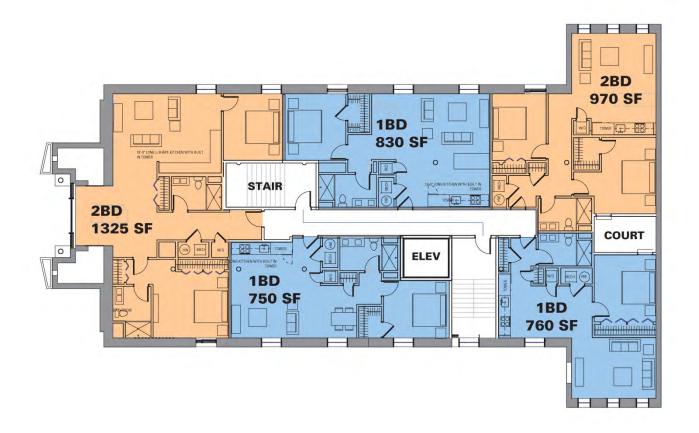


CHURCH BUILDING

1 BR LOFT = 4 UNITS

2 BR LOFT = 1 UNIT



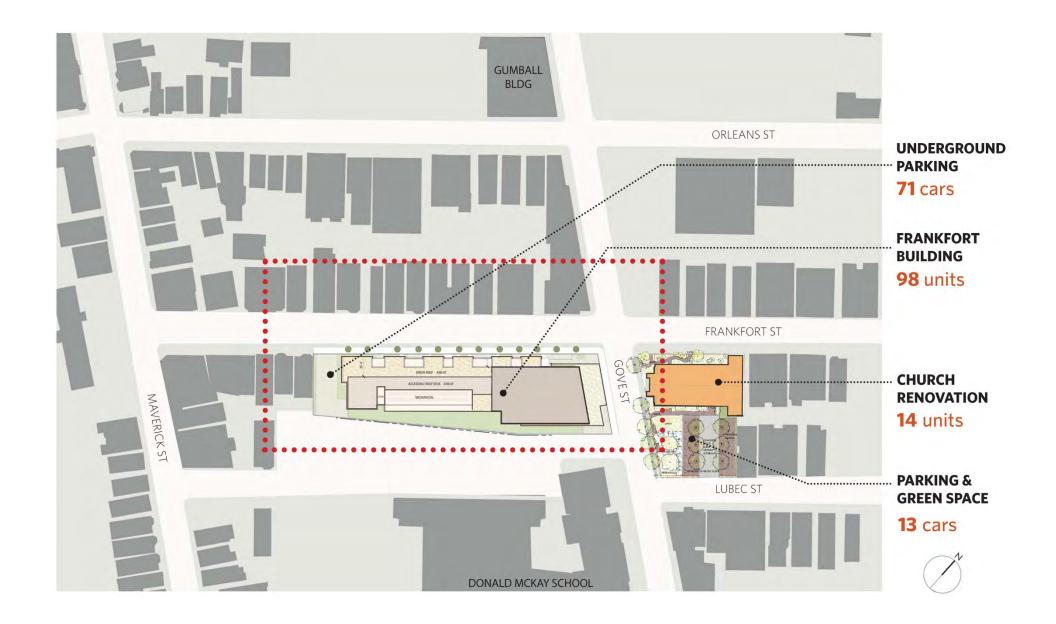


CHURCH BUILDING

1 BR LOFT = 3 UNITS

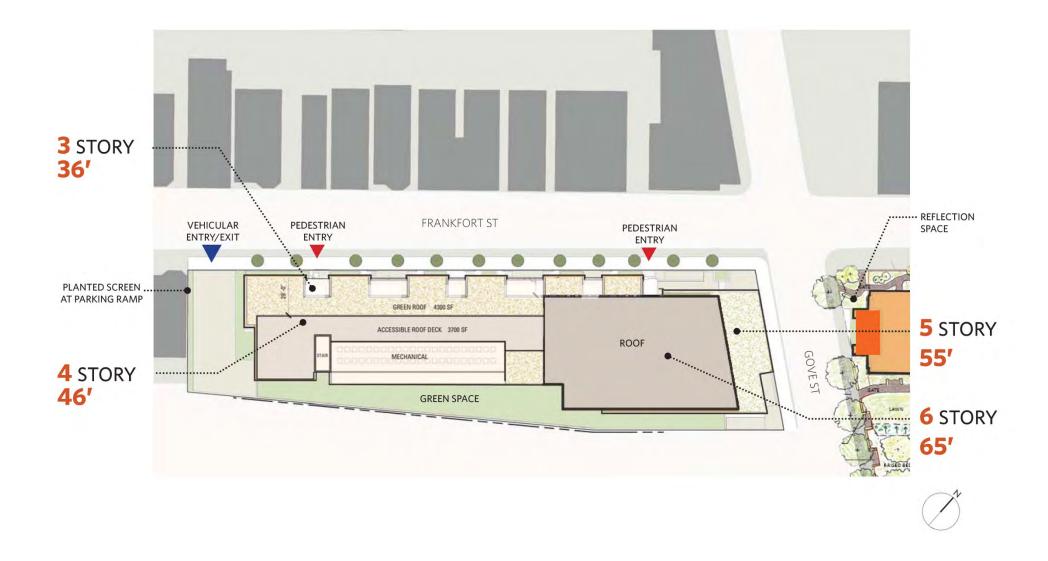
2 BR LOFT = 2 UNIT



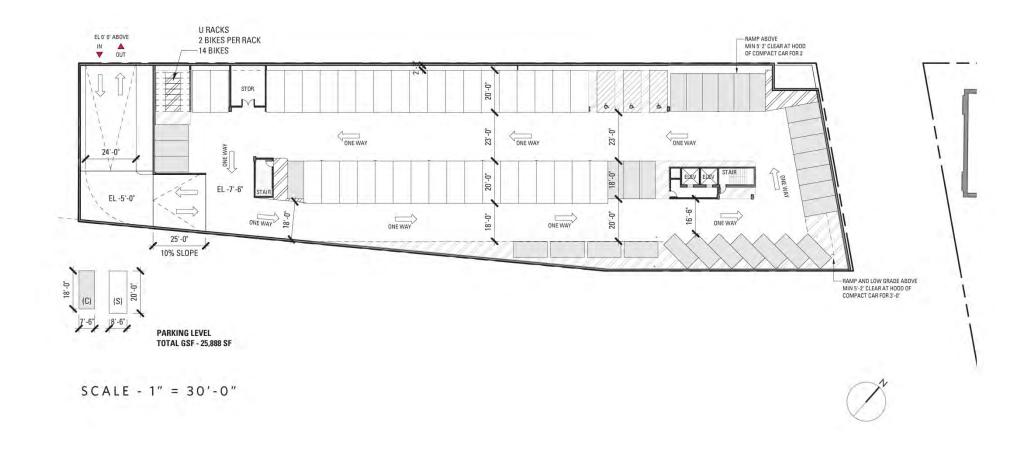


Frankfort Gove Street Housing

Boston, Massachusetts









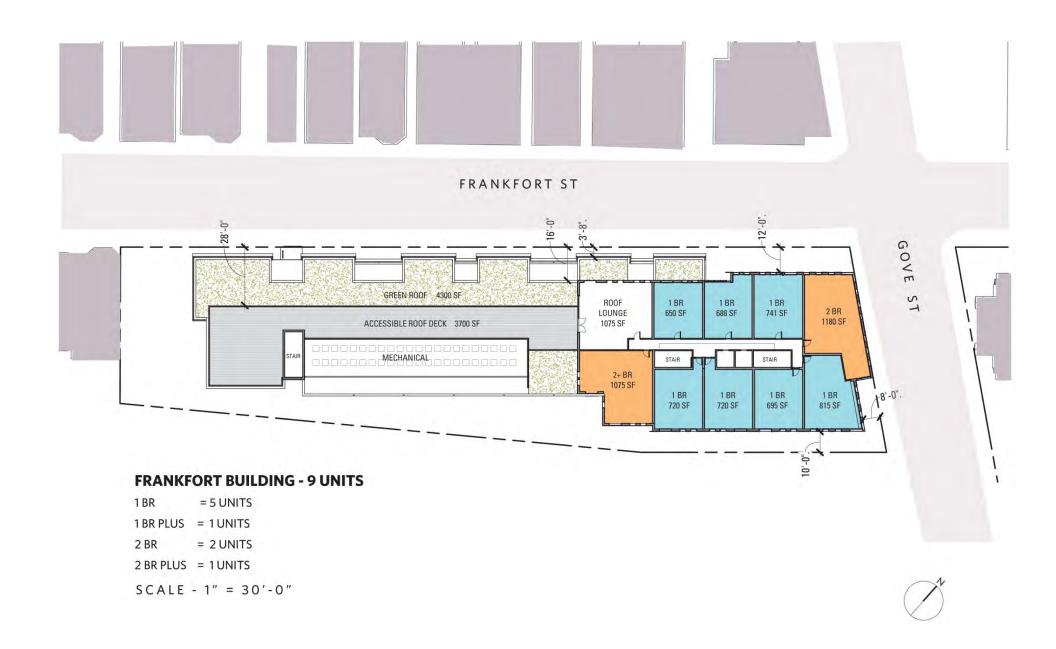




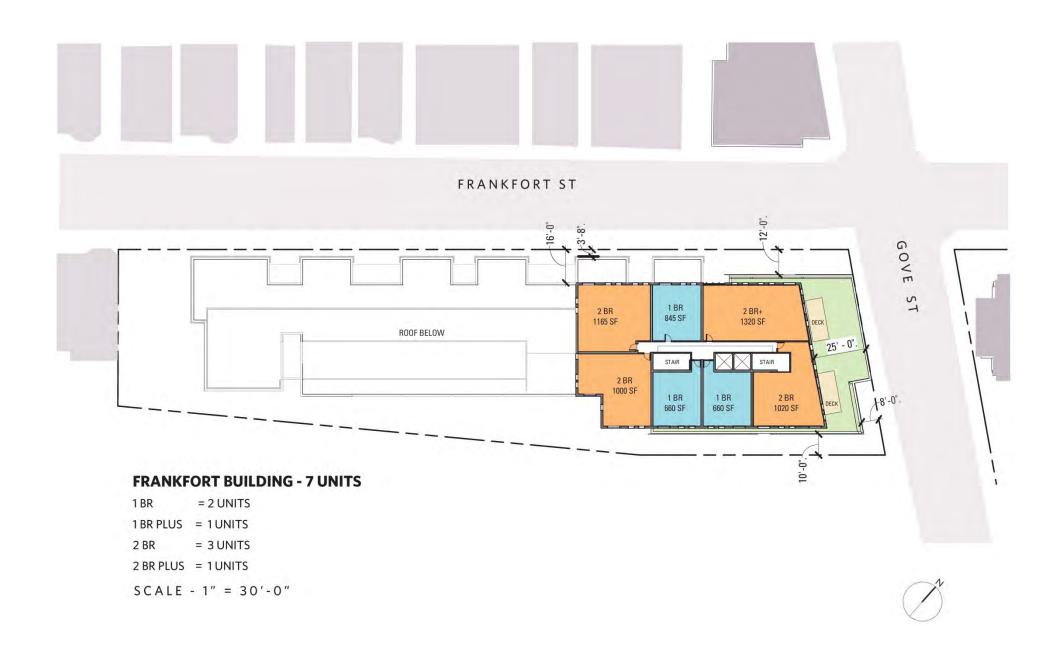








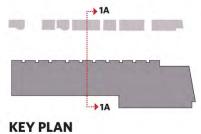


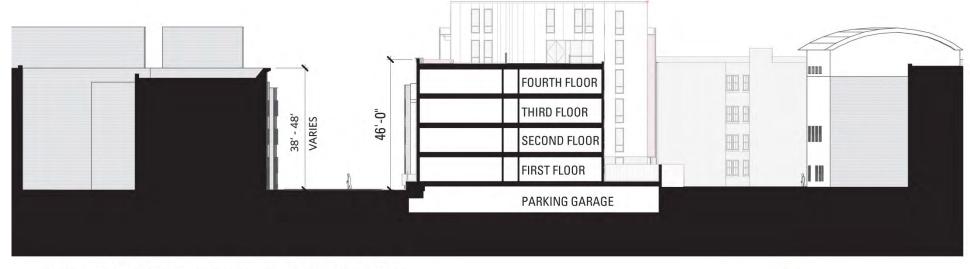




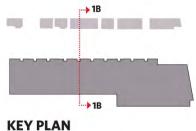


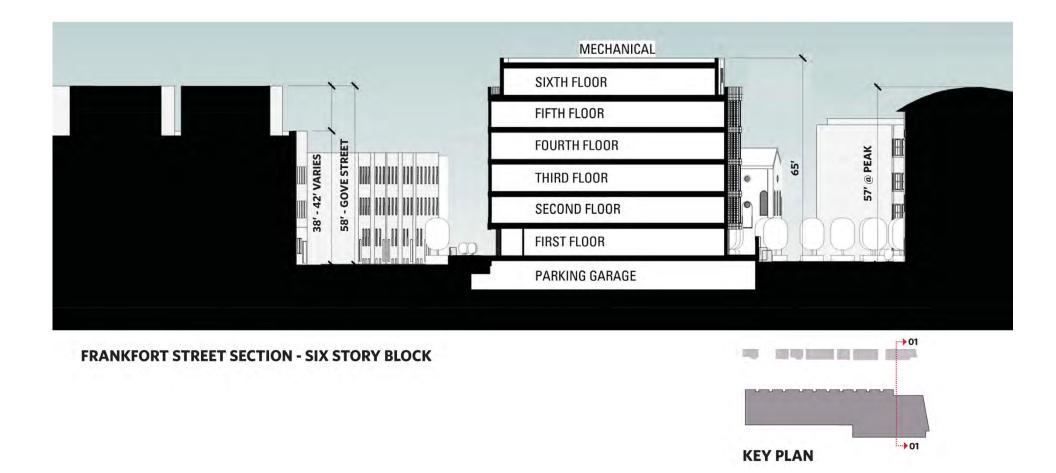
FRANKFORT STREET SECTION - FOUR STORY BLOCK

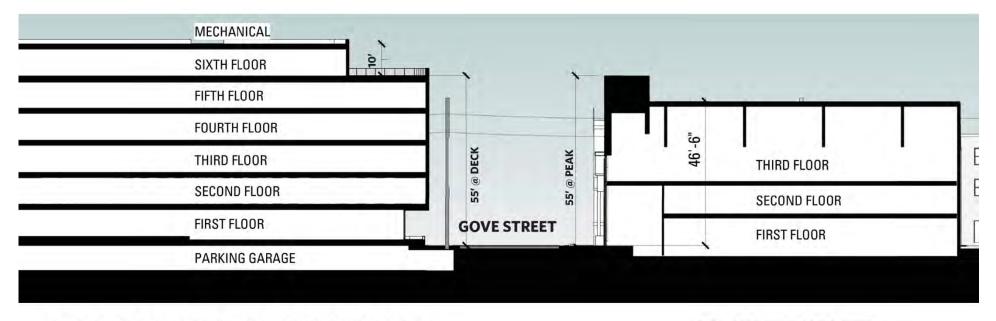




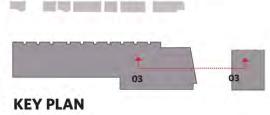
FRANKFORT STREET SECTION - FOUR STORY BLOCK







FRANKFORT STREET SECTION - ACROSS GOVE STREET



Chapter 3

Transportation

3.0 TRANSPORTATION

3.1 Introduction

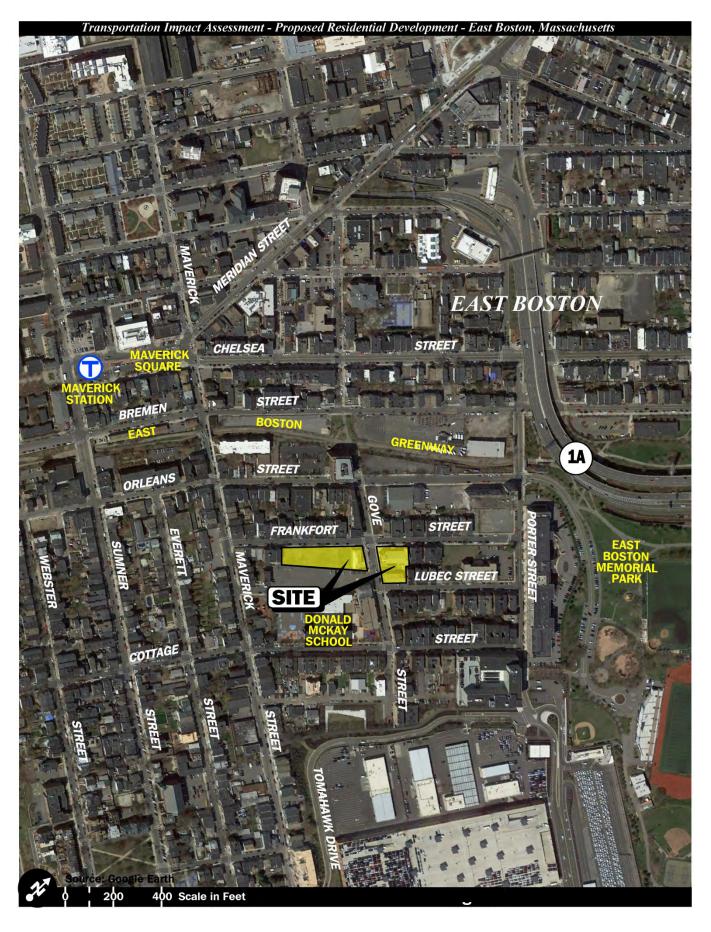
Vanasse & Associates, Inc. (VAI) has prepared this section in order to determine the potential impacts on the transportation infrastructure associated with the proposed construction of a multi-family residential community at the location of the former Mt. Our Lady of Mt. Carmel Church and rectory located off Gove Street and Frankfort Street in the East Boston Neighborhood of Boston, Massachusetts. Since the completion of the assessment, a new building that was originally programmed for the rectory site has been replaced with green space and 13 parking spaces. As a result of these changes, the initial development program that was to include 115 residential units has been reduced to 112 units and the overall number of parking spaces has increased from 82 to 84. This slight reduction in the number of residential units does not a have a material impact on the analysis results or the findings that are presented herein, which are based on the original 115 unit development program.

3.2 Project Description

The Project will entail the construction of a 112-unit multi-family residential community at the location of the former Our Lady of Mt. Carmel Church and rectory located off Gove Street and Frankfort Street in the East Boston Neighborhood of Boston, Massachusetts. The Project area encompasses several parcels of land which are shown on Figure 3-1 (collectively, Project site).

The north parcel contains the Church Building and is bounded by residential properties to the north, Gove Street to the south, Lubec Street to the east and Frankfort Street to the west. The former Church Building will be renovated and expanded to encompass 14 residential units. A surface parking lot will be provided that will accommodate parking for 13 vehicles and will be accessed from a new driveway that will intersect the west side of Lubec Street approximately 50-feet north of Gove Street.

The south parcel includes the former church Rectory Building and the paved parking lot to the south of the building. These existing features will be removed to accommodate the construction of a new six-story building that will encompass 98 residential units. Parking will be provided for 71 vehicles in a garage to be located beneath the residential building and will be accessed by way of a new driveway that will intersect the east side of Frankfort Street at the south end of the parcel.



Frankfort Gove Boston, Massachusetts



In total (north and south parcels), the Project will provide on-site parking for 84 vehicles, or an approximate parking ratio of 0.75 spaces per unit, which is within the maximum parking ratio range recommended by BTD for a residential development in the East Boston Neighborhood that is located within a ten-minute walk of a transit station.¹

3.3 Study Methodology

This study was prepared in consultation with the Boston Planning and Development Agency (BPDA), BTD and the Massachusetts Department of Transportation (MassDOT); was performed in accordance with the scope of work identified by BTD for the Project, MassDOT's Transportation Impact Assessment (TIA) Guidelines, and the standards of the Traffic Engineering and Transportation Planning professions for the preparation of such reports; and was conducted in three distinct stages.

The first stage involved an assessment of existing conditions in the study area and included an inventory of roadway geometrics; pedestrian and bicycle facilities; on-street parking; public transportation services; observations of traffic flow; and collection of pedestrian, bicycle and vehicle counts.

In the second stage of the study, future traffic conditions were projected and analyzed. Specific travel demand forecasts for the Project were assessed along with future traffic demands due to expected traffic growth independent of the Project. A seven-year time horizon from the current year was selected for analyses consistent with MassDOT's Transportation Impact Assessment (TIA) Guidelines. The analysis conducted in stage two identifies existing or projected future capacity, safety, and access issues, as these areas relate to the transportation infrastructure.

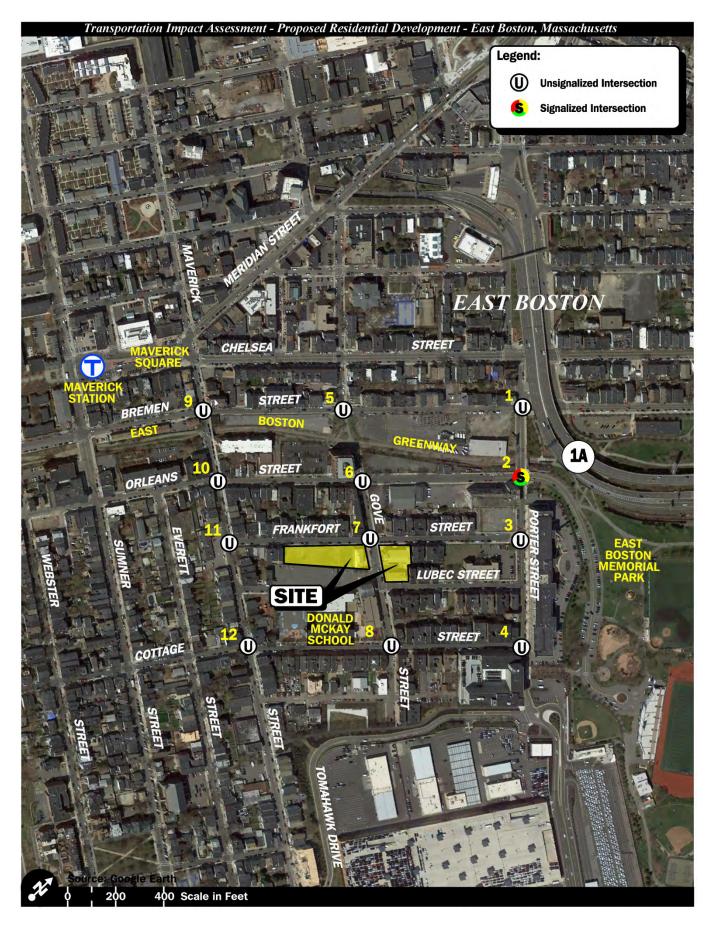
The third stage of the study presents and evaluates measures to address deficiencies in the transportation infrastructure, if any, identified in stage two of the study.

3.4 Existing Conditions

A comprehensive field inventory of existing conditions within the study area was conducted in January 2018. The field investigation consisted of an inventory of existing roadway geometrics; pedestrian and bicycle facilities; on-street parking; public transportation services; traffic volumes; and operating characteristics; as well as posted speed limits and land use information within the study area. The study area for the Project was developed in consultation with BTD and selected to contain the major roadways providing access to the Project site, including Gove Street, Frankfort Street, Porter Street and Maverick Street, as well as the following specific intersections which are depicted on Figure 3-2:

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Guidelines by the Boston Transportation Department for use by the Zoning Board of Appeal, Section 5, Parking Ratios.



Frankfort Gove Boston, Massachusetts



- 1. Porter Street at Bremen Street
- Porter Street at Orleans Street
- Porter Street at Frankfort Street
- 4. Porter Street at Cottage Street
- 5. Gove Street at Bremen Street
- 6. Gove Street at Orleans Street
- 7. Gove Street at Frankfort Street
- 8. Gove Street at Cottage Street
- 9. Maverick Street at Bremen Street
- 10. Maverick Street at Orleans Street
- 11. Mayerick Street at Frankfort Street
- 12. Maverick Street at Cottage Street

The following describes the study area roadways and intersections as observed in January 2018.

Roadways

Gove Street

- Two-lane roadway under City jurisdiction
- Traverses the study area in a general northwest-southeast direction between
- Orleans Street and Geneva Street
- Provides a 33-foot wide traveled-way that accommodates two-way travel with no marked centerline, regulated on-street parking and a sidewalk along both sides
- ◆ Citywide regulated travel speed pursuant to M.G.L. c. 90 § 17C is 25 miles per hour (mph)
- Illumination is provided by way of street lights mounted on wood or concrete poles
- Land use within the study area consists of the Project site, residential and commercial properties, the Donald McKay School and the East Boston Early Childhood Center

Frankfort Street

- ♦ Two-lane roadway under City jurisdiction
- ◆ Traverses the study area in a general northeast-southwest direction between Maverick Street and Porter Street
- Provides a 34-foot wide traveled-way that accommodates two-way travel with no marked centerline, regulated on-street parking and a sidewalk along both sides
- ♦ Citywide regulated travel speed is 25 mph
- Illumination is provided by way of street lights mounted on concrete poles
- ◆ Land use within the study area consists of the Project site and residential and commercial properties

Porter Street

- Two-lane urban minor arterial roadway under City jurisdiction
- ♦ Traverses the study area in a general northwest-southeast direction between
- Havre Street and Transportation Way; one-way in a southeast bound direction between Havre Street and Bremen Street
- Provides two 17-foot wide travel lanes separated by a double-yellow centerline with regulated on-street parking and a sidewalk along both sides
- Porter Street is closed by means of a gate at Cottage Street that restricts access to the east (Logan International Airport)
- Citywide regulated travel speed is 25 mph
- Illumination is provided by way of street lights mounted on wood or concrete poles
- Land use within the study area consists of residential and commercial properties

Maverick Street

- ◆ Two-lane urban minor arterial roadway under City jurisdiction
- Traverses a one-way northwest bound alignment between Tomahawk Drive and New Street
- Provides a 34-foot wide traveled-way that accommodates two-way travel with no marked centerline, regulated on-street parking and a sidewalk along both sides

- ♦ Citywide regulated travel speed is 25 mph
- Illumination is provided by way of street lights mounted on concrete poles
- ◆ MBTA bus Route 120, Orient Heights Maverick Station, provides service along Maverick Street with multiple curbside stops
- ◆ Land use within the study area consists of residential and commercial properties, and Maverick Station on the Massachusetts Bay Transportation Authority (MBTA) Blue Line subway system

Intersections

Table 3-1 and Figure 3-3 summarize lane use, traffic control, and pedestrian and bicycle accommodations at the study area intersections as observed in January 2018.

Table 3-1 Study Area Intersection Description

Intersection	Traffic Control Type ^a	No. of Travel Lanes Provided	Shoulder Provided? (Yes/No/Width)	Pedestrian Accommodations? (Yes/No/Description)	Bicycle Accommodations? (Yes/No/Description)
Porter St./ Bremen St.	S	1 lane on all approaches; Porter St. is one-way eastbound	No - on-street parking is permitted on all legs except on Bremen St. north of the intersection and Porter St. east of the intersection	Yes – sidewalks along both sides of the intersecting roadways; crosswalks provided for crossing both legs of Porter St. and the south leg of Bremen St.	Yes - Shared traveled-way ^b
Porter St./ Orleans St./ East Boston Memorial Pk Dwy	TS	1 lane on all approaches	No - on-street parking is permitted along both sides of Orleans St. and Porter St. east of the intersection	Yes – sidewalks along both sides of Porter St., Orleans St. and west side of East Boston Memorial Pk dwy; crosswalks across all legs; pedestrian traffic signal equipment and concurrent phasing provided	Yes - Shared traveled-way
Porter St./ Frankfort St.	S	1 lane on all approaches	No - on-street parking is permitted on all legs	Yes – sidewalks along both sides of Porter St. and Frankfort St.; crosswalk provided for crossing Frankfort St.	Yes - Shared traveled-way

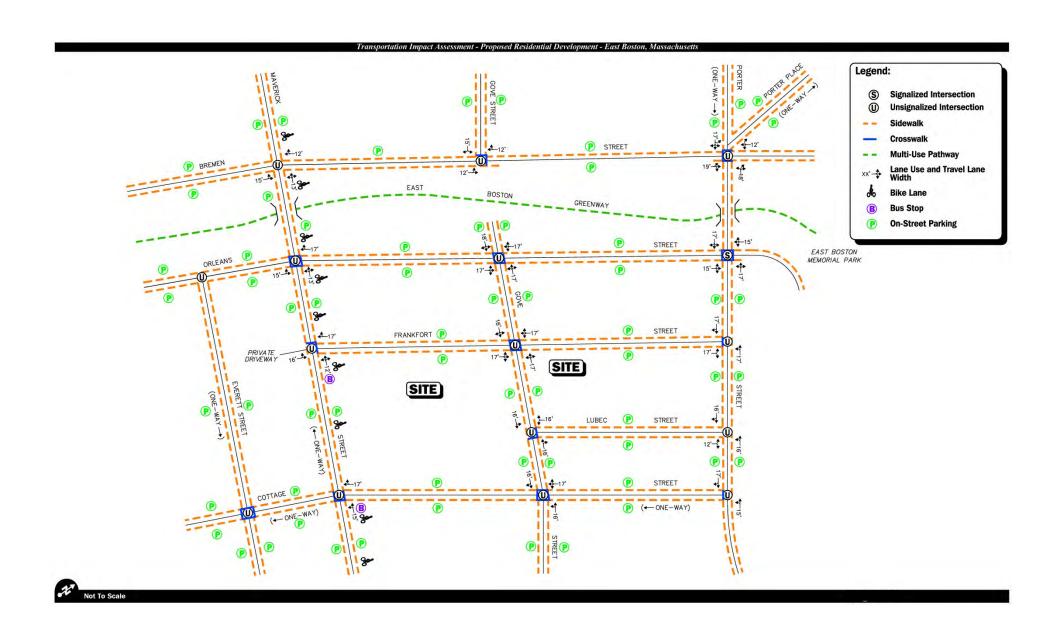
Table 3-1 Study Area Intersection Description (Continued)

Intersection	Traffic Control Type ^a	No. of Travel Lanes Provided	Shoulder Provided? (Yes/No/Width)	Pedestrian Accommodations? (Yes/No/Description)	Bicycle Accommodations? (Yes/No/Description)
Porter St./ Cottage St.	S	1 lane on all approaches; Cottage St. is one-way southbound	No - on-street parking is permitted along both sides of Cottage St. and Porter St. west of the intersection	Yes – sidewalks along both sides of Porter St. and Cottage St.; crosswalk provided for crossing Cottage St.	Yes - Shared traveled-way
Bremen St./ Gove St.	S	1 lane on all approaches	No - on-street parking is permitted on all legs	Yes - sidewalks along both sides of intersecting roadways except the east side of Bremen St. north of the intersection; crosswalks across the north and west legs of the intersection; entrance to the East Boston Greenway from Gove St.	Yes - Shared traveled-way on Gove St.
Orleans St./ Gove St.	S	1 lane on all approaches	No - on-street parking is permitted on all legs	Yes - sidewalks along both sides of intersecting roadways; crosswalks across all legs	Yes - Shared traveled-way
Frankfort St./ Gove St.	S	1 lane on all approaches	No - on-street parking is permitted on all legs	Yes - sidewalks along both sides of intersecting roadways; crosswalks across all legs	Yes - Shared traveled-way
Cottage St./ Gove St.	S	1 lane on all approaches; Cottage St. is one-way southbound	No - on-street parking is permitted on all legs	Yes - along both sides of the interesting roadways; crosswalks across all legs	Yes - Shared traveled-way
Maverick St./ Bremen St.	S	1 lane on all approaches; Maverick St. is one-way westbound	No - on-street parking is permitted on all legs	Yes - sidewalks along both sides of the intersecting roadways	Yes - Shared traveled-way with bicycle lane on Maverick St.

Table 3-1 Study Area Intersection Description (Continued)

Intersection	Traffic Control Type ^a	No. of Travel Lanes Provided	Shoulder Provided? (Yes/No/Width)	Pedestrian Accommodations? (Yes/No/Description)	Bicycle Accommodations? (Yes/No/Description)
Maverick St./ Orleans St.	S	1 lane on all approaches; Maverick St. is one-way westbound	No - on-street parking is permitted on all legs	Yes - sidewalks along both sides of the intersecting roadways; crosswalks provided across all legs	Yes - Shared traveled-way with bicycle lane on Maverick St.
Maverick St./ Frankfort St.	S	1 lane on all approaches; Maverick St. is one-way westbound	No - on-street parking is permitted on all legs	Yes - sidewalks along both sides of intersecting roadways; crosswalks across Frankfort St. and the west leg of Maverick St.; bus stop located on the north side of Maverick St. east of the intersection	Yes - Shared traveled-way with bicycle lane on Maverick St.
Maverick St./ Cottage St.	S	1 wide lane on all approaches; Maverick St. is one-way westbound and Cottage St. is one-way southbound	No - on-street parking is permitted on all legs	Yes - sidewalks along both sides of intersecting roadways; crosswalks across the north, south and east legs of the intersection; bus stop located on the north side of Maverick St. east of the intersection	Yes - Shared traveled-way with bicycle lane on Maverick St.

^aTS = traffic signal control; S = STOP-sign control; Y = YIELD-sign control; R = rotary/roundabout control; NC = no control present. ^bCombined shoulder and travel lane width equal to or exceed 14 feet.







3.4.1 Existing Traffic Volumes

In order to determine existing traffic-volume demands and flow patterns within the study area, manual turning movement counts (TMCs) and vehicle classification counts were completed at the study intersections in January 2018 while public schools were in regular session. The TMC's were performed during the weekday morning (7:00 to 9:00 AM) and evening (4:00 to 6:00 PM) peak periods on January 25, 2018 (Thursday). These time periods were selected for analysis purposes as they are representative of the peak-traffic-volume hours for both the Project and the adjacent roadway network.

3.4.2 Traffic Volume Adjustments

In order to evaluate the potential for seasonal fluctuation of traffic volumes within the study area, traffic volume data from MassDOT Continuous Count Station No. 8087 located on Route 1A, north of the Boston/Revere line, were reviewed.² Based on a review of this data, it was determined that traffic volumes for the month of January are approximately 7.1 percent below average-month conditions and, therefore, the January traffic count data was adjusted upward to average-month conditions. The 2018 Existing weekday morning and evening peak-hour traffic volumes graphically depicted on Figures 3-4 and 3-5, respectively.

3.4.3 Pedestrian and Bicycle Facilities

A comprehensive field inventory of pedestrian and bicycle facilities within the study area was undertaken in January 2018. The field inventory consisted of a review of the location of sidewalks and pedestrian crossing locations along the study roadways and at the study intersections, as well as the location of existing and planned future bicycle facilities.

3.4.4.1 Pedestrian Facilities

As detailed on Figure 3-3, sidewalks are generally provided along one or both sides of the study area roadways, with marked crosswalks provided for crossing one or more approaches of the study intersections. The crossings at the signalized study area intersection (Porter Street/ Orleans Street) are included as a part of the traffic signal system at the intersections (pedestrian pushbuttons, signal indications and phasing (concurrent) are provided for the crossings). In addition, the East Boston Greenway, a multi-use trail that traverses an alignment from East Boston Piers Park (Marginal Street) to Neptune Road and Wood Island Station on the MBTA Blue Line subway system, is located to the south of the Project site and is accessible from Gove Street at Orleans Street.

² MassDOT Traffic Volumes for the Commonwealth of Massachusetts; 2018.

Transportation Impact Assessment - Proposed Residential Development - East Boston, Massachusetts GOVE STREET **1**5 ←156 STREET **1**58 **1**4−81 64_* 99 32 BREMEN 5_**→** 154**→** EAST BOSTON GREENWAY 998 STREET EAST BOSTON MEMORIAL PARK 129_* 1--3--ORLEANS 0.→ 100→ 20→ 332→ 39° ¢ STREET FRANKFORT 74_***** 5—***** 5...) 43...→ 7...→ SITE EVERETT STREET SITE LUBEC STREET 2907 5 5 17 ← 252 ← 15 STREET COTTAGE (← ONE-WAY) (4-ONE-WAY)



Note: Imbalances exist due to numerous curb cuts and side streets that are not shown.

Not To Scale



Transportation Impact Assessment - Proposed Residential Development - East Boston, Massachusett. GOVE STREET STREET 46**→** 196→ 24→ BREMEN EAST BOSTON GREENWAY 12.J 271.J 997 ← 0 -73 -11 STREET EAST BOSTON MEMORIAL PARK ORLEANS 1_* 81-* 16-* 233→ STREET FRANKFORT 44_***** 6¬ SITE SITE LUBEC STREET 21 166 ← 19 **1**02 **←**97 STREET COTTAGE (← ONE-WAY) (ONE-WAY)



Note: Imbalances exist due to numerous curb cuts and side streets that are not shown.

Not To Scale



Figures 3-6 and 3-7 depict the 2018 Existing weekday morning and evening peak-hour pedestrian volumes at the study area intersections, respectively, which were collected in conjunction with the January 2018 TMCs. A review of the pedestrian volume data at the study intersections indicates that the largest number of pedestrian crossings occurred at the Maverick Street/ Bremen Street intersection (proximate to Maverick Station on the MBTA Blue Line subway system) during both the weekday morning and evening peak hours (338 to 385 crossings were observed).

3.4.4.2 Bicycle Facilities

The study area roadways generally provide sufficient width to support bicycle travel in a shared traveled-way configuration³ given the low travel speed environment and relatively modest traffic volumes. A marked bicycle lane is provided along Maverick Street and Orleans Street is designated (by signs) as a bicycle route. In addition, as described in the previous section, the East Boston Greenway multi-use trail is accessible from Gove Street at Orleans Street.

Figures 3-8 and 3-9 depict the 2018 Existing weekday morning and evening peak-hour bicycle volumes at the study area intersections, respectively, which were collected in conjunction with the January 2018 TMCs. Given the seasonality of the bicycle count data (January), bicycle activity within the study area was found to be relatively modest, with bi-directional bicycle volumes found to range from approximately one to ten bicyclists during the peak hours.

3.4.4 Public Transportation

Public transportation services are provided within the study area by the MBTA (Blue Line subway and fixed-route bus service) and are accessible to residents of the Project.

Maverick Station on the Blue Line subway system is located at Maverick Square and is within a 7-minute walking distance of the Project site. MBTA bus Route 120, Orient Heights - Maverick Station, provides service along Maverick Street to both Maverick Station and Orient Heights Station on the Blue Line subway system, with a stop located at the Maverick Street/ Frankfort Street intersection which is within a two-minute walking distance of the Project site. Sidewalks and bicycle accommodations are provided along the study area roadways that link the Project site to both Maverick Station and the Route 120 bus stop.

The public transportation schedules and fare information are provided in Attachment B.

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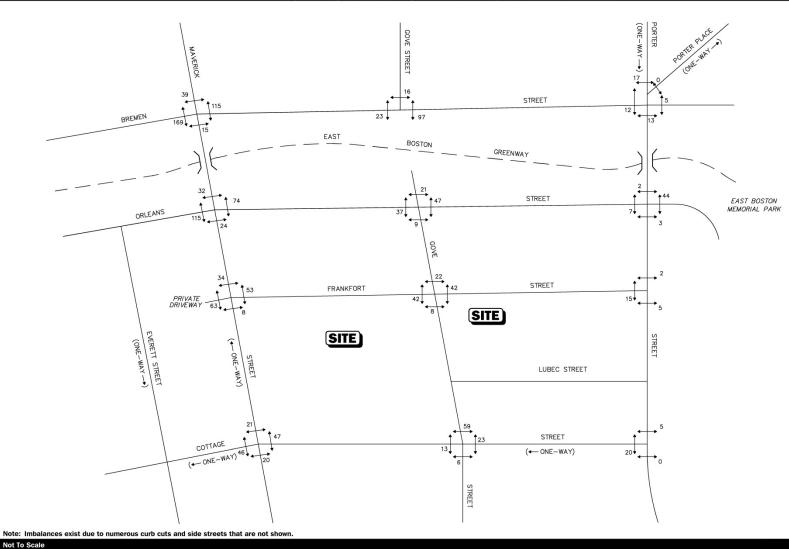
A minimum combined travel lane and paved shoulder width of 14-feet is required to support bicycle travel in a shared traveled-way condition.



Note: Imbalances exist due to numerous curb cuts and side streets that are not shown.

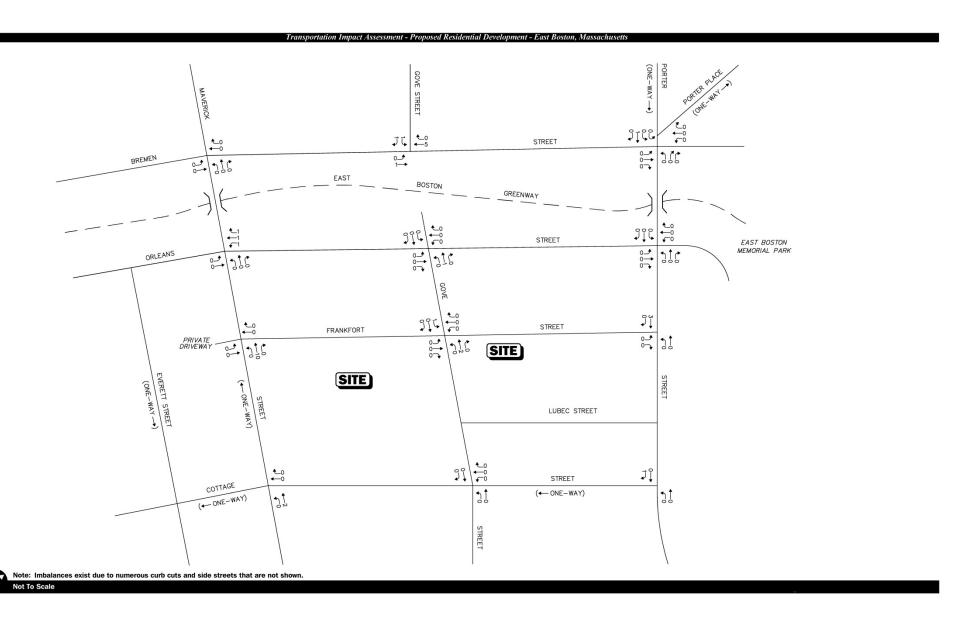
Not To Scale

















Transportation Impact Assessment - Proposed Residential Development - East Boston, Massachusett. GOVE STREET Ĵζ STREET BREMEN EAST BOSTON GREENWAY 310 000 ΙĮζ STREET EAST BOSTON MEMORIAL PARK 396 STREET FRANKFORT % <u>31</u> SITE SITE LUBEC STREET 37 **€**0 STREET COTTAGE (ONE-WAY) 1 (← ONE-WAY) Note: Imbalances exist due to numerous curb cuts and side streets that are not shown.





Not To Scale

3.4.5 Parking

An inventory of on-street parking accommodations within the study area was conducted in January 2018 in conjunction with the field inventories, and is depicted on Figure 3-10. The majority of the on-street parking within the study area consists of resident permit parking and/or 2-hour parking with a resident permit exclusion (i.e., vehicles with a resident parking permit can exceed the 2-hour parking restriction). No parking regulations were identified for the segments of Gove Street between Frankfort Street and Lubec Street (south side) and between Lubec Street and Cottage Street (north side). On-street parking is prohibited along both sides of Porter Street between Orleans Street and Bremen Street, along both sides of Porter Street north of Bremen Street, and along the east side of Bremen Street between Maverick Street and Gove Street.

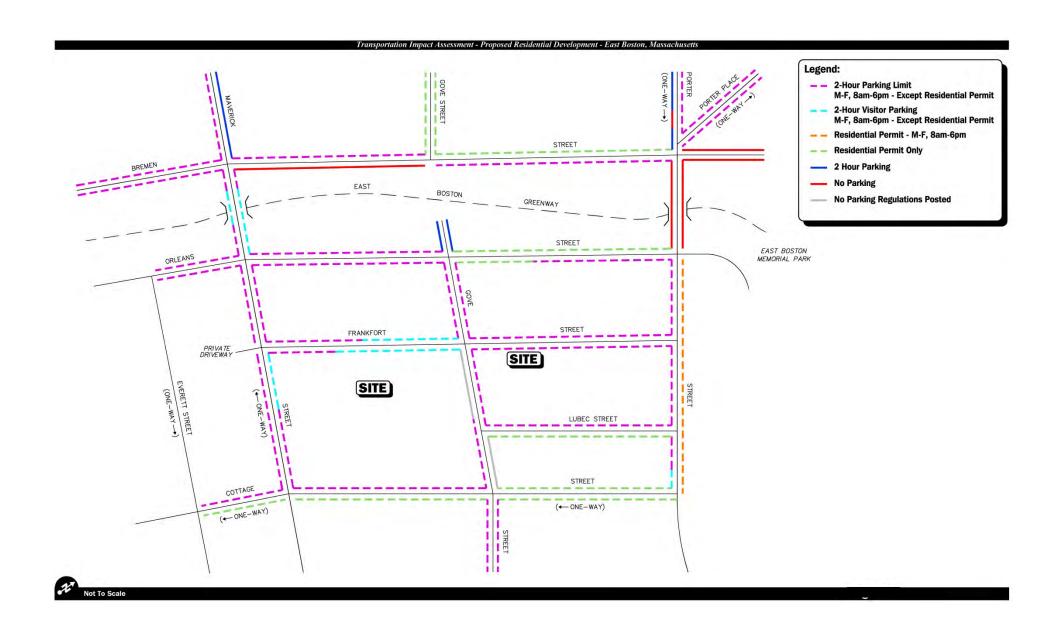
3.4.6 Motor Vehicle Crash Data

A review of the MassDOT statewide High Crash Location List indicated that there were no locations within the study area that were included on MassDOT's Highway Safety Improvement Program (HSIP) listing as a high crash cluster location for 2013-2015. MassDOT defines a HSIP eligible cluster as: "...a cluster in which the total number of 'equivalent property damage only' crashes is within the top 5 percent of all clusters in that region. 'Equivalent property damage only' is a method of combining the number of crashes with the severity of crashes based on a weighted scale where a fatal crash is worth 10, an injury crash is worth 5 and a property damage only crash is worth 1." Designation as a HSIP location allows for MassDOT to prioritize funding for safety-related improvements in a specific region of the state.

The MassDOT High Crash Location mapping is provided in Attachment B.

3.5 Future Conditions

Traffic volumes in the study area were projected to the year 2025, which reflects a seven-year planning horizon consistent with MassDOT's Transportation Impact Assessment (TIA) Guidelines. Independent of the Project, traffic volumes on the roadway network in the year 2025 under No-Build conditions include all existing traffic and new traffic resulting from background traffic growth. Anticipated Project-generated traffic volumes superimposed upon the 2025 No-Build traffic volumes reflect 2025 Build traffic volume conditions with the Project.







3.5.1 Future Traffic Growth

Future traffic growth is a function of the expected land development in the immediate area and the surrounding region. Several methods can be used to estimate this growth. A procedure frequently employed estimates an annual percentage increase in traffic growth and applies that percentage to all traffic volumes under study. The drawback to such a procedure is that some turning volumes may actually grow at either a higher or a lower rate at particular intersections.

An alternative procedure identifies the location and type of planned development, estimates the traffic to be generated, and assigns it to the area roadway network. This procedure produces a more realistic estimate of growth for local traffic; however, potential population growth and development external to the study area would not be accounted for in the resulting traffic projections.

To provide a conservative analysis framework, both procedures were used, the salient components of which are described below.

3.5.2 Specific Development by Others

The BPDA website and BTD were consulted in order to determine if there were any projects planned within the study area that would have an impact on future traffic volumes at the study intersections. Based on this consultation, the following projects were identified for inclusion in this assessment:

- ◆ 135 Bremen Street 94 residential units and 8,300 square feet (sf) of commercial space
- ♦ 31 Orleans Street 14 residential units
- ◆ 175 Orleans Street (Boston Loftel) 127 room hotel
- ♦ 202 Maverick Street 23 residential units
- ◆ 70 Bremen Street 32 residential units and 1,028 sf of commercial space
- ♦ 125 Summer Street 52 residential units and 3,400 sf of retail space
- ♦ 114 Orleans Street 23 residential units
- ♦ 91-111 Summer Street 119 residential units and 7,200 sf of commercial space
- ◆ 10-16 Everett Street 19 residential units

Traffic volumes associated with the aforementioned specific development projects by others were obtained from their respective traffic studies or using trip-generation information available from the Institute of Transportation Engineers (ITE)⁴ for the appropriate land use, and were assigned onto the study area roadway network based on existing traffic patterns where no other information was available. No other developments were identified at this time that are expected to result in an increase in traffic within the study area beyond the general background traffic growth rate.

3.5.3 General Background Traffic Growth

Traffic-volume data compiled by MassDOT from Continuous Count Station No. 8087 located on Route 1A, north of the Boston/Revere line, were reviewed. Based on a review of this data, it was determined that traffic volumes within the study area have generally increase by approximately 0.92 percent per year over the past several years. As such, a slightly higher 1.0 percent per year compounded annual background traffic growth rate was used in order to account for future traffic growth and presently unforeseen development within the study area.

3.5.4 Roadway Improvement Projects

MassDOT and BTD were contacted in order to determine if there were any planned future roadway improvement projects expected to be complete by 2025 within the study area. Based on these discussions, no roadway improvement projects aside from routine maintenance activities were identified to be planned within the study area at this time.

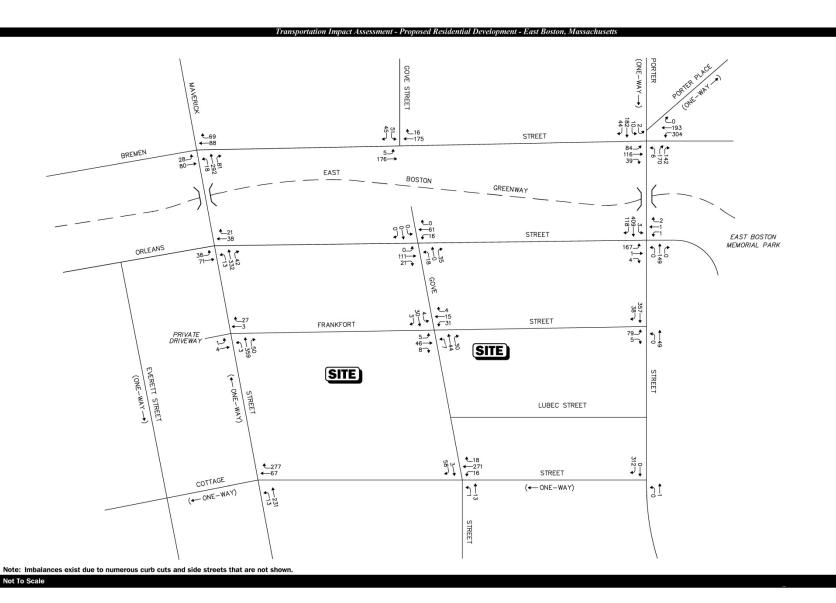
3.5.5 No-Build Traffic Volumes

The 2025 No-Build condition peak-hour traffic-volumes were developed by applying the 1.0 percent per year compounded annual background traffic growth rate to the 2018 Existing peak-hour traffic volumes and then adding the peak-hour traffic volumes associated with the identified specific development projects by others. The resulting 2025 No-Build weekday morning and evening peak-hour traffic volumes are shown on Figures 3-11 and 3-12, respectively.

3.5.6 Project-Generated Traffic

Design year (2025 Build) traffic volumes for the study area roadways were determined by estimating Project-generated traffic volumes and assigning those volumes on the study roadways. The following sections describe the methodology used to develop the anticipated traffic characteristics of the Project.

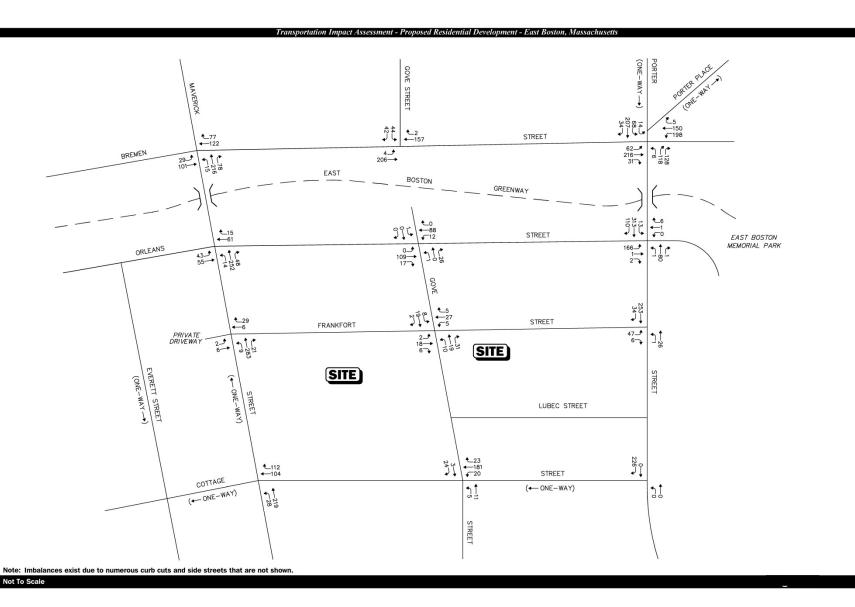
⁴ Ibid.





Boston, Massachusetts









As proposed, the Project will entail the construction of a 115-unit multi-family residential apartment community. In order to develop the traffic characteristics of the Project, tripgeneration statistics published by the ITE⁵ for a similar land use as that proposed were used. ITE Land Use Code (LUC) 221, Multifamily Housing (Mid-Rise), was used to develop the base traffic characteristics of the Project.

3.5.7 Alternative Modes of Transportation

Given the availability of public transportation services within walking distance of the Project site (MBTA bus and subway services) and the interconnected network of sidewalks and bicycle accommodations, it is expected that a portion of the residents of the Project will use public transportation services, walk or bicycle, thereby reducing the volume of traffic that may be associated with the Project. In order to determine the proportion of residents of the Project that may use public transportation, walk or bicycle as their primary mode of transportation, travel mode data obtained from BTD for Area 7, East Boston, and vehicle occupancy data obtained from the 2009 National Household Travel Survey were used. This data is summarized in Table 3-2.

Table 3-2 Travel Mode Data and Vehicle Occupancy Ratio

	Mod			
Time Period	Automobile (Enter/Exit)	Transit (Enter/Exit)	Pedestrian/ Bicycle (Enter/Exit)	Vehicle Occupancy Ratio ^b
Weekday Daily Weekday Morning Peak Weekday Evening Peak	54%/54% 51%/45% 45%/51%	17%/17% 15%/25% 25%/15%	29%/29% 34%/30% 30%/34%	1.13 1.13 1.13

^aBTD Mode Share for Area 7.

The base trip-generation calculations obtained using the ITE data were converted to person trips using the vehicle occupancy ratio (VOR) shown in Table 3-2 and were then disseminated to the available modes of transportation. The automobile person trips were converted back to vehicle trips by dividing by the VOR. Table 3-3 shows the resulting calculations for the Project using the above methodology.

^bPersons per vehicle. Source: Summary of Travel Trends: 2009 National Household Travel Survey; FHWA; Washington, D.C.; June 2011.

⁵ Ibid 1.

Table 3-3 Trip Generation Summary

			Person	Trips		
Time Period/Direction	ITE Vehicle Trips ^a	Total Person Trips ^b	Automobile Person Trips ^c	Transit Trips ^c	Pedestrian/ Bicycle Trips ^c	Automobile Trips ^d
Average Weekday Daily:						
Entering	313	354	191	60	103	169
<u>Exiting</u>	<u>313</u>	354	<u>191</u>	60	<u>103</u>	<u> 169</u>
Total	626	708	382	120	206	338
Weekday Morning Peak						
Hour:						
Entering	11	12	6	2	4	5
<u>Exiting</u>	30 41	$\frac{34}{46}$	<u>15</u> 21	<u>9</u> 11	<u>10</u> 14	<u>13</u> 18
Total	41	46	21	11	14	18
Weekday Evening Peak						
Hour:						
Entering	31	35	16	9	10	14
<u>Exiting</u>	<u>20</u> 51	<u>23</u> 58	<u>12</u>	$\frac{3}{12}$	<u>8</u> 18	<u>11</u>
Total	51	58	28	12	18	25

^aBased on ITE LUC 221, Multifamily Housing (Mid-Rise), and 115 units.

3.5.8 Project-Generated Trip Volume Summary

As can be seen in Table 3-3, after applying appropriate adjustments to account for the use of public transportation and pedestrian and bicycle trips, the Project is expected to generate approximately 338 automobile trips, 120 transit trips and 206 pedestrian/bicycle trips on an average weekday (two way, 24 hour volumes), with 18 automobile trips (5 vehicles entering and 13 exiting), 11 transit trips and 14 pedestrian/bicycle trips expected during the weekday morning peak-hour, and 25 automobile trips (14 vehicles entering and 11 exiting), 12 transit trips and 18 pedestrian/bicycle trips expected during the weekday evening peak-hour.

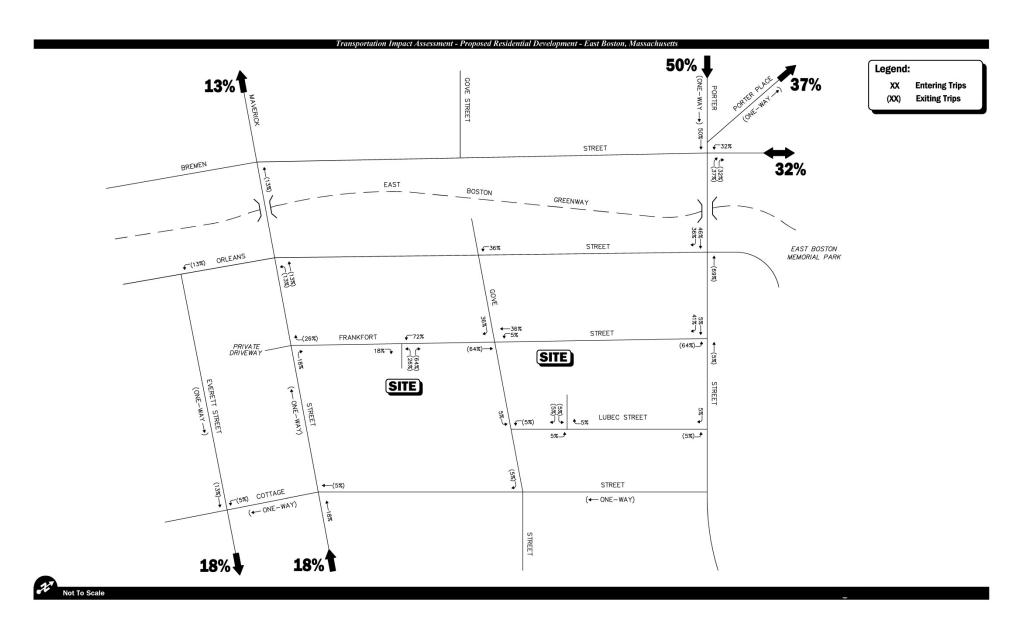
3.5.9 Trip Distribution and Assignment

The directional distribution of generated trips to and from the Project site was determined based on origin-destination data obtained from BTD for Area 7, East Boston. The general trip distribution for the Project is graphically depicted on Figure 3-13. The additional traffic expected to be generated by the Project was assigned on the study area roadway network as shown on Figures 3-14 and 3-15.

^bITE vehicle trips x 1.13 persons per vehicle.

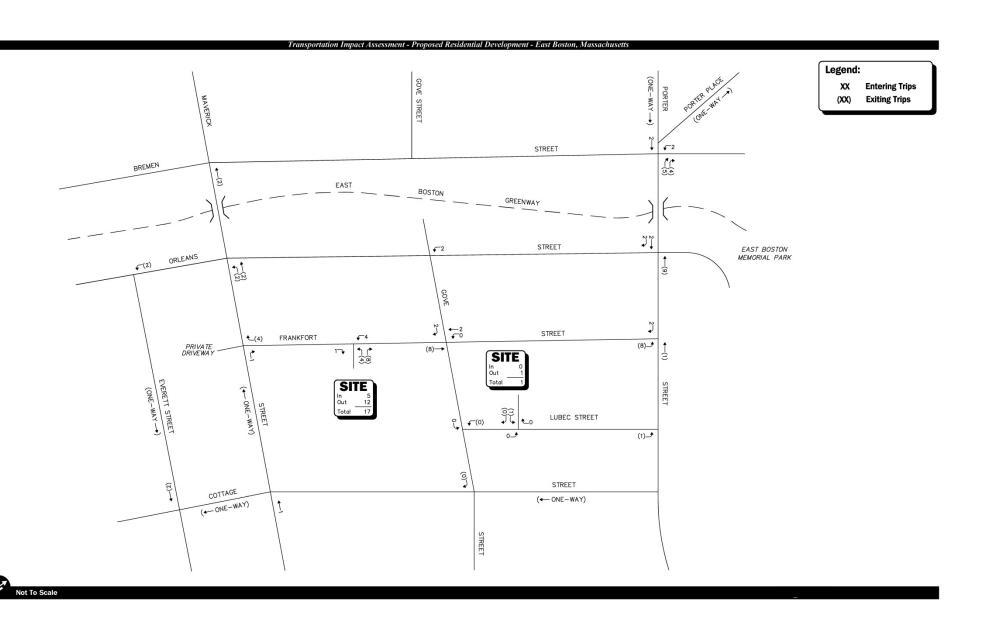
^cTotal person trips x BTD Mode Share for Area 7 (Table 2).

^eAutomobile person trips divided by 1.13.



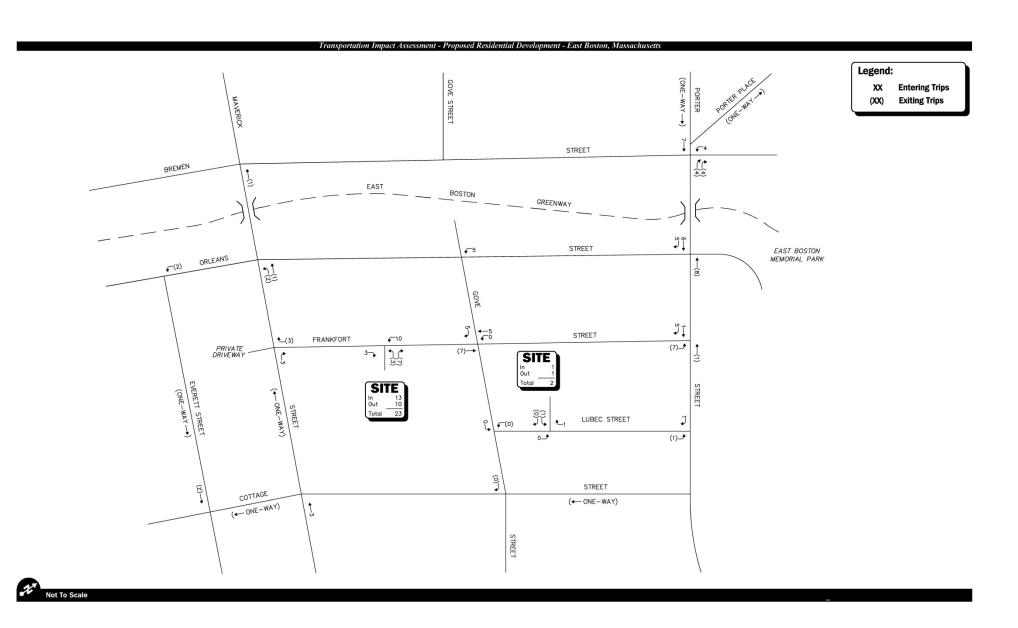
Frankfort Gove Street Housing Boston, Massachusetts















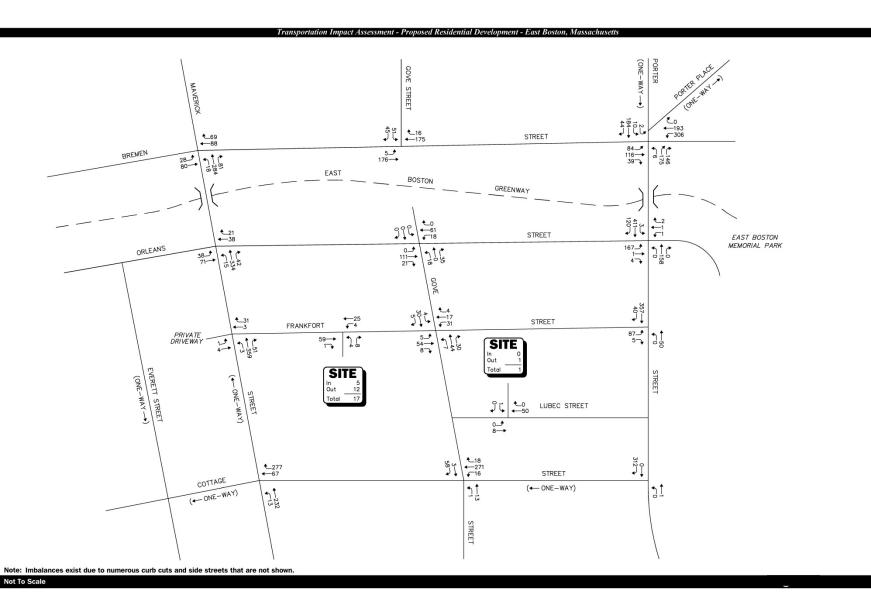
3.5.10 Future Traffic Volumes – Build Condition

The 2025 Build condition traffic volumes were developed by adding the traffic expected to be generated by the Project to the 2025 No-Build traffic volumes. The resulting 2025 Build peak-hour traffic-volumes are graphically depicted on Figures 3-16 and 3-17.

A summary of peak-hour projected traffic-volume increases outside of the study area that is the subject of this assessment is shown in Table 3-4. These volumes are based on the expected increases from the Project.

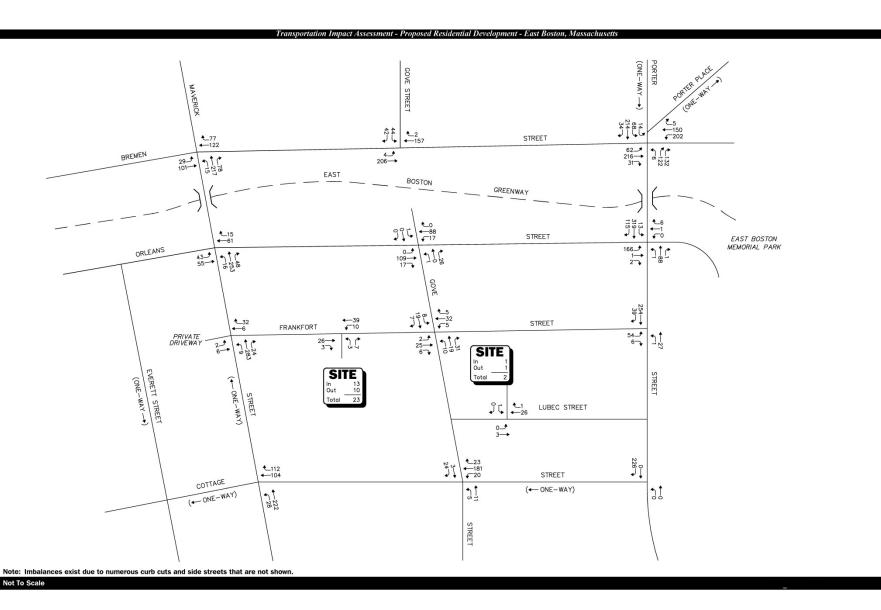
Table 3-4 Peak Hour Traffic – Volume Increases

		2025		Traffic Volume Increase Over	Percent Increase
Location/Peak Hour	2018 Existing	No- Build	2025 Build	No- Build	Over No-Build
Bremen Street, north of Porter Street:					
Weekday Morning	696	765	771	6	0.8
Weekday Evening	691	765	773	8	1.0
Bremen Street, south of Maverick Street:					
Weekday Morning	192	214	214	0	0.0
Weekday Evening	234	267	267	0	0.0
Orleans Street, south of Maverick Street:					
Weekday Morning	145	160	162	2	1.3
Weekday Evening	152	173	175	2	1.2
Maverick Street, east of Cottage Street:					
Weekday Morning	219	244	245	1	0.4
Weekday Evening	218	247	250	3	1.2
Maverick Street, west of Bremen Street:					
Weekday Morning	353	389	391	2	0.5
Weekday Evening	293	322	323	1	0.3
Porter Street, west of Bremen Street:					
Weekday Morning	181	238	240	2 7	0.8
Weekday Evening	252	323	330	7	2.2













As shown in Table 3-4, Project-related traffic-volume increases outside of the study area relative to 2025 No-Build conditions are anticipated to range from 0 to 2.2 percent during the peak periods, with vehicle increases shown to range from 0 to 8 vehicles. When dispersed over the peak-hour, such increases would not result in a significant impact (increase) on motorist delays or vehicle queueing outside of the immediate study area this is the subject of this assessment.

3.6 Traffic Operations Analysis

Measuring existing and future traffic volumes quantifies traffic flow within the study area. To assess quality of flow, roadway capacity and vehicle queue analyses were conducted under Existing, No-Build and Build traffic volume conditions. Capacity analyses provide an indication of how well the roadway facilities serve the traffic demands placed upon them, with vehicle queue analyses providing a secondary measure of the operational characteristics of an intersection or section of roadway under study.

3.6.1 Methodology

3.6.1.1 Levels of Service

A primary result of capacity analyses is the assignment of level of service to traffic facilities under various traffic-flow conditions.⁶ The concept of level of service is defined as a qualitative measure describing operational conditions within a traffic stream and their perception by motorists and/or passengers. A level-of-service definition provides an index to quality of traffic flow in terms of such factors as speed, travel time, freedom to maneuver, traffic interruptions, comfort, convenience, and safety.

Six levels of service are defined for each type of facility. They are given letter designations from A to F, with level-of-service (LOS) A representing the best operating conditions and LOS F representing congested or constrained operating conditions.

Since the level of service of a traffic facility is a function of the traffic flows placed upon it, such a facility may operate at a wide range of levels of service, depending on the time of day, day of week, or period of year.

3.6.1.2 Signalized Intersections

The six levels of service for signalized intersections may be described as follows:

◆ *LOS A* describes operations with very low control delay; most vehicles do not stop at all.

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The capacity analysis methodology is based on the concepts and procedures presented in the Highway Capacity Manual; Transportation Research Board; Washington, DC; 2010.

- ◆ *LOS B* describes operations with relatively low control delay. However, more vehicles stop than LOS A.
- ◆ LOS C describes operations with higher control delays. Individual cycle failures may begin to appear. The number of vehicles stopping is significant at this level, although many still pass through the intersection without stopping.
- ◆ LOS D describes operations with control delay in the range where the influence of congestion becomes more noticeable. Many vehicles stop and individual cycle failures are noticeable.
- ◆ *LOS E* describes operations with high control delay values. Individual cycle failures are frequent occurrences.
- ◆ *LOS F* describes operations with high control delay values that often occur with over-saturation. Poor progression and long cycle lengths may also be major contributing causes to such delay levels.

Levels of service for signalized intersections were calculated using the Percentile Delay Method implemented as a part of the Synchro™ 8 software as required by MassDOT. The Percentile Delay Method assesses the effects of signal type, timing, phasing, and progression; vehicle mix; and geometrics on "percentile" delay. Level-of-service designations are based on the criterion of percentile delay per vehicle and is a measure of: i) driver discomfort; ii) motorist frustration; and iii) fuel consumption; and includes a uniform delay based on percentile volumes using a Poisson arrival pattern, an initial queue move-up time, and a queue interaction delay that accounts for delays resulting from queues extending from adjacent intersections. Table 3-5 summarizes the relationship between level-of-service and percentile delay, and uses the same numerical delay thresholds as the HCM method. The tabulated percentile delay criterion may be applied in assigning level-of-service designations to individual lane groups, to individual intersection approaches, or to entire intersections.

Table 3-5 Level-of-Service Criteria for Signalized Intersections

Level of Service	Percentile Delay Per Vehicle (Seconds)
A B C D E	<pre>< 10.0 10.1 to 20.0 20.1 to 35.0 35.1 to 55.0 55.1 to 80.0 > 80.0</pre>

3.6.1.3 Unsignalized Intersections

The six levels of service for unsignalized intersections may be described as follows:

- ♦ *LOS A* represents a condition with little or no control delay to minor street traffic.
- ♦ *LOS B* represents a condition with short control delays to minor street traffic.
- ♦ LOS C represents a condition with average control delays to minor street traffic.
- ♦ *LOS D* represents a condition with long control delays to minor street traffic.
- ♦ *LOS E* represents operating conditions at or near capacity level, with very long control delays to minor street traffic.
- ◆ *LOS F* represents a condition where minor street demand volume exceeds capacity of an approach lane, with extreme control delays resulting.

The levels of service of unsignalized intersections are determined by application of a procedure described in the 2010 Highway Capacity Manual.⁷ Level of service is measured in terms of average control delay. Mathematically, control delay is a function of the capacity and degree of saturation of the lane group and/or approach under study and is a quantification of motorist delay associated with traffic control devices such as traffic signals and STOP signs. Control delay includes the effects of initial deceleration delay approaching a STOP sign, stopped delay, queue move-up time, and final acceleration delay from a stopped condition. Definitions for level of service at unsignalized intersections are also given in the 2010 Highway Capacity Manual. Table 3-6 summarizes the relationship between level of service and average control delay for two-way stop controlled and all-way stop controlled intersections.

Table 3-6 Level-of-Service Criteria for Unsignalized Intersections^a

Level-of-Service by \	Volume-to-Capacity Ratio	Average Control Delay
v/c ≤ 1.0	v/c > 1.0	(Seconds Per Vehicle)
Α	F	≤10.0
В	F	10.1 to 15.0
С	F	15.1 to 25.0
D	F	25.1 to 35.0
Е	F	35.1 to 50.0
F	F	> 50.0

^aSource: *Highway Capacity Manual*; Transportation Research Board; Washington, DC; 2010; page 19-2.

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⁷ Highway Capacity Manual; Transportation Research Board; Washington, DC; 2010.

3.6.1.4 Vehicle Queue Analysis

Vehicle queue analyses are a direct measurement of an intersection's ability to process vehicles under various traffic control and volume scenarios and lane use arrangements. The vehicle queue analysis was performed using the Synchro™ intersection capacity analysis software which is based upon the methodology and procedures presented in the 2010 Highway Capacity Manual. The Synchro™ vehicle queue analysis methodology is a simulation based model which reports the number of vehicles that experience a delay of six seconds or more at an intersection. For signalized intersections, Synchro™ reports both the average (50th percentile) the 95th percentile vehicle queue. For unsignalized intersections, Synchro™ reports the 95th percentile vehicle queue. Vehicle queue lengths are a function of the capacity of the movement under study and the volume of traffic being processed by the intersection during the analysis period. The 95th percentile vehicle queue is the vehicle queue length that will be exceeded only 5 percent of the time, or approximately three minutes out of 60 minutes during the peak one hour of the day (during the remaining 57 minutes, the vehicle queue length will be less than the 95th percentile queue length).

3.6.1.5 Analysis Results

Level-of-service and vehicle queue analyses were conducted for 2018 Existing, 2025 No-Build and 2025 Build conditions for the intersections within the study area. The results of the intersection capacity and vehicle queue analyses are summarized in Tables 3-7 and 3-8. The detailed analysis results are presented in Attachment B.

The following is a summary of the level-of-service and vehicle queue analyses for the intersections within the study area. For context, a LOS of "D" or better is generally defined as "acceptable" operating conditions.

3.6.1.5.1 Signalized Intersections

Porter Street/Orleans Street – Under 2018 Existing conditions, this signalized intersection was shown to operate at an overall LOS A during both the weekday morning and evening peak hours. Under 2025 No-Build and 2025 Build conditions, overall operating conditions at the intersection were shown to degrade slightly (1.5 second increase in average motorist delay) from LOS A to LOS B during the weekday morning peak-hour as a result of traffic volume increases independent of the Project and to remain operating at LOS A during the weekday evening peak-hour. Project-related impacts at this intersection were defined as an increase in motorist delay of less than 1.0 seconds with no change in LOS or increase in vehicle queuing.

3.6.1.5.2 Unsignalized Intersections

Porter Street/Bremen Street – Under 2018 Existing conditions, critical movements at this intersection (all movements from Bremen Street southbound) were shown to operate at LOS E during the weekday morning peak-hour and at LOS C during the weekday evening peak-

hour. Under 2025 No-Build and 2025 Build conditions, the critical movements were shown to degrade from LOS E to LOS F during the weekday morning peak-hour and from LOS C to LOS D during the weekday evening peak-hour as a result of traffic volume increases independent of the Project. Project-related impacts were defined as an increase in motorist delay of less than 3.0 seconds and in vehicle queuing of up to one (1) vehicle.

Porter Street/Frankfort Street – All movements at this intersection were shown to operate at LOS B or better under all analysis conditions, with no change in LOS or vehicle queuing for any movement predicted to occur as a result of the Project.

Porter Street/Cottage Street – All movements at this intersection were shown to operate at LOS A under all analysis conditions, with no change in LOS or vehicle queuing for any movement predicted to occur as a result of the Project.

Bremen Street/Gove Street – All movements at this intersection were shown to operate at LOS A under all analysis conditions, with no change in LOS or vehicle queuing for any movement predicted to occur as a result of the Project.

Orleans Street/Gove Street – All movements at this intersection were shown to operate at LOS B or better under all analysis conditions, with no change in LOS or vehicle queuing for any movement predicted to occur as a result of the Project.

Frankfort Street/Gove Street – All movements at this intersection were shown to operate at LOS A under all analysis conditions, with no change in LOS or vehicle queuing for any movement predicted to occur as a result of the Project.

Cottage Street/Gove Street – All movements at this intersection were shown to operate at LOS A under all analysis conditions, with no change in LOS or vehicle queuing for any movement predicted to occur as a result of the Project.

Maverick Street/Bremen Street – All movements at this intersection were shown to operate at LOS B or better under all analysis conditions, with no change in LOS or vehicle queuing for any movement predicted to occur as a result of the Project.

Maverick Street/Orleans Street – All movements at this intersection were shown to operate at LOS B or better under all analysis conditions, with no change in LOS or vehicle queuing for any movement predicted to occur as a result of the Project.

Maverick Street/Frankfort Street – All movements at this intersection were shown to operate at LOS B or better under all analysis conditions, with no change in LOS or vehicle queuing for any movement predicted to occur as a result of the Project.

Maverick Street/Cottage Street – All movements at this intersection were shown to operate at LOS B or better under all analysis conditions, with no change in LOS or vehicle queuing for any movement predicted to occur as a result of the Project.

Frankfort Street/Project Site Driveway – All movements at this intersection were shown to operate at LOS A during both the weekday morning and evening peak hours, with negligible vehicle queuing predicted.

Lubec Street/Project Site Driveway – All movements at this intersection were shown to operate at LOS A during both the weekday morning and evening peak hours, with negligible vehicle queuing predicted.

Table 3-7 Signalized Intersection Level-of-Service and Vehicle Queue Summary

		2018 Existing					2025 No-Build				2025 Build			
Signalized Intersection/Peak- hour/Movement	V/C ^a	Delay ^b	LOSc	Queue ^d 50 th /95 th	V/C	Delay	LOS	Queue 50 th /95 th	V/C	Delay	LOS	Queue 50 th /95 th		
Porter Street at Orleans Street														
Weekday Morning:														
Porter Street EB LT/TH/RT	0.54	8.6	Α	2/6	0.57	9.1	Α	3/8	0.58	9.1	Α	3/8		
Porter Street WB LT/TH/RT	0.17	5.9	Α	1/2	0.18	5.9	Α	1/2	0.18	5.9	Α	1/2		
Orleans Street NB LT/TH/RT	0.34	15.0	В	1/3	0.48	19.9	В	2/5	0.48	20.0	В	2/5		
Driveway SB LT/TH/RT	0.01	10.5	В	0/1	0.01	12.8	В	0/1	0.01	12.8	В	0/1		
Overall		9.2	Α			10. <i>7</i>	В			10. <i>7</i>	В			
Weekday Evening:														
Porter Street EB LT/TH/RT	0.38	6.0	Α	1/4	0.51	8.4	Α	2/6	0.52	8.4	Α	2/6		
Porter Street WB LT/TH/RT	0.08	4.9	Α	1/1	0.10	5.9	Α	1/1	0.10	5.8	Α	1/1		
Orleans Street NB LT/TH/RT	0.22	10.1	В	1/2	0.37	14.0	В	1/4	0.37	14.4	В	1/4		
Driveway SB LT/TH/RT	0.01	7.0	Α	0/1	0.01	8.0	Α	0/1	0.01	8.1	Α	0/1		
Overall	_	6.8	A	-		9.5	A		-	9.5	A			

^aVolume-to-capacity ratio.

NB = northbound; SB = southbound; EB = eastbound; WB = westbound; LT = left-turning movements; TH = through movements; RT = right-turning movements.

^bPercentile delay per vehicle in seconds.

^cLevel-of-Service.

^dQueue length in vehicles.

Table 3-8 Unsignalized Intersection Level-of-Service and Vehicle Queue Summary

		2010 5	i-ti			2025 No. 1	لدان۵			2025 [اداند	
		2018 Ex	Isting	<u> </u>	2025 No-Build				2025 Build			
Unsignalized Intersection/ Peak Hour/Movement	Demand ^a	Delay ^b	LOSc	Queue ^d 95 th	Demand	Delay	LO S	Queue 95 th	Demand	Delay	LOS	Queue 95 th
Porter Street at Bremen Street Weekday Morning:			_	_								
Porter Street EB LT/TH/RT	181	14.3	В	2	238	20.6	C	4	240	21.1	С	4
Porter Street WB LT/TH/RT	270	16.9	С	3	318	26.8	D	6	327	28.5	D	6
Bremen Street NB LT/TH/RT Bremen Street SB LT/TH/RT	195	14.3 39.1	B E	2 10	239 497	20.4 >50.0	C F	4 14	239 499	20.8 > 50.0	C F	4 14
Weekday Evening:	457	39.1	E	10	497	> 50.0		14	499	> 50.0	-	14
Porter Street EB LT/TH/RT	252	14.5	В	3	323	24.4	C	5	330	26.2	D	6
Porter Street WB LT/TH/RT	201	12.4	В	2	252	18.1	C	3	260	19.5	С	4
Bremen Street NB LT/TH/RT	266	14.5	В	3	309	22.7	C	5	309	23.8	С	5
Bremen Street SB LT/TH/RT	314	16.6	С	4	350	27.9	D	6	357	30.1	D	7
Porter Street at Frankfort Street												
Weekday Morning:												
Porter Street EB TH/RT	367	0.0	Α	0	395	0.0	Α	0	397	0.0	Α	0
Porter Street WB LT/TH	46	0.0	Α	0	49	0.0	Α	0	50	0.0	Α	0
Frankfort Street NB LT/RT	79	12.5	В	1	84	13.1	В	1	92	13.3	В	1
Weekday Evening:												
Porter Street EB TH/RT	265	0.0	Α	0	287	0.0	Α	0	293	0.0	Α	0
Porter Street WB LT/TH	25	0.3	A	0	27	0.3	Α	0	28	0.3	A	0
Frankfort Street NB LT/RT	50	10.8	В	1	53	11.1	В	1	60	11.2	В	1

See notes at end of table.

Table 3-8 Unsignalized Intersection Level-of-Service and Vehicle Queue Summary (Continued)

		2018 E	xisting			2025 No	o-Build			2025 E	Build	
Unsignalized Intersection/ Peak Hour/Movement	Deman d ^a	Delay b	LOSc	Queue ^d 95 th	Deman d	Delay	LOS	Queue 95 th	Demand	Delay	LOS	Queue 95 th
Porter Street at Cottage Street												
Weekday Morning:												
Porter Street EB TH/RT	290	0.0	Α	0	312	0.0	Α	0	312	0.0	Α	0
Porter Street WB LT/TH	1	0.0	Α	0	1	0.0	Α	0	1	0.0	Α	0
Weekday Evening:												
Porter Street EB TH/RT	208	0.0	Α	0	226	0.0	Α	0	226	0.0	Α	0
Porter Street WB LT/TH	0	0.0	Α	0	0	0.0	Α	0	0	0.0	Α	0
Bremen Street at Gove Street												
Weekday Morning:												
Gove Street EB LT/RT	90	8.3	Α	1	96	8.5	Α	1	96	8.5	Α	1
Bremen Street NB LT/TH	159	8.6	Α	1	181	8.9	Α	1	181	8.9	Α	1
Bremen Street SB TH/RT	1 <i>7</i> 1	8.7	Α	1	191	9.0	Α	1	191	9.0	Α	1
Weekday Evening:												
Gove Street EB LT/RT	80	8.1	Α	1	86	8.3	Α	1	86	8.3	Α	1
Bremen Street NB LT/TH	184	8.6	Α	1	210	9.0	Α	1	210	9.0	Α	1
Bremen Street SB TH/RT	141	8.3	Α	1	159	8.6	Α	1	159	8.6	Α	1
Orleans Street at Gove Street												
Weekday Morning:												
Gove Street EB LT/TH/RT	0	0.0	Α	0	0	0.0	Α	0	0	0.0	Α	0
Gove Street WB LT/TH/RT	50	9.5	Α	1	53	9.7	Α	1	53	9.7	Α	1
Orleans Street NB LT/TH/RT	120	0.0	Α	0	132	0.0	Α	0	132	0.0	Α	0
Orleans Street SB LT/TH/RT	66	1.7	Α	0	77	1.7	Α	0	79	1.7	Α	0
Weekday Evening:												
Gove Street EB LT/TH/RT	1	10.0	В	0	1	10.5	В	0	1	10.6	В	0
Gove Street WB LT/TH/RT	25	8.9	Α	0	27	9.1	Α	0	27	9.1	Α	0
Orleans Street NB LT/TH/RT	92	0.1	Α	0	127	0.0	Α	0	127	0.1	Α	0
Orleans Street SB LT/TH/RT	84	1.0	A	0	100	1.0	Α	0	105	1.2	Α	0

See notes at end of table.

Table 3-8 Unsignalized Intersection Level-of-Service and Vehicle Queue Summary (Continued)

		2018 Ex	isting			2025 No	-Build			2025 E	uild	_
Unsignalized Intersection/ Peak Hour/Movement	_Demand ^a _	Delayb	LOSc	Queue ^d 95 th	Demand	Delay	LOS	Queue 95 th	Demand	Delay	LOS	Queue 95 th
Frankfort Street at Gove Street												
Weekday Morning:												
Gove Street EB LT/TH/RT	35	7.5	Α	1	37	7.6	Α	1	39	7.6	Α	1
Gove Street WB LT/TH/RT	76	7.5	Α	1	81	7.6	Α	1	81	7.7	Α	1
Frankfort Street NB LT/TH/RT	55	7.6	Α	1	59	7.7	Α	1	66	7.7	Α	1
Frankfort Street SB LT/TH/RT	47	7.7	Α	1	50	7.8	Α	1	52	7.8	Α	1
Weekday Evening:												
Gove Street EB LT/TH/RT	27	7.3	Α	0	29	7.3	Α	0	34	7.3	Α	0
Gove Street WB LT/TH/RT	56	7.1	Α	1	60	7.2	Α	1	60	7.2	Α	1
Frankfort Street NB LT/TH/RT	25	7.2	Α	0	26	7.2	Α	0	33	7.3	Α	0
Frankfort Street SB LT/TH/RT	35	7.3	Α	1	37	7.3	Α	1	42	7.4	Α	1
Cottage Street at Gove Street												
Weekday Morning:												
Gove Street EB TH/RT	58	7.3	Α	1	61	7.4	Α	1	61	7.4	Α	1
Gove Street WB LT/TH	13	7.3	Α	0	14	7.8	Α	0	14	7.8	Α	0
Cottage Street SB LT/TH/RT	284	9.1	Α	2	305	9.4	Α	2	305	9.4	Α	2
Weekday Evening:												
Gove Street EB TH/RT	25	7.0	Α	0	27	7.0	Α	0	27	7.0	Α	0
Gove Street WB LT/TH	15	7.6	Α	0	16	7.6	Α	0	16	7.6	Α	0
Cottage Street SB LT/TH/RT	206	8.2	Α	1	224	8.4	Α	1	224	8.4	Α	1
Maverick Street at Bremen Street												
Weekday Morning:												
Maverick Street WB LT/TH/RT	357	11.3	В	3	391	12.4	В	3	393	12.4	В	3
Bremen Street NB LT/TH	87	9.0	Α	1	108	9.3	Α	1	108	9.3	Α	1
Bremen Street SB TH/RT	139	9.0	Α	1	157	9.4	Α	1	157	9.4	Α	1
Weekday Evening:			_	_			_	_			_	
Maverick Street WB LT/TH/RT	278	10.1	В	2	309	10.9	В	2	310	10.9	В	2
Bremen Street NB LT/TH	113	8.9	A	1	130	9.2	Α	1	130	9.2	Α	1
Bremen Street SB TH/RT	178	9.0	Α	1	199	9.4	Α	1	199	9.4	Α	1

Table 3-8 Unsignalized Intersection Level-of-Service and Vehicle Queue Summary (Continued)

			2025 No	-Build			2025 E	Build				
Unsignalized Intersection/ Peak Hour/Movement	Demand ^a	Delay ^b	LOSc	Queue ^d 95 th	Demand	Delay	LOS	Queue 95 th	Demand	Delay	LOS	Queue 95 th
Maverick Street at Orleans Street												
Weekday Morning:												
Maverick Street WB LT/TH/RT	350	11.4	В	3	387	12.6	В	4	391	12.7	В	4
Orleans Street NB LT/TH	101	9.0	Α	1	109	9.3	Α	1	109	9.3	Α	1
Orleans Street SB TH/RT	52	8.3	Α	1	59	8.5	Α	1	59	8.6	Α	1
Weekday Evening:												
Maverick Street WB LT/TH/RT	280	9.7	Α	2	314	10.3	В	2	317	10.4	В	2
Orleans Street NB LT/TH	90	8.5	Α	1	98	8.8	Α	1	98	8.8	Α	1
Orleans Street SB TH/RT	63	8.1	Α	1	76	8.3	Α	1	76	8.4	Α	1
Maverick Street at Frankfort Street												
Weekday Morning:												
Maverick Street WB LT/TH/RT	374	0.1	Α	0	412	0.1	Α	0	413	0.1	Α	0
Driveway NB LT/TH	5	12.1	В	0	5	12.6	В	0	5	12.6	В	0
Frankfort Street SB TH/RT	28	10.9	В	1	30	11.3	В	1	34	11.3	В	1
Weekday Evening:												
Maverick Street WB LT/TH/RT	280	0.3	Α	0	313	0.3	Α	0	316	0.3	Α	0
Driveway NB LT/TH	8	11.4	В	0	8	11.8	В	0	8	11.9	В	0
Frankfort Street SB TH/RT	33	10.2	В	1	35	10.5	В	1	38	10.6	В	1
Maverick Street at Cottage Street												
Weekday Morning:												
Maverick Street WB LT/TH	219	9.7	Α	2	244	10.3	В	2	245	10.3	В	2
Cottage Street SB TH/RT	320	9.6	Α	2	344	10.2	В	3	344	10.2	В	3
Weekday Evening:												
Maverick Street WB LT/TH	218	9.7	Α	2	247	10.3	В	2	250	10.3	В	2
Cottage Street SB TH/RT	199	8.9	Α	2	216	9.3	Α	2	216	9.3	Α	2

Table 3-8 Unsignalized Intersection Level-of-Service and Vehicle Queue Summary (Continued)

		2018 Existing 2025 No-Build				2025 Build						
Unsignalized Intersection/ Peak Hour/Movement	Demand ^a	Delay ^b	LOSc	Queue ^d 95 th	Demand	Delay	LOS	Queue 95 th	Demand	Delay	LOS	Queue 95 th
Frankfort Street at the Project Site Drive Weekday Morning:												
Project Site Drive WB LT/RT									12	8.8	Α	0
Frankfort Street NB TH/RT									60	0.0	Α	0
Frankfort Street SB LT/TH									29	1.0	Α	0
Weekday Evening:												
Project Site Drive WB LT/RT									10	8.7	Α	0
Frankfort Street NB TH/RT									29	0.0	Α	0
Frankfort Street SB LT/TH						-			49	1.5	Α	0
Lubec Street at the Project Site Drive												
Weekday Morning:												
Project Site Drive EB LT/RT									1	8.8	Α	0
Lubec Street NB LT/TH			-						8	0.0	Α	0
Lubec Street SB TH/RT	_								50	0.0	Α	0
Weekday Evening:												
Project Site Drive EB LT/RT									1	8.7	Α	0
Lubec Street NB LT/TH									3	0.0	Α	0
Lubec Street SB TH/RT									27	0.0	Α	0

^aDemand in vehicles per hour.

NB = northbound; SB = southbound; EB = eastbound; WB = westbound; LT = left-turning movements; TH = through movements; RT = right-turning movements.

^bAverage control delay per vehicle (in seconds).

^cLevel-of-Service.

^dQueue length in vehicles.

3.7 Sight Distance Evaluation

Sight distance measurements were performed at the Project site driveway intersections with

Frankfort Street and Lubec Street in accordance with MassDOT and American Association of State Highway and Transportation Officials (AASHTO)⁸ requirements. Both stopping sight distance (SSD) and intersection sight distance (ISD) measurements were performed. In brief, SSD is the distance required by a vehicle traveling at the design speed of a roadway, on wet pavement, to stop prior to striking an object in its travel path. ISD or corner sight distance (CSD) is the sight distance required by a driver entering or crossing an intersecting roadway to perceive an on-coming vehicle and safely complete a turning or crossing maneuver with on-coming traffic. In accordance with AASHTO standards, if the measured ISD is at least equal to the required SSD value for the appropriate design speed, the intersection can operate in a safe manner. Table 3-9 presents the measured SSD and ISD at the subject intersections.

As can be seen in Table 3-9, the available lines of sight at the Project site driveway intersections were found to meet or exceed the recommended minimum sight distance (SSD) to function in a safe manner based on the appropriate approach speed along the intersecting roadway and with consideration of the urban environment in which the Project is located.⁹ It is recommended that on-street parking be prohibited within 20-feet (one parking space) of the Project site driveways in order to provide the requisite sight lines and to allow for vehicle maneuvering entering/exiting the Project site.

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A Policy on Geometric Design of Highway and Streets, 6th Edition; American Association of State Highway and Transportation Officials (AASHTO); Washington D.C.; 2011.

In an urban environment with a sidewalk and on-street parking, exiting motorists are assumed to temporarily occupy the sidewalk and the protected area formed by the parking lane in order to observe vehicles on the intersecting roadway.

Table 3-9 Sight Distance Measurements

Intersection/Sight Distance Measurement	Required Minimum (SSD)	Desirable (ISD) ^b	Measured
Frankfort Street at the Project Site Driveway Stopping Sight Distance:			
Frankfort Street approaching from the north	155		315
Frankfort Street approaching from the south	155		223
Intersection Sight Distance:			
Looking to the north from the Project Site Drive	155	240/280	216
Looking to the south from the Project Site Drive	155	240/280	223°
Lubec Street at the Project Site Driveway Stopping Sight Distance:			
Lubec Street approaching from the north	155		158
Lubec Street approaching from the south	80		85 ^d
Intersection Sight Distance:			
Looking to the north from the Project Site Driveway	155	240/280	158
Looking to the south from the Project Site Driveway	80	145/1 <i>7</i> 0	85 ^d

^aRecommended minimum values obtained from *A Policy on Geometric Design of Highways and Streets,* 6th Edition; American Association of State Highway and Transportation Officials (AASHTO); 2011; and based on a 25 mph approach speed for Frankfort Street and Lubec Street northbound, and a 15 mph approach speed for Lubec Street southbound given the proximity of the driveway to Gove Street.

3.8 Conclusions and Recommendations

3.8.1 Conclusions

VAI has completed a detailed assessment of the potential impacts on the transportation infrastructure associated with the proposed construction of a 115-unit multi-family residential community at the location of the former Our Lady of Mt. Carmel Church sanctuary and rectory located off Gove Street and Frankfort Street in the East Boston Neighborhood of Boston, Massachusetts. The following specific areas have been evaluated as they relate to the Project: i) access requirements; ii) potential off-site improvements; and iii) safety considerations; under existing and future conditions, both with and without the Project. Based on this assessment, we have concluded the following with respect to the Project:

^bValues shown are the intersection sight distance for a vehicle turning right/left exiting a roadway under STOP control such that motorists approaching the intersection on the major street should not need to adjust their travel speed to less than 70 percent of their initial approach speed.

cSight line that is available with driver positioned 10 feet from the edge of roadway (from within the sidewalk area).

dClear line of sight is provided to/from Gove Street

- 1. Using trip-generation statistics published by the ITE¹⁰ and with adjustment to account for the use of public transportation and pedestrian and bicycle trips, the Project is expected to generate approximately 338 automobile trips, 120 transit trips and 206 pedestrian/bicycle trips on an average weekday (two-way, 24 hour volumes), with 18 automobile trips, 11 transit trips and 14 pedestrian/bicycle trips expected during the weekday morning peak-hour, and 25 automobile trips, 12 transit trips and 18 pedestrian/bicycle trips expected during the weekday evening peak-hour;
- 2. The Project will not have a significant impact (increase) on motorist delays or vehicle queuing over Existing or anticipated future conditions without the Project (No-Build conditions), with the majority of the movements at the study intersections shown to operate at LOS D or better under all analysis conditions where an LOS of "D" or better is defined as "acceptable" operating conditions;
- 3. Independent of the Project, the Bremen Street southbound approach to Porter Street was identified as operating at or over capacity (defined as LOS "E" or "F", respectively) during the weekday morning peak-hour, with Project-related impacts at the intersection defined as an increase in vehicle queuing of up to one vehicle;
- 4. All movements at the Project site driveway intersections with Frankfort Street and Lubec Street are expected to operate at LOS A with negligible vehicle queueing predicted;
- 5. A review of the MassDOT statewide High Crash Location List indicated that there were no locations within the study area that were included on MassDOT's Highway Safety Improvement Program (HSIP) listing as a high crash cluster location for 2013-2015; and
- 6. Lines of sight to and from the Project site driveways were found to meet or exceed the recommended minimum sight distance to function in a safe manner with consideration of the urban environment in which the Project is located.

In consideration of the above, we have concluded that the Project can be accommodated within the confines of the existing transportation infrastructure in a safe and efficient manner with implementation of the recommendations that follow.

3.8.2 Recommendations

A detailed transportation improvement program has been developed that is designed to provide safe and efficient access to the Project site and address any deficiencies identified at off-site locations evaluated in conjunction with this study. The following improvements

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¹⁰ Ibid 1.

have been recommended as a part of this evaluation and, where applicable, will be completed in conjunction with the Project subject to receipt of all necessary rights, permits, and approvals.

3.8.2.1 Project Access

Access to the Project will be provided by way of two driveways configured as follows: north parcel - a full access driveway that will intersect the west side of Lubec Street approximately 50-feet north of Gove Street; south parcel - a full access driveway that will intersect the east side of Frankfort Street at the south end of the parcel. The following recommendations are offered with respect to Project access, internal circulation and parking:

- The Project site driveways should be a minimum of 20-feet in width with vehicles exiting the driveways under stop control.
- Where perpendicular parking is provided, the drive aisle behind the parking should be a minimum of 23-feet in order to allow for vehicle maneuvering.
- All signs and pavement markings to be installed within the Project site shall conform to the applicable standards of the Manual on Uniform Traffic Control Devices (MUTCD).¹¹
- ♦ Marked crosswalks with Americans with Disabilities Act (ADA) compliant wheelchair ramps should be provided at all proposed pedestrian crossings.
- Signs and landscaping to be installed as a part of the Project within intersection sight triangle areas should be designed and maintained so as not to restrict lines of sight.
- Snow windrows within sight triangle areas shall be promptly removed where such accumulations would impede sight lines.
- ♦ Audible and visual pedestrian warning devices will be installed at the garage exit driveway to warn pedestrians of vehicles that may be exiting the driveway.
- On-street parking should be prohibited within 20-feet (one parking space) of the Project site driveways in order to provide the requisite sight lines and to allow for vehicle maneuvering entering/exiting the Project site.
- Five percent of the parking spaces to be constructed as a part of the Project will include electric vehicle (EV) charging stations, with accommodations (reserve conduit and space available for electric system components) provided to expand the number of EV charging stations from 5 percent to 15 percent.

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¹¹ Ibid 2.

3.8.2.2 Off-Site

Frankfort Street at Gove Street

The addition of Project-related traffic to the intersection of Frankfort Street at Gove Street was not shown to result in a change in LOS, with all movements expected to continue to operate at LOS A during the peak hours. In an effort to enhance pedestrian accessibility and safety in the area, the Project proponent will install pedestrian actuated flashing beacons with accompanying pedestrian crossing warning signs for each of the crosswalks at the intersection. These improvements will be completed prior to the issuance of a Certificate of Occupancy for the Project and subject to receipt of all necessary rights, permits and approvals.

Lubec Street at Gove Street

In an effort to enhance pedestrian accessibility and safety at the Lubec Street/Gove Street intersection and to/from the Donald McKay School and the East Boston Early Childhood Learning Center, the Project proponent will install pedestrian actuated flashing beacons with accompanying pedestrian crossing warning signs for the Gove Street crosswalk at the intersection. These improvements will be completed prior to the issuance of a Certificate of Occupancy for the Project and subject to receipt of all necessary rights, permits and approvals.

3.8.2.3 Transportation Demand Management

Public transportation services are provided within the study area by the Massachusetts Bay Transportation Authority (MBTA) (Blue Line subway and fixed-route bus service) and are accessible to residents of the Project. Maverick Station on the Blue Line subway system is located at Maverick Square and is within a 7-minute walking distance of the Project site. MBTA bus Route 120, Orient Heights - Maverick Station, provides service along Maverick Street to both Maverick Station and Orient Heights Station on the Blue Line subway system, with a stop located at the Maverick Street/Frankfort Street intersection which is within a 2-minute walking distance of the Project site. Sidewalks are provided along the study area roadways that link the Project site to both Maverick Station and the Route 120 bus stop. In addition, bicycle lanes are provided along Maverick Street and the East Boston Greenway multi-use path is located to the west of the Project site and is accessible from Gove Street.

In an effort to encourage the use of alternative modes of transportation to single-occupant vehicles, the following Transportation Demand Management (TDM) measures will be implemented as a part of the Project:

◆ A Transportation Coordinator will be assigned for the Project and the name and contact information for said person will be provided to BTD.

- ◆ The owner or property manager will join the A Better City (ABC) Transportation Management Association (TMA) and the Transportation Coordinator will contact MassRIDES to obtain information on facilitating and encouraging healthy transportation options for residents of the Project;
- Information regarding public transportation services, maps, schedules and fare information will be posted in a central location and/or otherwise made available to residents;
- ◆ A "welcome packet" will be provided to residents detailing available public transportation services, bicycle and walking alternatives, and commuter options available through MassRIDES' and their Bay State Commute (formerly NuRide) program which rewards individuals that choose to walk, bicycle, carpool, vanpool or that use public transportation to travel to and from work;
- Residents will be made aware of the Emergency Ride Home (ERH) program available through MassRIDES, which reimburses employees of a participating MassRIDES employer partner worksite that is registered for ERH and that carpool, take transit, bicycle, walk or vanpool to work;
- Sidewalks along the Project site frontage on Frankfort Street, Gove Street and Lubec Street will be reconstructed as necessary to provide an ADA accessible travel route for pedestrians;
- A mail drop will be provided in a central location in each building;
- Bicycle parking will be provided consisting of: i) secure bicycle parking conveniently located proximate to the building entrance; and ii) weather protected bicycle parking located in a secure area within the building.
- ◆ Two parking spaces will be offered for use by car-share services; and
- Real-time transportation display technologies will be installed in building lobbies.

With implementation of the above recommendations, safe and efficient vehicular, pedestrian and bicycle access will be provided to the Project site and the Project can be accommodated within the confines of the existing and improved transportation system.

Environmental Review Component

4.0 ENVIRONMENTAL REVIEW COMPONENT

4.1 Wind

Major buildings, especially those that protrude above their surroundings, may cause increased local wind speeds at the pedestrian level. Typically, wind speeds increase with elevation above the ground surface, and taller buildings intercept these faster winds and deflect them down to the pedestrian level. The funneling of wind through gaps between buildings and the acceleration of wind around corners of buildings may also cause increases in wind speed. Conversely, if a building is surrounded by others of equivalent height, it may be protected from the prevailing upper-level winds, resulting in no significant changes to the local pedestrian-level wind environment.

The Project ranges from four to six-stories, and approximately 65 feet tall at its highest point. The other buildings surrounding the Project site are primarily two to six-stories tall and similar in height to the Project. Additionally, the Project will include new trees on the sidewalks surrounding the Project, which serve to reduce wind speeds. Due to the Project's low height, along with significant landscaping included in the Project, wind impacts are not anticipated.

4.2 Shadow

4.2.1 Introduction and Methodology

A shadow impact analysis was conducted to investigate shadow impacts from the Project during three time periods (9:00 a.m., 12:00 noon, and 3:00 p.m.) during the vernal equinox (March 21), summer solstice (June 21), autumnal equinox (September 21), and winter solstice (December 21). In addition, shadow studies were conducted for the 6:00 p.m. time period during the summer solstice and autumnal equinox.

The shadow impact analysis presents the existing shadow and new shadow that would be created by the Project, illustrating the incremental impact of the Project. The analysis focuses on nearby open spaces and sidewalks adjacent to and in the vicinity of the Project site. Shadows have been determined using the applicable Altitude and Azimuth data for Boston. Figures showing the net new shadow from the Project are provided in Figures 4.2-1 to 4.2-14 at the end of this section.

The shadow impact analysis shows that new shadow will generally be limited to the surrounding streets and Project site.

4.2.2 Vernal Equinox (March 21)

At 9:00 a.m. during the vernal equinox, new shadow from the Project will be cast to the northwest onto Frankfort Street and its sidewalks.

At 12:00 p.m., new shadow from the Project will be cast to the north and onto Frankfort Street and its southern sidewalk and a portion of Gove Street and its western sidewalk.

At 3:00 p.m., new shadow from the Project will be cast to the northeast across Gove Street and its sidewalks.

No new shadow will be cast onto nearby existing open spaces or bus stops during the time periods studied.

4.2.3 Summer Solstice (June 21)

At 9:00 a.m. during the summer solstice, new shadow from the Project will be cast to the northwest across Frankfort Street and its southern sidewalk.

At 12:00 p.m., new shadow from the Project will be minimal and cast to the north across Frankfort Street's southern sidewalk.

At 3:00 p.m., new shadow from the Project will be cast to the east across a portion of Gove Street and its western sidewalk.

At 6:00 p.m., new shadow from the Project will be cast to the southeast across a portion of Gove Street and its western sidewalk and the surface parking lot adjacent to the Project site.

No new shadow will be cast onto nearby existing open spaces or bus stops during the time periods studied.

4.2.4 Autumnal Equinox (September 21)

At 9:00 a.m. during the autumnal equinox, new shadow from the Project will be cast to the northwest and across Frankfort Street and its sidewalks.

At 12:00 pm., new shadow from the Project will be cast to the north and across Frankfort Street and its southern sidewalk.

At 3:00 p.m., new shadow from the Project will be cast to the northeast and across a portion of Gove Street and its sidewalks.

At 6:00 p.m., new shadow from the Project will be cast to the east and across Gove Street and its sidewalks and a portion of the surface parking lot adjacent to the Project site.

No new shadow will be cast onto nearby existing open spaces or bus stops during the time periods studied.

4.2.5 Winter Solstice (December 21)

The winter solstice creates the least favorable conditions for sunlight in New England. Because the sun angle during the winter is lower than in other seasons, shadows are made longer and reach further into the surrounding area.

At 9:00 a.m., new shadow from the Project will be cast to the northeast across Frankfort Street and its sidewalks.

At 12:00 p.m., new shadow from the Project will be cast to the north across Frankfort Street and its sidewalks and a portion of Gove Street and its sidewalks.

At 3:00 p.m., new shadow from the Project will be cast to the northeast across a sliver of Frankfort Street and its southern sidewalks and onto a portion of Gove Street and its sidewalks.

No new shadow will be cast onto nearby existing open spaces or bus stops during the time periods studied.

4.2.6 Conclusions

Fourteen time periods were studied to determine the extent of new shadow to be cast by the Project. The shadow study shows that new shadow will mainly be cast across nearby streets and sidewalks. No new shadow will be cast across existing open space or nearby bus stops in the area.





Frankfort Gove Street Housing Boston, Massachusetts



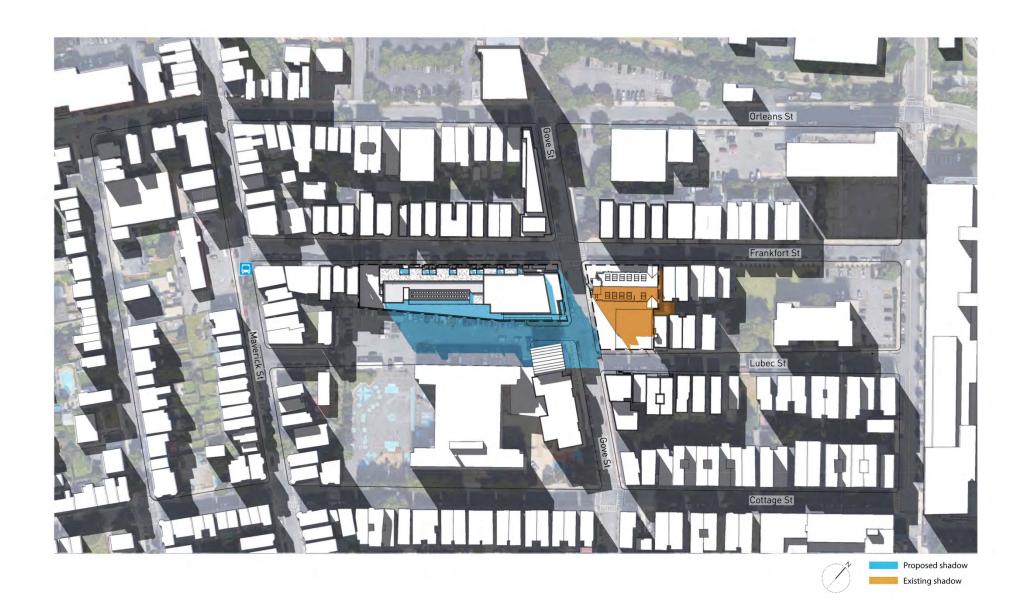












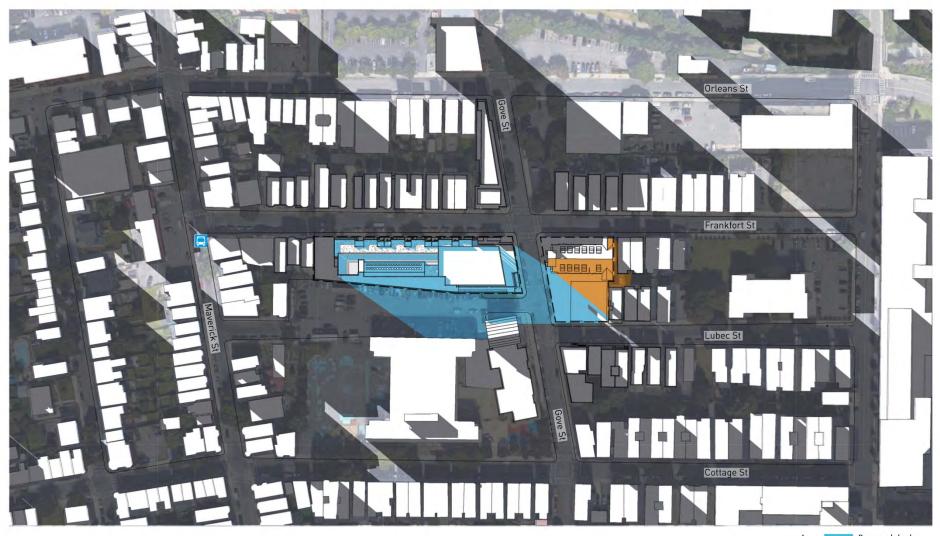






















4.3 Daylight Analysis

4.3.1 Introduction

The purpose of the daylight analysis is to estimate the extent to which a proposed project will affect the amount of daylight reaching the streets and the sidewalks in the immediate vicinity of a project site. The daylight analysis for the Project considers the existing and proposed conditions, as well as daylight obstruction values of the surrounding area.

4.3.2 Methodology

The daylight analysis was performed using the Boston Redevelopment Authority Daylight Analysis (BRADA) computer program¹. This program measures the percentage of "sky dome" that is obstructed by a project and is a useful tool in evaluating the net change in obstruction from existing to build conditions at a specific site.

Using BRADA, a silhouette view of the building is taken at ground level from the middle of the adjacent city streets or pedestrian ways centered on the proposed building. The façade of the building facing the viewpoint, including heights, setbacks, corners and other features, is plotted onto a base map using lateral and elevation angles. The two-dimensional base map generated by BRADA represents a figure of the building in the "sky dome" from the viewpoint chosen. The BRADA program calculates the percentage of daylight that will be obstructed on a scale of 0 to 100 percent based on the width of the view, the distance between the viewpoint and the building, and the massing and setbacks incorporated into the design of the building; the lower the number, the lower the percentage of obstruction of daylight from any given viewpoint.

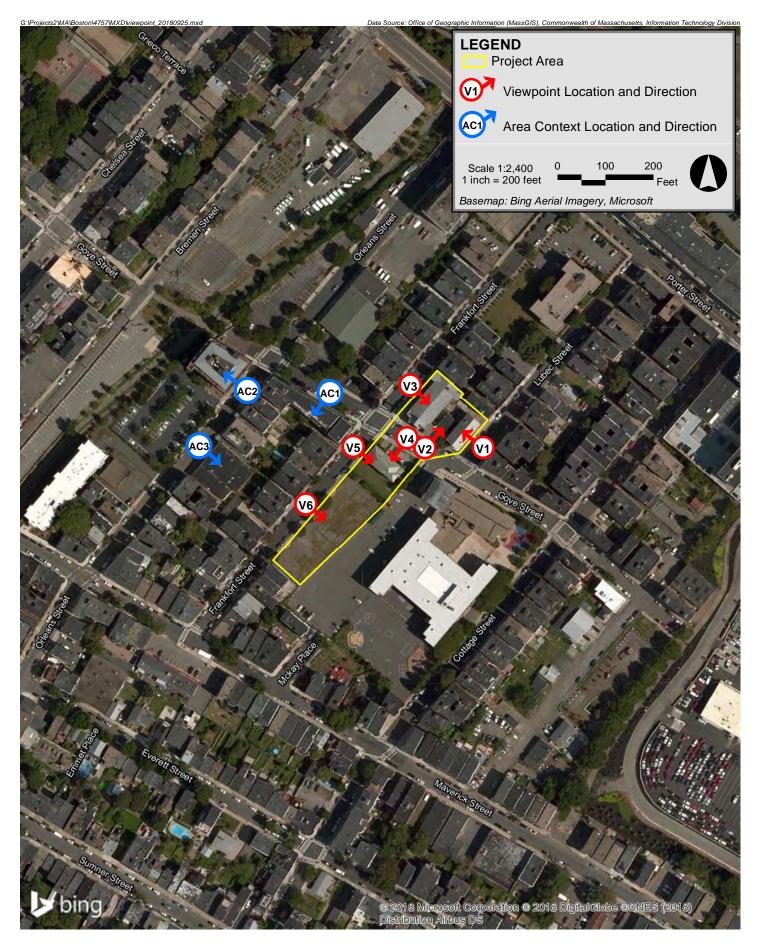
The analysis compares three conditions: Existing Conditions; Proposed Conditions; and the context of the area.

Six viewpoints were chosen to evaluate the daylight obstruction for the Existing and Proposed Conditions. Three area context points were considered to provide a basis of comparison to existing conditions in the surrounding area. The viewpoint and area context viewpoints were taken in the following locations and are shown on Figure 4.3-1.

- ◆ Viewpoint 1: View from the center of Lubec Street facing northwest toward the 128-134 Gove Street parcel.
- ♦ Viewpoint 2: View from the center of Gove Street facing north toward the 128-134 Gove Street parcel and the Church Building.

-

Method developed by Harvey Bryan and Susan Stuebing, computer program developed by Ronald Fergle, Massachusetts Institute of Technology, Cambridge, MA, September 1984.



Frankfort Gove Street Housing

Boston, Massachusetts



- ◆ Viewpoint 3: View from the center of Frankfort Street facing southeast toward the Church Building.
- ◆ Viewpoint 4: View from the center of Gove Street facing southwest toward the 115 Gove Street parcel.
- ◆ Viewpoint 5: View from the center of Frankfort Street facing southeast toward the 115 Gove Street parcel.
- Viewpoint 6: View from the center of Frankfort Street facing southeast toward the Frankfort Street parcel.
- ◆ Area Context Viewpoint AC1: View from the center of Gove Street facing southwest toward 99 Gove Street.
- ◆ Area Context Viewpoint AC2: View from the center of Orleans Street facing northwest toward 150 Orleans Street.
- ◆ Area Context Viewpoint AC3: View from the center of Orleans Street facing southeast toward 117 Orleans Street.

4.3.3 Results

The results of each viewpoint are described in Table 4.3-1. Figures 4.3-2 through 4.3-9 illustrate the BRADA results for each analysis.

Table 4.3-1 Daylight Analysis Results

Viewpoint Loca	ations	Existing Conditions	Proposed Conditions
Viewpoint 1	View from the center of Lubec Street facing northwest toward the 128-134 Gove Street parcel.	50.6%	9.8%
Viewpoint 2	View from the center of Gove Street facing north toward the 128-134 Gove Street parcel and the Church Building.	49.2%	24.3%
Viewpoint 3	View from the center of Frankfort Street facing southeast toward the Church Building	62.1%	62.1%
Viewpoint 4	View from the center of Gove Street facing southwest toward the 115 Gove Street parcel.	18.3%	49.1%
Viewpoint 5	View from the center of Frankfort Street facing southeast toward the 115 Gove Street parcel.	20.4%	50.8%
Viewpoint 6	View from the center of Frankfort Street facing southeast toward the Frankfort Street parcel.	0%	64.9%

Table 4.3-1 Daylight Analysis Results (Continued)

Viewpoint Loca	tions	Existing Conditions	Proposed Conditions
Area Context Po	pints		
AC1	View from the center of Gove Street facing southwest toward 99 Gove Street	69.3%	N/A
AC2	View from the center of Orleans Street facing northwest toward 150 Orleans Street	83.1%	N/A
AC3	View from the center of Orleans Street facing southeast toward 117 Orleans Street	69.7%	N/A

Lubec Street - Viewpoint 1

Lubec Street runs along the southern portion of the 128-134 Gove Street parcel. Viewpoint 1 was taken from the center of Lubec Street facing northwest toward the parcel. Since the existing Rectory Building that currently occupies the site will be demolished and replaced with surface parking and green space, the development of the Project would result in a lower daylight obstruction value of 9.8%.

Gove Street - Viewpoint 2

Gove Street runs along the western portion of the 120 Gove Street parcel and the 128-134 Gove Street parcel. Viewpoint 2 was taken from the center of Gove Street facing northeast toward the 128-134 Gove Street and the Church Building. Since the existing Rectory Building that currently occupies the site will be demolished and replaced with surface parking and green space, the development of the Project would result in a lower daylight obstruction value of 24.3%.

Gove Street - Viewpoint 3

Gove Street runs along the southern portion of the 120 Gove Street parcel. Viewpoint 3 was taken from the center of Gove Street facing northeast toward the Church Building. As described earlier, the Church Building will remain on the Project site and therefore the daylight obstruction value will remain the same.

Gove Street - Viewpoint 4

Gove Street runs along the eastern portion of the 115 Gove Street parcel. Viewpoint 4 was taken from the center of Frankfort Street facing southwest toward the 115 Gove Street parcel. The existing condition includes a three-story Convent Building that is set back from the property line, resulting in a lower daylight obstruction value of 18.3% for Viewpoint 4. The development of the Project will result in a daylight obstruction value of 49.1%. While

this is an increase over existing conditions, the Project will have a daylight obstruction value similar to buildings in the vicinity of the Project site, including the Area Context buildings.

Gove Street - Viewpoint 5

Gove Street runs along the northern portion of the 115 Gove Street parcel. Viewpoint 5 was taken from the center of Gove Street facing southeast toward the 115 Gove Street parcel. The existing condition includes a three-story Convent Building that is set back from the property line, resulting in a lower daylight obstruction value of 20.4% for Viewpoint 5. The Proposed Project will increase the daylight obstruction value to 50.8%. While this is an increase over existing conditions, the Project will have a daylight obstruction similar to buildings in the vicinity of the Project, including the Area Context buildings.

Frankfort Street - Viewpoint 6

Frankfort Street runs along the northern portion of the Frankfort Street parcel. Viewpoint 6 was taken from the center of Frankfort Street facing the Frankfort Street parcel. The parcel is currently vacant, resulting in an existing daylight obstruction value of 0%. The development of the Project will result in a daylight obstruction value of 64.9%. While this is an increase over existing conditions, the Project will have a daylight obstruction similar to buildings in the vicinity of the Project, including the Area Context buildings.

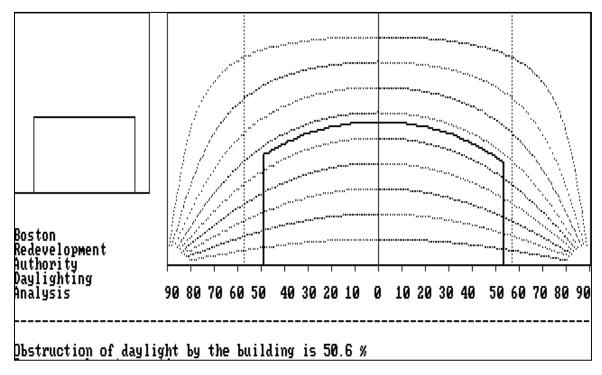
Area Context Viewpoints

The Project site is located in East Boston in an area with a mix of relatively low density residential and institutional uses and surface parking lots. To provide a larger context for comparison of daylight conditions, obstruction values were calculated for the three Area Context Viewpoints described above and shown on Figure 4.3-1. The daylight obstruction values ranged from 69.3% for AC1 and 83.1% for AC2. Daylight obstruction values for the Project are consistent with the Area Context values.

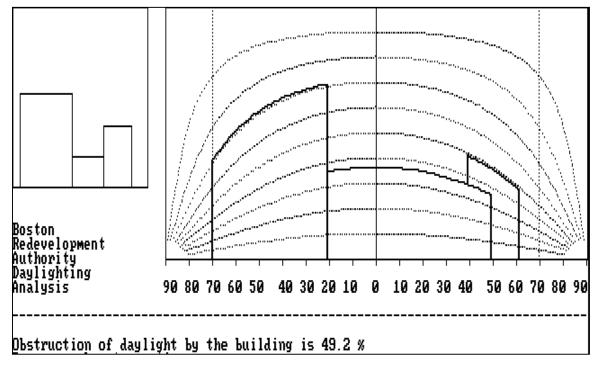
4.3.4 Conclusions

The daylight analysis conducted for the Project describes existing and proposed daylight obstruction conditions at the Project site and in the surrounding area. The results of the BRADA analysis indicate that while the development of the Project will result in increased daylight obstruction over existing conditions from certain locations, the resulting conditions will be similar to or less than the daylight obstruction values within the surrounding area. The design includes setbacks from the streets, space between buildings, open space, and a variety of heights that allow for views of the sky and minimizes daylight impacts.

Viewpoint 1: View from Lubec Street facing northwest toward the 128-134 Gove Street parcel



Viewpoint 2:View from Gove Street facing north toward the 128-134 parcel and the Church Building

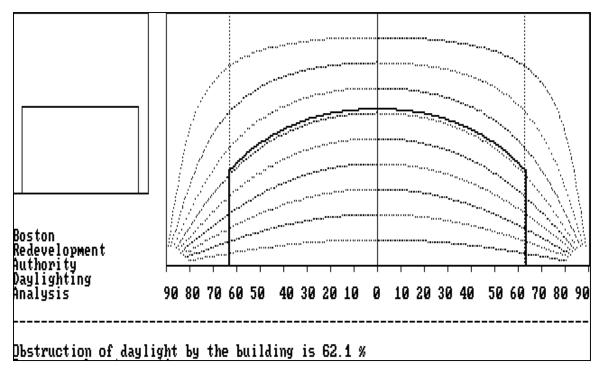


Frankfort Gove Street Housing

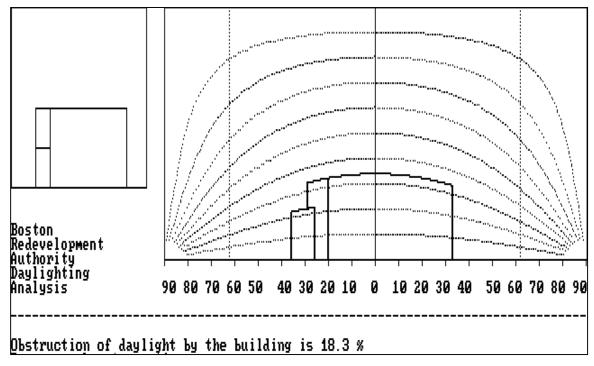
Boston, Massachusetts



Viewpoint 3: View from Frankfort Street facing southeast toward the Church Building



Viewpoint 4:View from Gove Street facing southwest toward the 115 Gove Street parcel

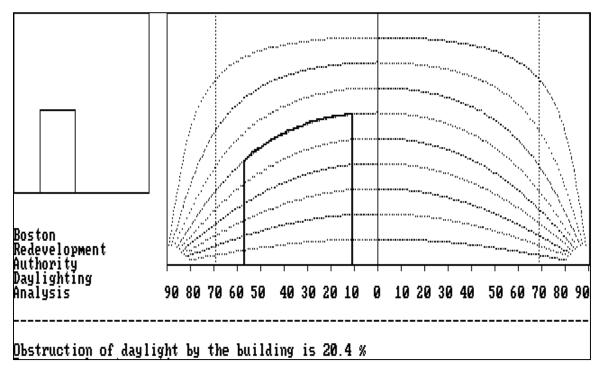


Frankfort Gove Street Housing

Boston, Massachusetts



Viewpoint 5: View from Frankfort Street facing southeast toward the 115 Gove Street parcel

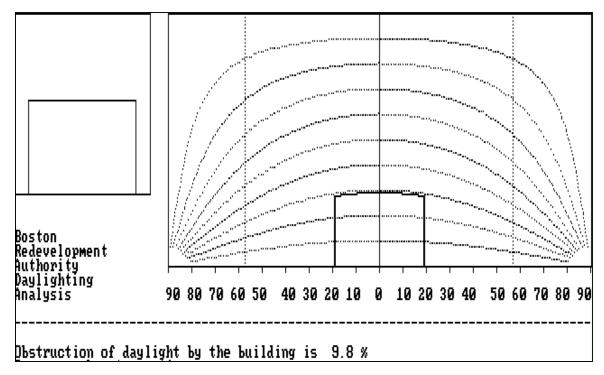


Viewpoint 6:View from Frankfort Street facing southeast toward the Frankfort Street parcel

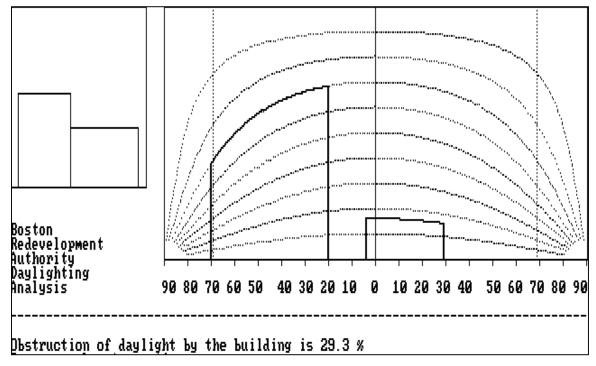
0% since the lot is vacant



Viewpoint 1: View from Lubec Street facing northwest toward the 128-134 Gove Street parcel



Viewpoint 2:View from Gove Street facing north toward the 128-134 parcel and the Church Building

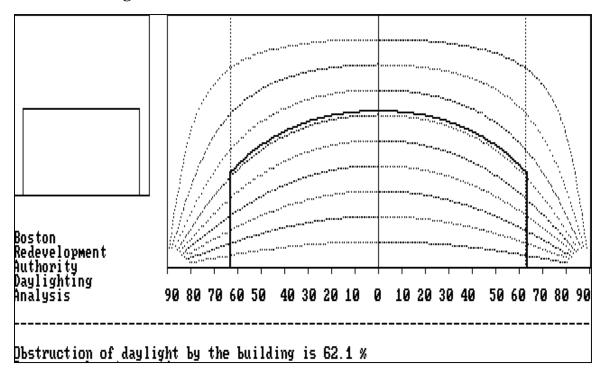


Frankfort Gove Street Housing

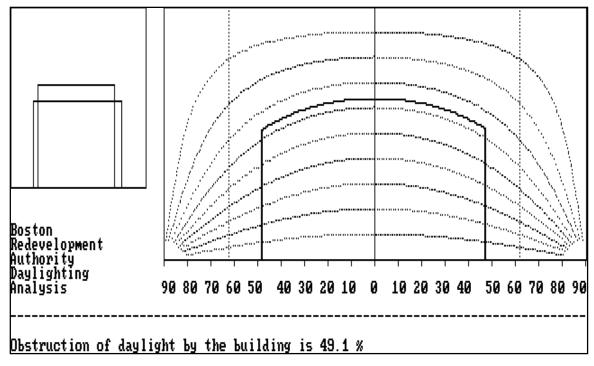
Boston, Massachusetts



Viewpoint 3: View from Frankfort Street facing southeast toward the Church Building



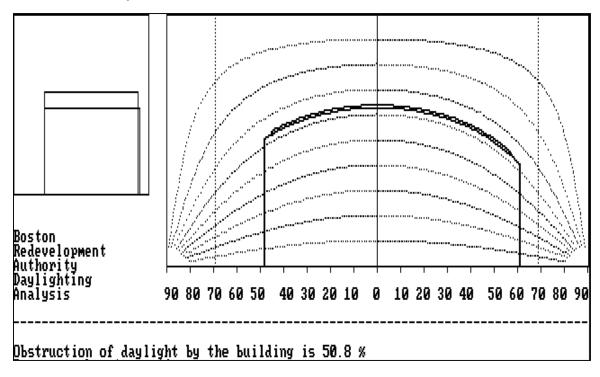
Viewpoint 4:View from Gove Street facing southwest toward the 115 Gove Street parcel



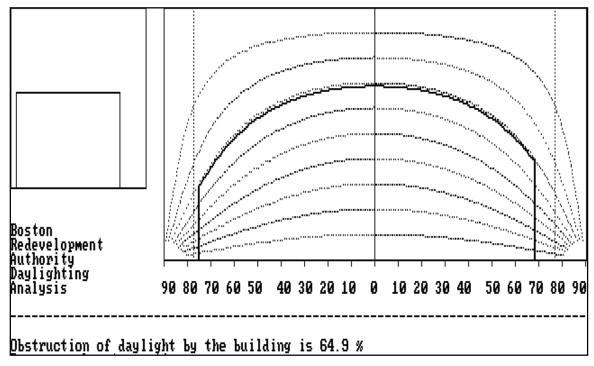




Viewpoint 5: View from Frankfort Street facing southeast toward the Frankfort Street parcel



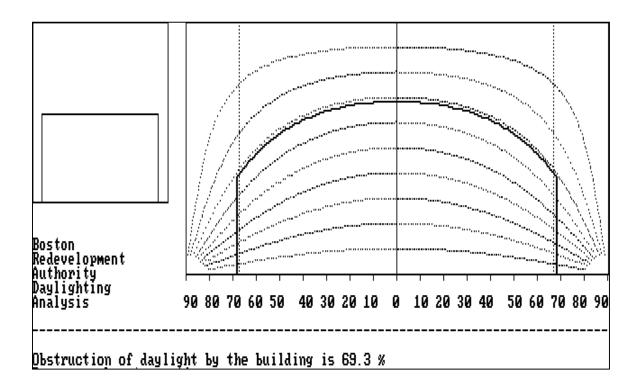
Viewpoint 6:View from Frankfort Street facing southeast toward the Frankfort Street parcel



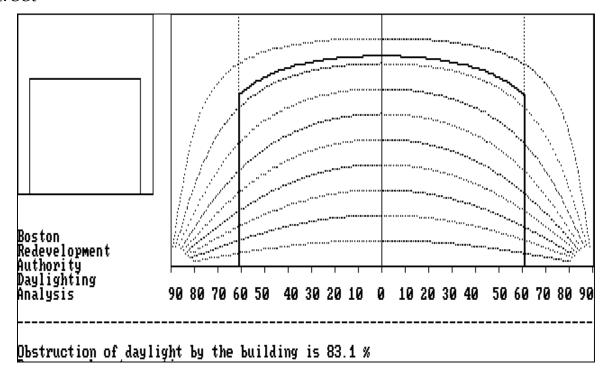




AC1: View from Gove Street facing southwest toward 99 Gove Street

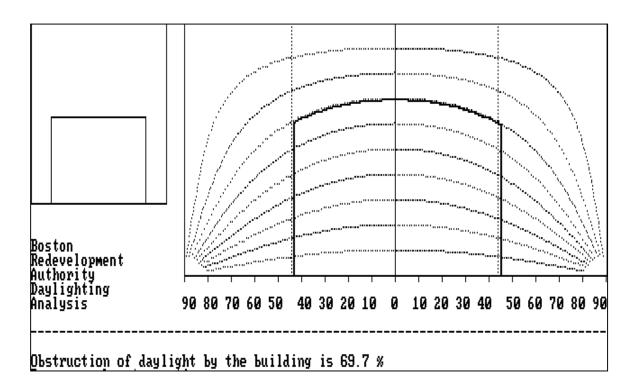


AC2:View from Orleans Street facing northwest toward 150 Orleans Street





AC3: View from Orleans Street facing southeast toward 117 Orleans Street



4.4 Solar Glare

The Project materials are still being studied and glazing of the windows will be determined as the design progresses. Due to the type of potential glass and glazing proposed, solar glare impacts are not currently anticipated.

4.5 Air Quality

4.5.1 Introduction

BPDA requires that proposed projects evaluate the air quality in the local area, and assess any adverse air quality impacts attributable to the project. The BPDA guidelines state that impacts from stationary sources (boilers, engines) and mobile sources (vehicles) must be addressed.

4.5.2 BPDA Air Quality Analysis Requirements

BPDA Guidelines state:

A mesoscale analysis predicting the change in regional emissions of volatile organic compounds ("VOCs") and nitrogen oxides ("NOx") should be performed for projects that generate more than 10,000 vehicle trips per day. The above analyses shall be conducted in accordance with the modeling protocols established by the Massachusetts Department of Environmental Protection (and the U.S. Environmental Protection Agency.

For this Project, the vehicle trip threshold is not exceeded. Therefore, a mesoscale analysis was not prepared.

BPDA Guidelines also state:

A microscale analysis predicting localized carbon monoxide concentrations should be performed, including identification of any locations projected to exceed the National or Massachusetts Ambient Air Quality Standards, for projects in which: 1) project traffic would impact intersections or roadway links currently operating at Level of Service ("LOS") D, E, or F or would cause LOS to decline to D, E, or F; 2) project traffic would increase traffic volumes on nearby roadways by 10% or more (unless the increase in traffic volume is less than 100 vehicles per hour); or, 3) the project will generate 3,000 or more new average daily trips on roadways providing access to a single location.

For this Project, the transportation analysis shows that Project traffic shows no intersections are currently operating at LOS D or worse, or projected to operate at LOS D or worse for future cases. Therefore a microscale analysis is not required. All intersections evaluated in

the transportation analysis in Section 2.0 are below the BPDA thresholds requiring a microscale analysis of carbon monoxide. Additionally, the Project doesn't generate enough traffic to require a mesoscale vehicle emissions quantification analysis.

Any new stationary sources will be reviewed by the Massachusetts Department of Environmental Protection during permitting under the Environmental Results Program, as required. It is expected that all stationary sources will be small, and any impacts from stationary sources would be insignificant.

Therefore, a qualitative assessment of current air quality in the area is presented.

4.5.3 National Ambient Air Quality Standards and Background Concentrations

Background air quality concentrations and federal air quality standards were utilized to conduct the above air quality impact analyses. Federal National Ambient Air Quality Standards (NAAQS) were developed by the U.S. Environmental Protection Agency (EPA) to protect the human health against adverse health effects with a margin of safety. The modeling methodologies were developed in accordance with the latest Massachusetts Department of Environmental Protection (MassDEP) modeling policies and Federal modeling guidelines.² The following sections outline the NAAQS standards and detail the sources of background air quality data.

4.5.3.1 National Ambient Air Quality Standards

The 1970 Clean Air Act was enacted by the U.S. Congress to protect the health and welfare of the public from the adverse effects of air pollution. As required by the Clean Air Act, EPA promulgated NAAQS for the following criteria pollutants: nitrogen dioxide (NO₂), sulfur dioxide (SO₂), particulate matter (PM) (PM-10 and PM-2.5), carbon monoxide (CO), ozone (O₃), and lead (Pb). The NAAQS are listed in Table 4.5-1. Massachusetts Ambient Air Quality Standards (MAAQS) are typically identical to NAAQS (differences are highlighted in **bold** in Table 4.5-1).

NAAQS specify concentration levels for various averaging times and include both "primary" and "secondary" standards. Primary standards are intended to protect human health, whereas secondary standards are intended to protect public welfare from any known or anticipated adverse effects associated with the presence of air pollutants, such as damage to vegetation. The more stringent of the primary or secondary standards were applied when comparing to the modeling results for this Project.

⁴⁰ CFR 51 Appendix W, Guideline on Air Quality Models, 70 FR 68228, Nov. 9, 2005

The NAAQS also reflect various durations of exposure. The non-probabilistic short-term periods (24 hours or less) refer to exposure levels not to be exceeded more than once a year. Long-term periods refer to limits that cannot be exceeded for exposure averaged over three months or longer.

Table 4.5-1 National (NAAQS) and Massachusetts (MAAQS) Ambient Air Quality Standards

	Averaging	NAAQS (µg/m³)		MAAQS (µg/m³)		
Pollutant	Period	Primary	Secondary	Primary	Secondary	
NO ₂	Annual (1)	100	Same	100	Same	
INO2	1-hour (2)	188	None	None	None	
	Annual (1)(9)	80	None	80	None	
SO ₂	24-hour (3)(9)	365	None	365	None	
302	3-hour (3)	None	1300	None	1300	
	1-hour (4)	196	None	None	None	
PM-2.5	Annual (1)	12	15	None	None	
F/M-2.5	24-hour (5)	35	Same	None	None	
PM-10	Annual (1)(6)	None	None	50	Same	
PM-10	24-hour (3)(7)	150	Same	150	Same	
СО	8-hour (3)	10,000	Same	10,000	Same	
CO	1-hour (3)	40,000	Same	40,000	Same	
Ozone	8-hour (8)	14 <i>7</i>	Same	235	Same	
Pb	3-month (1)	1.5	Same	1.5	Same	

⁽¹⁾ Not to be exceeded.

4.5.3.2 Background Concentrations

To estimate background pollutant levels representative of the area, the most recent air quality monitor data reported by the MassDEP in their Annual Air Quality Reports was obtained for 2014 to 2016. The three-hour and 24-hour SO₂ values are no longer reported in the annual reports. Data for these pollutant and averaging time combinations were obtained from the EPA's AirData website.

The Clean Air Act allows for one exceedance per year of the CO and SO₂ short-term NAAQS per year. The highest second-high accounts for the one exceedance. Annual NAAQS are never to be exceeded. The 24-hour PM-10 standard is not to be exceeded more than once per year on average over three years. To attain the 24-hour PM-2.5 standard, the three-year average of the 98th percentile of 24-hour concentrations must not exceed 35 μ g/m³. For annual PM-2.5 averages, the average of the highest yearly

^{(2) 98}th percentile of one-hour daily maximum concentrations, averaged over three years.

⁽³⁾ Not to be exceeded more than once per year.

^{(4) 99}th percentile of one-hour daily maximum concentrations, averaged over three years.

^{(5) 98}th percentile, averaged over three years.

⁽⁶⁾ EPA revoked the annual PM-10 NAAQS in 2006.

⁽⁷⁾ Not to be exceeded more than once per year on average over three years.

⁽⁸⁾ Annual fourth-highest daily maximum eight-hour concentration, averaged over three years.

⁽⁹⁾ EPA revoked the annual and 24-hour SO₂ NAAQS in 2010. However, they remain in effect until one year after the area's initial attainment designation, unless designated as "nontattinment".

observations was used as the background concentration. To attain the one-hour NO₂ standard, the three-year average of the 98th percentile of the maximum daily one-hour concentrations must not exceed 188 $\mu g/m^3$.

Background concentrations were determined from the closest available monitoring stations to the proposed development. All pollutants are not monitored at every station, so data from multiple locations are necessary. The closest monitor is at 174 North Street (1.1 miles west-southwest), but this site only samples PM-2.5. The next closest site is at Kenmore Square, roughly 3.5 miles west-southwest of the Project location. However, this site only samples for SO₂ and NO₂. A site on Harrison Avenue is roughly 3.7 miles southwest of the Project. This site samples for the remaining pollutants. A summary of the background air quality concentrations are presented in Table 4.5-2.

Table 4.5-2 Observed Ambient Air Quality Concentrations and Selected Background Levels

Pollutant	Averaging Time	2014	2015	2016	Background Concentration (µg/m³)	NAAQS	Percent of NAAQS
	1-Hour (5)	25.4	14.4	10.7	16.9	196.0	9%
CO (1)(C)	3-Hour	24.6	11.5	10.0	24.6	1300.0	2%
SO ₂ (1)(6)	24-Hour	13.1	7.6	5.2	13.1	365.0	4%
	Annual	2.5	1.3	1.1	2.5	80.0	3%
D) 4 10	24-Hour	53.0	30.0	30.0	53.0	150.0	35%
PM-10	Annual	14.9	14.2	14.1	14.9	50.0	30%
D) 4 2 F	24-Hour (5)	14.4	16.7	14.7	15.2	35.0	44%
PM-2.5	Annual (5)	6.9	7.3	7.7	7.3	12.0	61%
NO (2)	1-Hour (5)	92.1	105.3	88.4	95.3	188.0	51%
NO ₂ (3)	Annual	32.3	32.5	28.3	32.5	100.0	33%
CO (2)	1-Hour	1963.1	1560.9	2750.4	2750.4	40000.0	7%
CO (2)	8-Hour	1489.8	1031.4	2062.8	2062.8	10000.0	21%
Ozone (4)	8-Hour	106.0	109.9	113.9	113.9	147.0	77%
Lead	Rolling 3- Month	0.014	0.016	0.017	0.017	0.15	12%

Notes:

From 2012-2014 EPA's AirData Website

- (1) SO₂ reported ppb. Converted to $\mu g/m^3$ using factor of 1 ppm = 2.62 $\mu g/m^3$.
- (2) CO reported in ppm. Converted to $\mu g/m^3$ using factor of 1 ppm = 1146 $\mu g/m^3$.
- (3) NO₂ reported in ppb. Converted to $\mu g/m^3$ using factor of 1 ppm = 1.88 $\mu g/m^3$.
- (4) O₃ reported in ppm. Converted to $\mu g/m^3$ using factor of 1 ppm = 1963 $\mu g/m^3$.
- (5) Background level is the average concentration of the three years.
- (6) The 24-hour and Annual standards were revoked by EPA on June 22, 2010, Federal Register 75-119, p. 35520.

Air quality in the vicinity of the Project site is generally good, with all local background concentrations found to be well below the NAAQS.

4.5.4 Stationary Sources

Stationary sources of air pollution are typically units that combust fuel. In this case, these sources consist of heating and hot water units and emergency electrical generators. Cooling towers, although not a combustion source, are a source of particulate emissions.

It is expected that the majority of stationary sources (boilers, engines, etc.) may be subject to the MassDEP's Environmental Results Program (ERP). The Proponent will complete the required applications and submittals for the equipment, as necessary.

4.5.5 Mobile Sources

Mobile sources of air pollution include gasoline, diesel, and natural gas fueled vehicles. Emissions from mobile sources have continually decreased as engine technology and efficiency have been improved.

Given that the Project does not significantly increase vehicle volumes, and does not affect any already poorly functioning intersections, it can be reasonably assumed that the vehicle trips generated by the Project will not cause adverse air quality impacts in the area.

4.6 Stormwater/Water Quality

Section 8.0 includes a discussion of stormwater and water quality.

4.7 Flood Hazard Zones/Wetlands

The most current version of the Federal Emergency Management Agency (FEMA) Flood Insurance Rate Map (FIRM) for the site located in the City of Boston – Community Panel Number 25025C0081J, effective March 16, 2016, indicates the FEMA Flood Zone Designations for the site area. The map shows that the Project is located within the 100-year flood zone.

The design team is studying the incorporation of a number of measures to mitigate against flood impacts, including:

- Placing essential mechanical equipment above the future flood level;
- Water-tight utility conduits;
- Wastewater backflow prevention; and
- Reliant materials on the first floor that can either withstand flooding or easily be replaced.

The Project site does not contain wetlands.

4.8 Geotechnical Impacts

Design Consultants, Inc. (DCI) has provided a summary report for the geotechnical investigations at the former Our Lady of Mount Carmel property and associated buildings located at the corner of Gove Street and Frankfort Street in East Boston. The Project consists of the following:

- ◆ Demolishing the existing Convent Building (115 Gove Street) and removing the adjoining parking lot on Frankfort Street;
- Removing the existing Rectory Building and its addition (120 Gove Street);
- Rehabilitating the Church Building (120 Gove Street);
- Constructing a new four-story structure at 128 Gove Street and along Frankfort Street to Gove Street.

The proposed structure along Frankfort Street is intended to have an underground garage. The lower garage area will be accessed by from Frankfort Street. It is also anticipated that the new structures will be connected to the existing Boston Water and Sewer utilities.

DCI conducted subsurface investigations and prepared the following geotechnical engineering summary, below. This section summarizes the findings and conclusions from the study. The primary objective of the investigations was to obtain sufficient subsurface data to assist with the preliminary design and earthwork program.

4.8.1 Sub-soil Conditions

DCI worked with EBI Consultants, Inc. (EBI) to develop the subsurface investigation plan discussed below and shown in Attachment C. Borings were observed by a DCI geotechnical engineer. Boring logs are provided in Attachment C and are summarized in Table 4.8-1.

A total of ten borings (B-1 through B-10) were located at the site. Two borings (B-1 and B-2) were located north of the existing Rectory Building; two borings were located in Gove Street (B-3 and B-4); and six borings (B-5 through B-10) were located south of the former convent and in the vacant parking lot. DCI developed coordinates for the ten boring locations and surveyed them at the site to sub-meter accuracy.

The borings were drilled to depths ranging from 29- to 42-feet. Borings were advanced using a combination of hollow stem auger and drive-and-wash drilling techniques. Split spoon samples and standard penetration test (SPTs) were typically collected at the surface and at approximate five-foot intervals. Continuous samples were collected at times to identify potential unsuitable materials, such as organics.

Table 4.8-1 Summary of Boring Data

Borehole	Approx. Ground Elevation	Est. Bottom of Fill		Est. Bottom of Organic Sand/Silt/Clay and Peat		Boring Depth
Dorenoic	(Boston City Base)	Depth (ft)	Elevation	Depth (ft)	Elevation	(ft)
B-1	18.0	8.5	9.5	19.3	-1.3	29.0
B-2	18.0	6.5	11.5	18.5	-0.5	32.0
B-3	17.0	10.0	7	17.0	0	29.0
B-4	17.5	10.5	7	16.0	1.5	29.0
B-5	17.5	14.0	3.5	18.5	-1.0	37.0
B-6	17.5	14.5	3	18.5	-1.0	37.0
B-7	17.5	12.5	5	20.0	-2.5	29.0
B-8	17.5	10.0	7.5	18.5	-1.0	32.0
B-9	17.5	11.0	6.5	18.0	-0.5	42.0
B-10	17.5	10.5	7	17.0	0.5	32.0

The borings generally encountered the following subsurface conditions from the ground surface to depth:

<u>Concrete and Asphalt</u> – Seven of the ten borings encountered a four-inch thick layer of concrete (B-1 and B-2) or a one- to six-inch thick layer of old asphalt (B-3, B-4, B-8, B-9 and B-10) at the ground surface.

<u>Topsoil</u> – Three borings (B-5, B-6 and B-7) encountered a two- to four-inch thick topsoil layer consisting of a silty sand with little organics.

<u>Urban Fill</u> – All borings encountered a near surface fill layer consisting of sand with little to some silt and trace to little gravel, or sand and silt (silt and sand) with varying amounts of gravel. The fill also contained trace to little glass, brick, coal ash and wood. The fill was encountered to an estimated depth of 3.5-feet (B-5) to 11.5-feet (B-2). SPTs indicate that the material is generally loose to medium dense, or soft to medium stiff.

Organic Sand/Silt/Clay and Peat – Below the fill, all borings encountered a layer of stratified sand, silt and clay with varying amounts of organic material. Distinct layers of peat were encountered in this zone. In four borings (B-1, B-2, B-4 and B-5) the peat layers varied for 2- to 18-inches-thick. In the remaining borings (B-3, and B-6 through B-10) the peat layer was more significant and ranged from two- to seven-feet thick. Clay and Silt – Three borings (B-1, B-2 and B-3) encountered a layer of clay and .The underlying clay and silt layer is medium stiff to very stiff.

<u>Stratified Sand, Silt and Clay</u> – Several borings (B-3 through B-10) encountered stratified layers of sand, silt and clay to the bottom of the borehole. The stratified sand, silt and clay ranges from being medium dense to dense to medium stiff to very stiff.

4.8.2 Groundwater

Groundwater observations were made at the time of drilling between April 19 and April 24, 2017. DCI'S observations are provided in Table 4.8-2. Groundwater was observed to be about 9- to 10-feet (about El. 8.5 to El. 7.0) below the existing ground surface. Groundwater observations are expected to fluctuate and higher levels will be factored into the foundation design.

Table 4.8-2 Summary of Groundwater Observations

Borehole	Approx. Ground Borehole Elevation	Date _	Est. Groundwater Observation		
Doronoic	(Boston City Base)		Depth (ft)	Elevation	
B-1	18.0	4/21/17	10.0	8.0	
B-2	18.0	4/21/17	10.0	8.0	
B-3	17.0	4/20/17	10.0	7.0	
B-4	1 <i>7</i> .5	4/20/17	10.0	<i>7</i> .5	
B-5	1 <i>7</i> .5	4/24/17	9.0	8.5	
B-6	17.5	4/24/17	9.0	8.5	
B-7	17.5	4/20/17	10.0	7.5	
B-8	17.5	4/19/17	10.0	7.5	
B-9	17.5	4/19/17	10.0	<i>7</i> .5	
B-10	17.5	4/19/17	10.0	7.5	

4.8.3 Groundwater Conservation Overlay District

The site is not located within a Groundwater Conservation Overlay District as outline in Article 32 of the City of Boston Zoning Code.

4.9 Solid and Hazardous Waste

4.9.1 Hazardous Waste

Prior to any demolition or renovation to the existing structures a survey for asbestos will be performed in accordance with applicable Federal, State, and local regulatory requirements (including AHERA and 310 CMR 7.00) as well as applicable asbestos-industry standards.

Lead-containing paint survey will be conducted for compliance with OSHA Lead in Construction regulation CFR 1926.62. A survey will also be performed for other hazardous materials including PCBs and Universal Wastes.

4.9.2 Operation Solid and Hazardous Waste Generation

The Project will generate solid waste typical of other residential projects. Solid waste generated by the Project will be approximately 106 tons per year, based on the number of bedrooms proposed at a generation rate of four pounds (lbs) per bedroom per day. Other than typical wastes generated by residential use (e.g., paint, detergents, etc.), no hazardous wastes are anticipated to be generated by the Project.

4.9.3 Recycling

Recyclable materials will be recycled through a program implemented by building management.

The building will include areas for trash collection and recycling collection on each floor, and a trash room in close proximity to the loading dock. Recycling facilities will be provided on-site for paper, glass, plastic and metal.

4.10 Noise Impacts

The City of Boston has both a noise ordinance and noise regulations. Chapter 16 §26 of the Boston Municipal Code sets the general standard for noise that is unreasonable or excessive: louder than 50 decibels between the hours of 11:00 p.m. and 7:00 a.m., or louder than 70 decibels at all other hours. The Boston Air Pollution Control Commission (APCC) has adopted regulations based on the City's ordinance – "Regulations for the Control of Noise in the City of Boston", which distinguish among residential, business, and industrial districts in the City. In particular, APCC Regulation 2 is applicable to the sounds from the Proposed Project.

Table 4.10-1 below presents the "Zoning District Noise Standards" contained in Regulation 2.5 of the APCC "Regulations for the Control of Noise in the City of Boston," adopted December 17, 1976. These maximum allowable sound pressure levels apply at the line of the receiving property. Zoning District Standards are presented below in Table 4.10-1.

Table 4.10-1 City of Boston Zoning District Noise Standards, Maximum Allowable Sound Pressure Levels

Octave-band	Residential		Residential-Industrial		Business	Industrial
Center	Zoning	Zoning District Zoning Distr		Zoning District		Zoning District
Frequency	Daytime	All Other	Daytime	All Other	Anytime	Anytime
(Hz)	(dB)	Times (dB)	(dB)	Times (dB)	(dB)	(dB)
32	76	68	79	72	79	83
63	75	67	78	<i>7</i> 1	78	82
125	69	61	73	65	73	77
250	62	52	68	57	68	73
500	56	46	62	51	62	67
1000	50	40	56	45	56	61
2000	45	33	51	39	51	5 <i>7</i>
4000	40	28	47	34	47	53
8000	38	26	44	32	44	50
A-Weighted	60	50	65	55	65	<i>7</i> 0
(dBA)						

Notes: Noise standards are extracted from Regulation 2.5, City of Boston Air Pollution Control Commission, "Regulations for the Control of Noise in the City of Boston", adopted December 17, 1976.

All standards apply at the property line of the receiving property.

dB and dBA based on a reference sound pressure of 20 micropascals.

'Daytime' refers to the period between 7:00 a.m. and 6:00 p.m. daily, excluding Sunday.

Additionally, the MassDEP has the authority to regulate noise under 310 CMR 7.10, which is part of the Commonwealth's air pollution control regulations. According to MassDEP, "unnecessary" noise is considered an air contaminant and thus prohibited by 310 CMR 7.10. The MassDEP administers this regulation through Noise Policy DAQC 90-001 which limits a source to a 10-dBA increase above the L90 ambient sound level measured at the Project property line and at the nearest residences. The MassDEP policy further prohibits "pure tone" conditions where the sound pressure level in one octave-band is 3 dB or more than the sound levels in each of two adjacent bands.

While the details of the mechanical equipment associated with the Project have not yet been precisely determined, steady operational noise from stationary sources will primarily involve heating, cooling, and ventilation equipment for the residential units. During the final design phase of the Project, mechanical equipment will be specified to meet the applicable City of Boston and MassDEP noise limits. Reasonable efforts will be made, if necessary, to minimize noise impacts from the Project using routinely employed methods of noise control.

With appropriate noise control, the Project is not expected to result in any adverse noise impacts at nearby sensitive receptors. Short-term, intermittent increases in noise levels will occur during Project construction. However, every reasonable effort will be made to minimize the noise impacts and ensure the Project complies with the requirements of the City of Boston noise ordinance.

4.11 Construction Impacts

4.11.1 Introduction

A Construction Management Plan (CMP) in compliance with the City's Construction Management Program will be submitted to the Boston Transportation Department (BTD) once final plans are developed and the construction schedule is fixed. The construction contractor will be required to comply with the details and conditions of the approved CMP.

Proper pre-planning with the City and neighborhood will be essential to the successful construction of the Project. Construction methodologies, which ensure public safety and protect nearby residences and businesses, will be employed. Techniques such as barricades, walkways and signage will be used. The CMP will include routing plans for trucking and deliveries, plans for the protection of existing utilities, and control of noise and dust.

During the construction phase of the Project, the Proponent will provide the name, telephone number and address of a contact person to communicate with on issues related to the construction.

The Proponent intends to follow the guidelines of the City of Boston and the MassDEP, which direct the evaluation and mitigation of construction impacts.

4.11.2 Construction Methodology/Public Safety

Construction methodologies that ensure public safety and protect nearby tenants will be employed. Techniques such as barricades and signage will be used. Construction management and scheduling will minimize impacts on the surrounding environment and will include plans for construction worker commuting and parking, routing plans for trucking and deliveries, and the control of noise and dust.

As the design of the Project progresses, the Proponent will meet with BTD to discuss the specific location of barricades, the need for lane closures, pedestrian walkways, and truck queuing areas. Secure fencing, signage, and covered walkways may be employed to ensure the safety and efficiency of all pedestrian and vehicular traffic flows. In addition, sidewalk areas and walkways near construction activities will be well marked and lighted to protect pedestrians and ensure their safety. Public safety for pedestrians on abutting sidewalks will also include covered pedestrian walkways when appropriate. If required by BTD and the

Boston Police Department, police details will be provided to facilitate traffic flow. These measures will be incorporated into the CMP which will be submitted to BTD for approval prior to the commencement of construction work.

4.11.3 Construction Schedule

It is anticipated that construction activities will start in the third quarter of 2019, with completion by the first quarter of 2021. The Project is proposed to be built in one phase; however, it may be necessary to separate the rehabilitation of the Church Building from the construction of the Frankfort Street Building.

Typical construction hours will be from 7:00 a.m. to 6:00 p.m., Monday through Friday, with most shifts ordinarily ending at 3:30 p.m. No substantial sound-generating activity will occur before 7:00 a.m. If longer hours, additional shifts, or Saturday work is required, the construction manager will place a work permit request to the Boston Air Pollution Control Commission and BTD in advance. Notification should occur during normal business hours, Monday through Friday. It is noted that some activities such as finishing activities could run beyond 6:00 p.m. to ensure the structural integrity of the finished product; certain components must be completed in a single pour, and placement of concrete cannot be interrupted.

4.11.4 Construction Staging/Access

Access to the Project site and construction staging areas will be provided in the CMP.

Although specific construction and staging details have not been finalized, the Proponent and its construction management consultant will work to ensure that staging areas will be located to minimize impacts to pedestrian and vehicular flow. Secure fencing and barricades will be used to isolate construction areas from pedestrian traffic adjacent to the Project site. Construction procedures will be designed to meet all Occupational Safety and Health Administration (OSHA) safety standards for specific site construction activities.

4.11.5 Construction Mitigation

The Proponent will follow City and MassDEP guidelines which will direct the evaluation and mitigation of construction impacts. As part of this process, the Proponent and construction team will evaluate the Commonwealth's Clean Air Construction Initiative.

A CMP will be submitted to BTD for review and approval prior to issuance of a Building Permit. The CMP will include detailed information on specific construction mitigation measures and construction methodologies to minimize impacts to abutters and the local community. The CMP will also define truck routes which will help in minimizing the impact of trucks on City and neighborhood streets.

"Don't Dump - Drains to Boston Harbor" plaques will be installed at storm drains that are replaced or installed as part of the Project.

4.11.6 Construction Employment and Worker Transportation

The number of workers required during the construction period will vary. It is anticipated that approximately 124 construction jobs will be created over the length of construction. The Proponent will make reasonable good-faith efforts to have at least 51% of the total employee work hours be for Boston residents, at least 40% of total employee work hours be for minorities and at least 12% of the total employee work hours be for women. The Proponent will enter into jobs agreements with the City of Boston.

Construction worker parking will be available at the Project site, however, all workers will be strongly encouraged to use public transportation and ridesharing options. The general contractors will work aggressively to ensure that construction workers are well informed of the public transportation options serving the area. Space on-site will be made available for workers' supplies and tools so they do not have to be brought to the Project site each day.

4.11.7 Construction Truck Routes and Deliveries

Truck traffic will vary throughout the construction period, depending on the activity. The construction team will manage deliveries to the Project site during morning and afternoon peak hours in a manner that minimizes disruption to traffic flow on adjacent streets. Construction truck routes to and from the Project site for contractor personnel, supplies, materials, and removal of excavations required for the development will be coordinated with BTD. Traffic logistics and routing will be planned to minimize community impacts. Truck access during construction will be determined by the BTD as part of the CMP. These routes will be mandated as a part of all subcontractors' contracts for the development. The construction team will provide subcontractors and vendors with Construction Vehicle & Delivery Truck Route Brochures in advance of construction activity.

"No Idling" signs will be included at the loading, delivery, pick-up and drop-off areas.

4.11.8 Construction Air Quality

Short-term air quality impacts from fugitive dust may be expected during demolition, excavation and the early phases of construction. Plans for controlling fugitive dust during demolition, excavation and construction include mechanical street sweeping, wetting portions of the Project site during periods of high wind, and careful removal of debris by covered trucks. The construction contract will provide for a number of strictly enforced measures to be used by contractors to reduce potential emissions and minimize impacts, pursuant to this Article 80 approval. These measures are expected to include:

Using wetting agents on areas of exposed soil on a scheduled basis;

- Using covered trucks;
- Minimizing spills on the construction site;
- Monitoring of actual construction practices to ensure that unnecessary transfers and mechanical disturbances of loose materials are minimized;
- Minimizing storage of debris on the construction site; and
- Periodic street and sidewalk cleaning with water to minimize dust accumulations.

4.11.9 Construction Noise

The Proponent is committed to mitigating noise impacts from the construction of the Project. Increased community sound levels, however, are an inherent consequence of construction activities. Construction work will comply with the requirements of the City of Boston Noise Ordinance. Every reasonable effort will be made to minimize the noise impact of construction activities.

Mitigation measures are expected to include:

- Instituting a proactive program to ensure compliance with the City of Boston noise limitation policy;
- Using appropriate mufflers on all equipment and ongoing maintenance of intake and exhaust mufflers;
- Muffling enclosures on continuously running equipment, such as air compressors and welding generators;
- Replacing specific construction operations and techniques by less noisy ones where feasible;
- Selecting the quietest of alternative items of equipment where feasible;
- Scheduling equipment operations to keep average noise levels low, to synchronize the noisiest operations with times of highest ambient levels, and to maintain relatively uniform noise levels;
- Turning off idling equipment; and
- Locating noisy equipment at locations that protect sensitive locations by shielding or distance.

4.11.10 Construction Waste

The Proponent will take an active role with regard to the reprocessing and recycling of construction waste. The disposal contract will include specific requirements that will ensure that construction procedures allow for the necessary segregation, reprocessing, reuse and recycling of materials when possible. For those materials that cannot be recycled, solid waste will be transported in covered trucks to an approved solid waste facility, per MassDEP Regulations for Solid Waste Facilities, 310 CMR 16.00. This requirement will be specified in the disposal contract. Construction will be conducted so that materials that may be recycled are segregated from those materials not recyclable to enable disposal at an approved solid waste facility.

4.11.11 Protection of Utilities

Existing public and private infrastructure located within the public right-of-way will be protected during construction. The installation of proposed utilities within the public way will be in accordance with the MWRA, BWSC, Boston Public Works, Dig Safe, and the governing utility company requirements. All necessary permits will be obtained before the commencement of the specific utility installation. Specific methods for constructing proposed utilities where they are near to, or connect with, existing water, sewer and drain facilities will be reviewed by BWSC as part of its Site Plan Review process.

4.11.12 Rodent Control

A rodent extermination certificate will be filed with each building permit application for the Project. Rodent inspection monitoring and treatment will be carried out before, during, and at the completion of all construction work for each phase of the Project, in compliance with the City's requirements.

Sustainable Design and Climate Change Preparedness

5.0 SUSTAINABLE DESIGN AND CLIMATE CHANGE PREPAREDNESS

5.1 Introduction

The Project site was formerly used as Our Lady of Mount Carmel Church and contains associated Rectory and Convent Buildings, and a parking lot. The main Church Building, which is located at 120 Gove Street, will be salvaged and renovated into residential use. The Rectory Building located at 128-134 Gove Street, and the Convent Building located across the street are both compromised structurally, and will therefore be removed. The development will serve to invigorate this section of East Boston and bring life to vacant buildings.

The Project will be constructed as distinct buildings along Gove Street and Frankfort Street. Overall, the Project will create 112 condominium units with associated parking and open space. The Church Building located on the corner of Frankfort and Gove Street will be restored and renovated into approximately 13 residential units. The second portion of the Project along Gove Street, which is the site of the former Rectory Building, will be transformed into a new landscaped corner with integrated parking to serve the Church Building renovation.

The proposed Frankfort Street Building will be located across Gove Street and will contain approximately 98 units. This building will be constructed to include two distinct designs. The first portion will be a six-story building with set-backs along the sixth floor. As the building moves along Frankfort Street, it will step down to four-stories, with a fourth-floor pullback matching the massing and height of the surrounding buildings. This section of the building will employ a row house style design to match context of the existing structures across the street as you move along Frankfort Street. Condominium sizes will include one-and two-bedroom units in flat and loft styles. There will be one common deck amenity and several private roof decks. Underground parking at Frankfort Street and some additional ground level parking at Gove Street will be provided.

Sustainability has been a priority for the Project from the beginning of design. The Project team aims to design the Project to LEEDv4-NC Silver standards. The team is focused on resiliency, durability, energy savings, and quality of living. Because the Project is in a BPDA flood zone, the team intends to mitigate potential damage and losses by placing living spaces and equipment above street level. Indoor environmental comfort and air quality, energy performance, and occupant access to the outdoors will all be incorporated into the buildings. The buildings will feature a variety of sustainable materials in both common and living spaces.

5.2 Sustainable Design

The Project team is currently targeting a LEED Silver level. 40 out of a possible 110 points in the LEEDv4 BD+C: New Construction and Major Renovation rating system have been identified as achievable and an additional 44 points are under ongoing consideration for eligibility and feasibility. As the Project develops, the final point total should place the Project within the anticipated range for silver, which requires 50 points.

Location and Transportation

The Project team has identified 4 achievable points out of a possible 16 points, with an additional potential 9 points which require further evaluation.

The points will be achieved through access to quality transit, and diverse land uses (i.e., food retail, community-serving retail, services, civic and community services, and community anchor uses). Additional points may be achieved under each of these credits, pending the outcome of calculations. Also, additional points are possible for development in a historic neighborhood, bicycle facilities, and a reduced parking permit, pending further research and Project team decisions.

Sustainable Sites

The Project team has identified 4 achievable points out of a possible 10 points, with an additional potential 5 points requiring further evaluation.

The achievable points are based on rainwater management, and a reduction of heat island effect of roofing, decking, and paving materials. Additional points may be available pending calculations of open space and vegetation available for habitat. A Site Assessment may also contribute. The Project team will continue to track and evaluate the feasibility of the open space and habitat credits, and further define the rainwater management and heat island mitigation efforts.

Water Efficiency

For the Water Efficiency category, the Project team was able to identify 3 target points out of a possible 11 points.

Landscape irrigation requirements will be achieved with the incorporation of drought-resistance plants and irrigation system efficiency. Indoor water use will be reduced at least 20%, and possibly up to 40%, using low-flow fixtures in residences and communal areas, as well as EnergyStar-rated eligible appliances. Full-building water meters will be installed, and the Project team is weighing the option of submetering.

Energy & Atmosphere

The Project team currently expects to achieve approximately 12 points of the possible 33 points. An additional 9 points may be possible pending further calculations and Project team decisions.

Enhanced commissioning is expected to be conducted, and exterior envelope commissioning may be conducted as well. Energy use will be optimized to achieve a goal of 20% reduction over baseline. Additional points may be possible pending final design of envelope and mechanical systems. Submetering of the energy systems is currently anticipated, and solar panels are under consideration. Achievement of the Enhanced Refrigerant Management credit depends on the final selection of mechanical equipment, and purchase of green power and/or carbon offsets are an option if additional points are necessary to meet certification goals.

Materials and Resources

Out of 13 possible Materials and Resources points, the Project team has identified 5 achievable points.

Three of the points will be achieved via specification of materials for which manufacturers provide disclosure and transparency, in form of Environmental Product Declarations (EPDs). The Project team will aim to use materials that are verified to minimize use and generation of harmful substances and to have improved life-cycle impacts. Raw materials sourced in a responsible manner will be specified where possible and cost-effective, and reuse of the Church Building will also contribute. Additional points may be available for use of greater quantities of environmentally preferable materials. The final number of points will depend on Project and materials cost tally. Also depending on quantities, exemplary performance points may be available for these credits.

An additional 2 points are anticipated for diverting waste streams from the landfill through recycling, reuse, and repurposing.

Indoor Environmental Quality

Indoor environmental quality is a priority for the Project. Out of 16 possible IEQ points, the team has identified 9 as achievable.

A Construction Indoor Air Quality Management Plan will be developed and implemented during construction for the benefit of the construction team as well as future occupants. Low-emitting products including paints and coatings, adhesives and sealants, flooring materials, composite woods, and ceilings, walls, and insulation will be specified.

Future building occupants will have a high degree of controllability over their spaces, including thermostats and operable windows in the units, and the HVAC systems will be designed to meet ASHRAE 55-2010 standards. Areas designed for chemical use and storage will be ventilated directly to the exterior, and entry mats and air filtration systems will prevent air contaminants from entering the building. Quality views will be available from most occupied spaces, and residents will enjoy acoustic isolation from exterior background noise and neighboring units.

Innovation and Design

The Project team has established numerous strategies for acquiring the points associated with both exemplary performance of some of the credits in other categories, as well as innovative ways to address sustainability not covered by existing credits in the Building Design & Construction rating system. The final strategies chosen for implementation will be determined based on final calculations and decisions from the Project team and will include a combination of approaches to earn at least 3 points, and up to all 6, offered in this category.

The exemplary strategies may include (pending final calculation numbers): minimum of 40 products from five manufacturers with qualified EPDs, a minimum of 40 products that meet the material ingredient reporting requirements will be installed, or 100 percent of the parking will be placed under cover to reduce heat island effect. Similarly, the innovative strategies implemented may include a green education campaign for occupants and visitors, or a green housekeeping policy for base building services. Additionally, the Project will earn 1 of the 6 points through the inclusion of a LEED Accredited Professional on the core Project team.

Regional Priority Credits

The 4 points available in this category are contingent upon meeting certain thresholds for credits in other categories, as determined by the USGBC. Out of 5 possibilities considered especially significant for the Project location (based on zip code), the 3 most achievable options for the Regional Priority category related to the following strategies: use of a high-priority site, rainwater management, and indoor water use reduction. Additional possibilities include renewable energy production and optimization of energy performance (threshold 8 points).

The points in this category are automatically awarded pending award of the original credits to which they are linked.

5.3 Climate Change Resilience

5.3.1 Introduction

Climate change conditions considered by the Project team include higher maximum and mean temperature, more frequent and longer extreme heat events, more frequent and longer droughts, more sever freezing rain and heavy rainfall events, and increased win gusts.

The expected life of the Project is anticipated to be approximately 50 years. Therefore, the Proponent has planned for climate-related conditions projected 50 years into the future. A copy of the completed Checklist is included in Attachment D. Given the preliminary level of design, the responses are also preliminary and may be updated as the Project design progresses.

5.3.2 Extreme Heat Events

The Intergovernmental Panel on Climate Change (IPCC) has predicted that in Massachusetts the number of days with temperatures greater than 90°F will increase from the current five-to-twenty days annually, to thirty-to-sixty days annually. The Project design will incorporate a number of measures to minimize the impact of high temperature events, including:

- ♦ Installing operable windows where possible;
- Planting shade trees around the site;
- Installing a high performance building envelope; and
- Specifying high reflective paving materials, high albedo roof tops and green roofs to minimize the heat island effect.

Energy modeling for the Project has not yet been completed; however, the Proponent will strive to reduce the Project's overall energy demand and greenhouse gas emissions that contribute to global warming. The Proponent will encourage alternative modes of transportation through the Project's TDM program, as described in Section 3.8.2.3.

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¹ IPCC (Intergovernmental Panel on Climate Change), 2007. Climate Change 2007: The Physical Science Basis. Contribution of Working Group I to the Fourth Assessment Report of the Intergovernmental Panel on Climate Change [Solomon, S., D. Qin, M. Manning, Z. Chen, M. Marquis, K. B. Avery, M. Tignor, and H. L. Miller (eds.)]. Cambridge University Press, Cambridge, UK, and New York, 996 pp.

5.3.3 Rain Events

As a result of climate change, the Northeast is expected to experience more frequent and intense storms. To mitigate this, the Proponent will take measures to minimize stormwater runoff and protect the Project's mechanical equipment. The Project will be designed to reduce the existing peak rates and volumes of stormwater runoff from the site, and promote runoff recharge to the greatest extent practicable. The Project is investigating if a closed drainage system is possible.

5.3.4 Drought Conditions

Although more intense rain storms are predicted, extended periods of drought are also predicted due to climate change. Under the high emissions scenario, the occurrence of droughts lasting one to three months could go up by as much as 75% over existing conditions by the end of the century. To minimize the Project's susceptibility to drought conditions, the landscape design is anticipated to incorporate native and adaptive plant materials and a high efficiency irrigation system will be installed. Aeration fixtures and appliances will be chosen for water conservation qualities, conserving potable water supplies.

5.4 Renewable Energy

The Proponent will evaluate the potential for a roof-mounted solar photovoltaic (PV) system, and the availability of grants and renewables funding. Due to the spacing of the windows on the Church Building, space for a solar PV system is limited. The amount of space suitable for the Project is limited due to the mechanical needs and the proposed roof deck amenity space for the Frankfort Street building. Approximately 477 sf of rooftop area could potentially be used for solar PV panels, after taking into account the space available for solar PV panels, as well as space necessary around the panels, between panels, etc. Assuming 12 watts per square foot, this allows for an approximately 6 kW array. In the locations proposed, an installation of this solar array equals an annual generation of approximately 7.8 MW hours. The feasibility of installing a solar PV system will be further evaluated and determined at the time of construction.



LEED v4 for BD+C: New Construction and Major Renovation

Project Checklist

Gove Street + Frankfort Street Housing 4/2/2018

Green Power and Carbon Offsets

Credit 7

			4/2/201	8	
Y	?	N	Credit 1	Integrative Process	1
4	9	3	Locati	on and Transportation Possible Points:	16
-	-	-	Credit 1	LEED for Neighborhood Development Location	16
		1	Credit 2	Sensitive Land Protection	1
	2		Credit 3	High Priority Site	2
2	2	1	Credit 4	Surrounding Density and Diverse Uses	5
2	3		Credit 5	Access to Quality Transit	5
	1		Credit 6	Bicycle Facilities	1
	1		Credit 7	Reduced Parking Footprint	1
		1	Credit 8	Green Vehicles	1
4	5	1	Sustai	nable Sites Possible Points:	10
Υ			Prereq 1	Construction Activity Pollution Prevention	Required
	1		Credit 1	Site Assessment	1
	2		Credit 2	Site DevelopmentProtect or Restore Habitat	2
	1		Credit 3	Open Space	1
2	1		Credit 4	Rainwater Management	3
2			Credit 5	Heat Island Reduction	2
		1	Credit 6	Light Pollution Reduction	1
3	4	4	Water	Efficiency Possible Points:	11
3 Y	4	4	Water Prereq 1	Efficiency Possible Points: Outdoor Water Use Reduction	11 Required
	4	4			
Υ	4	4	Prereq 1	Outdoor Water Use Reduction	Required
Y	4	4	Prereq 1 Prereq 2	Outdoor Water Use Reduction Indoor Water Use Reduction	Required Required
Y Y Y		4	Prereq 1 Prereq 2 Prereq 3	Outdoor Water Use Reduction Indoor Water Use Reduction Building-Level Water Metering Outdoor Water Use Reduction Indoor Water Use Reduction	Required Required Required
Y Y Y 1	1		Prereq 1 Prereq 2 Prereq 3 Credit 1	Outdoor Water Use Reduction Indoor Water Use Reduction Building-Level Water Metering Outdoor Water Use Reduction	Required Required Required 2
Y Y Y 1	1	2	Prereq 1 Prereq 2 Prereq 3 Credit 1 Credit 2	Outdoor Water Use Reduction Indoor Water Use Reduction Building-Level Water Metering Outdoor Water Use Reduction Indoor Water Use Reduction	Required Required Required 2 6
Y Y Y 1 2	1 2	2 2	Prereq 1 Prereq 2 Prereq 3 Credit 1 Credit 2 Credit 3 Credit 4	Outdoor Water Use Reduction Indoor Water Use Reduction Building-Level Water Metering Outdoor Water Use Reduction Indoor Water Use Reduction Cooling Tower Water Use Water Metering	Required Required Required 2 6 2
Y Y Y 1 2	1 2	2 2	Prereq 1 Prereq 2 Prereq 3 Credit 1 Credit 2 Credit 3 Credit 4	Outdoor Water Use Reduction Indoor Water Use Reduction Building-Level Water Metering Outdoor Water Use Reduction Indoor Water Use Reduction Cooling Tower Water Use Water Metering y and Atmosphere Possible Points:	Required Required Required 2 6 2 1
Y Y Y 1 2	1 2	2 2	Prereq 1 Prereq 2 Prereq 3 Credit 1 Credit 2 Credit 3 Credit 4	Outdoor Water Use Reduction Indoor Water Use Reduction Building-Level Water Metering Outdoor Water Use Reduction Indoor Water Use Reduction Cooling Tower Water Use Water Metering y and Atmosphere Possible Points: Fundamental Commissioning and Verification	Required Required 2 6 2 1 Required
Y Y Y 1 2	1 2	2 2	Prereq 1 Prereq 2 Prereq 3 Credit 1 Credit 2 Credit 3 Credit 4	Outdoor Water Use Reduction Indoor Water Use Reduction Building-Level Water Metering Outdoor Water Use Reduction Indoor Water Use Reduction Cooling Tower Water Use Water Metering y and Atmosphere Possible Points: Fundamental Commissioning and Verification Minimum Energy Performance	Required Required 2 6 2 1 Required Required
Y Y Y 1 2 12 Y Y	1 2	2 2	Prereq 1 Prereq 2 Prereq 3 Credit 1 Credit 2 Credit 3 Credit 4 Energy Prereq 1 Prereq 2 Prereq 3	Outdoor Water Use Reduction Indoor Water Use Reduction Building-Level Water Metering Outdoor Water Use Reduction Indoor Water Use Reduction Cooling Tower Water Use Water Metering y and Atmosphere Fundamental Commissioning and Verification Minimum Energy Performance Building-Level Energy Metering	Required Required 2 6 2 1 33 Required Required Required Required
Y Y Y 1 2 Y Y Y	1 2 1	2 2	Prereq 1 Prereq 2 Prereq 3 Credit 1 Credit 2 Credit 3 Credit 4 Energy Prereq 1 Prereq 2 Prereq 3 Prereq 4	Outdoor Water Use Reduction Indoor Water Use Reduction Building-Level Water Metering Outdoor Water Use Reduction Indoor Water Use Reduction Cooling Tower Water Use Water Metering y and Atmosphere Possible Points: Fundamental Commissioning and Verification Minimum Energy Performance Building-Level Energy Metering Fundamental Refrigerant Management	Required Required 2 6 2 1 Required Required Required Required Required Required
Y Y Y 1 2 12 Y Y Y Y 3	1 2 1	12	Prereq 1 Prereq 2 Prereq 3 Credit 1 Credit 2 Credit 3 Credit 4 Energy Prereq 1 Prereq 2 Prereq 3 Prereq 4 Credit 1	Outdoor Water Use Reduction Indoor Water Use Reduction Building-Level Water Metering Outdoor Water Use Reduction Indoor Water Use Reduction Cooling Tower Water Use Water Metering y and Atmosphere Possible Points: Fundamental Commissioning and Verification Minimum Energy Performance Building-Level Energy Metering Fundamental Refrigerant Management Enhanced Commissioning	Required Required 2 6 2 1 33 Required Required Required Required Required Required 6
Y Y Y 1 2 Y Y Y Y 3 8	1 2 1	2 2	Prereq 1 Prereq 2 Prereq 3 Credit 1 Credit 2 Credit 3 Credit 4 Energy Prereq 1 Prereq 2 Prereq 3 Prereq 4 Credit 1 Credit 2	Outdoor Water Use Reduction Indoor Water Use Reduction Building-Level Water Metering Outdoor Water Use Reduction Indoor Water Use Reduction Cooling Tower Water Use Water Metering y and Atmosphere Possible Points: Fundamental Commissioning and Verification Minimum Energy Performance Building-Level Energy Metering Fundamental Refrigerant Management Enhanced Commissioning Optimize Energy Performance	Required Required 2 6 2 1 33 Required
Y Y Y 1 2 12 Y Y Y Y 3	1 2 1	12	Prereq 1 Prereq 2 Prereq 3 Credit 1 Credit 2 Credit 3 Credit 4 Energy Prereq 1 Prereq 2 Prereq 3 Prereq 4 Credit 1 Credit 2 Credit 3	Outdoor Water Use Reduction Indoor Water Use Reduction Building-Level Water Metering Outdoor Water Use Reduction Indoor Water Use Reduction Cooling Tower Water Use Water Metering y and Atmosphere Possible Points: Fundamental Commissioning and Verification Minimum Energy Performance Building-Level Energy Metering Fundamental Refrigerant Management Enhanced Commissioning Optimize Energy Performance Advanced Energy Metering	Required Required Required 2 6 2 1 33 Required Required Required Required Required 18 1
Y Y Y 1 2 Y Y Y Y 3 8	1 2 1	12	Prereq 1 Prereq 2 Prereq 3 Credit 1 Credit 2 Credit 3 Credit 4 Energy Prereq 1 Prereq 2 Prereq 3 Prereq 4 Credit 1 Credit 2 Credit 3 Credit 4	Outdoor Water Use Reduction Indoor Water Use Reduction Building-Level Water Metering Outdoor Water Use Reduction Indoor Water Use Reduction Cooling Tower Water Use Water Metering y and Atmosphere Possible Points: Fundamental Commissioning and Verification Minimum Energy Performance Building-Level Energy Metering Fundamental Refrigerant Management Enhanced Commissioning Optimize Energy Performance Advanced Energy Metering Demand Response	Required Required Required 2 6 2 1 33 Required Required Required Required Required 1 1 2
Y Y Y 1 2 Y Y Y Y 3 8	1 2 1	12	Prereq 1 Prereq 2 Prereq 3 Credit 1 Credit 2 Credit 3 Credit 4 Energy Prereq 1 Prereq 2 Prereq 3 Prereq 4 Credit 1 Credit 2 Credit 3	Outdoor Water Use Reduction Indoor Water Use Reduction Building-Level Water Metering Outdoor Water Use Reduction Indoor Water Use Reduction Cooling Tower Water Use Water Metering y and Atmosphere Possible Points: Fundamental Commissioning and Verification Minimum Energy Performance Building-Level Energy Metering Fundamental Refrigerant Management Enhanced Commissioning Optimize Energy Performance Advanced Energy Metering	Required Required Required 2 6 2 1 33 Required Required Required Required Required 18 1

2

5	8	0	Materi	ials and Resources	Possible Points:	13
Υ			Prereq 1	Storage and Collection of Recyclables		Required
Υ			Prereq 2	Construction and Demolition Waste Management Planning		Required
	5		Credit 1	Building Life-Cycle Impact Reduction		5
1	1		Credit 2	Building Product Disclosure and Optimization - Environmental Product Declar	arations	2
1	1		Credit 3	Building Product Disclosure and Optimization - Sourcing of Raw Materials		2
1	1		Credit 4	Building Product Disclosure and Optimization - Material Ingredients		2
2			Credit 5	Construction and Demolition Waste Management		2
9	2	4	Indoor	Environmental Quality	Possible Points:	16
Υ			Prereq 1	Minimum Indoor Air Quality Performance		Required
Υ			Prereq 2	Environmental Tobacco Smoke Control		Required
1	1		Credit 1	Enhanced Indoor Air Quality Strategies		2
3			Credit 2	Low-Emitting Materials		3
1			Credit 3	Construction Indoor Air Quality Management Plan		1
		1	Credit 4	Indoor Air Quality Assessment		2
1			Credit 5	Thermal Comfort		1
1	1		Credit 6	Interior Lighting		2
		3	Credit 7	Daylight		3
1			Credit 8	Quality Views		1
1			Credit 9	Acoustic Performance		1
3	2	1	Innova	ation	* Possible Points:	6
1			Credit 1.1	Innovation - TBD (possible - MRc2 exemplary performance)		5
1			Credit 1.2	Innovation - TBD (possible - MRc4 exemplary performance)		
	1		Credit 1.3	Innovation - TBD (possible - SSc5 exemplary performance)		
	1		Credit 1.4	Innovation - TBD (possible IEQc1 exemplary performance)		
		1	Credit 1.5	Innovation - TBD		
1			Credit 2	LEED Accredited Professional		1
0	4	0	Region	nal Priority	** Possible Points:	4
	1		Credit 1	Regional Priority: High Priority Site (threshold 2 points)		1
	1		Credit 2	Regional Priority: Rainwater Management (threshold 2 points)		1
	1		Credit 3	Regional Priority: Indoor Water Use Reduction (threshold 4 points)		1
	1		Credit 4	Regional Priority: Renewable Energy Production (threshold 2 points)		1
			OR	Regional Priority: Optimize Energy Performance (threshold 8 points)		
40	44	25	Total		Possible Points:	110

Chapter 6

Urban Design

6.0 URBAN DESIGN

The approximately 1.13-acre Project site is located at the intersection of Frankfort and Gove streets in East Boston and is generally bound by Frankfort Street to the west, Lubec Street to the east, and residential and institutional properties to the north and south. The Project site currently includes Our Lady of Mt. Carmel and an attached Rectory Building, an existing Convent Building, and a vacant lot. Most of the surrounding uses are residential, commercial and institutional, with public open space, transportation and amenities such as the MBTA Maverick Blue Line stop and Maverick Square, East Boston Memorial Park, East Boston Greenway, and multiple major MBTA bus routes. The site is also within two miles of Boston Logan Airport.

The Project will introduce additional housing and new, sophisticated design, and will showcase the revitalized Church Building as a renewed cornerstone of the neighborhood. The principles governing this development are described below.

Connecting to the Surrounding Neighborhood

The design of the Project will complement and contemporize the neighborhood through the combination of warm materials such as brick and terracotta, and modern materials such as metal siding and balcony railings. The Project site will be further amplified with improved landscaping and paving materials.

The structures along Frankfort and Gove streets are approximately four-stories with residential and commercial uses. The Church Building is approximately four-stories, 55 feet tall at the ridge of its front roof.

The existing residences located between Frankfort and Maverick streets are generally midrise and three-stories (between 38 and 48 feet to the cornice line). These buildings consist of red brick and details such as lintels, cornices and articulated entrances. See Figure 6-1 for the existing materials context on Frankfort Street. For consistency, the proposed facades of the building along Frankfort Street will be four-stories and will have brick facades and details such as cornices and metal balcony railings. At the intersection of Frankfort and Gove streets, the building is five stories and 55 feet to the parapet, which is in kind with the larger buildings along the Gove Street corridor. The sixth story is set back 25 feet from the Gove Street property line, and 16 feet back from the Frankfort Street property line. The portion of the Frankfort Street Building closest to this intersection will be clad in terracotta and metal siding to provide design interest and material depth. The design of the Project will respect the materials and heights of the existing neighborhood structures, and provide modern materials and details for the Frankfort Street Building. See Figures 6-2 through 6-11 for perspectives, elevations, materials and the interior design precedent for the Frankfort Street Building.



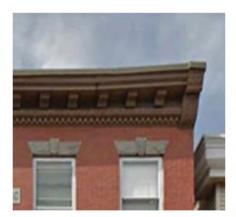


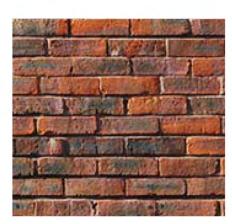
























GOVE ST SCHEMATIC ELEVATION

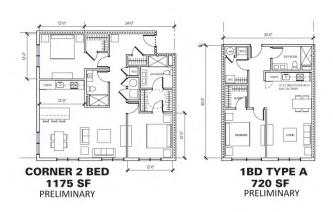


FRANKFORT ST - REAR - SCHEMATIC ELEVATION











FRANKFORT STREET BUILDING 98 TOTAL UNITS

•	ONE BEDROOM	53 UNITS	AVERAGE 700SF
•	ONE BEDROOM PLUS DEN	16 UNITS	AVERAGE 800SF
•	TWO BEDROOM	17 UNITS	AVERAGE 1050SF
•	TWO BEDROOM PLUS DEN	12 UNITS	AVERAGE 1100SF



Revitalizing a Neighborhood Cornerstone

Our Lady of Mt. Carmel Catholic Church was officially closed in 2004. The proposed Project design will respect the structure and will retain the existing brick and limestone façade. The entrance onto Gove Street will be revitalized to create a prominent focal point of the neighborhood. The Church Building will be creatively transformed into a residential use). It will be cleaned and repaired, especially where religious iconography was removed. Stairs will be removed at the front entry and joined with a larger, glazed entrance to open up the lobby and activate the ground floor. On the east and west facades, window openings will be extended to the ground to the ground to increase transparency. See Figures 6-12 through 6-17 include perspectives, elevations, materials and interior design precedent for the proposed Church Building renovation.

Activate public realm and open space

The Project will greatly improve the pedestrian experience of the public realm on every side of the site. The landscaped open space surrounding the Church Building gives character and a pleasant destination to the streetscape. A green space includes benches, raised plantings, trees and landscaped parking area will be located to the east of the Church Building. This is a significant open space on the site and will provide a quiet and serene retreat set back from the street. On the west side of the Church Building, there will be a reflection garden, which will be a place to reflect on the history of the Church Building, perhaps with informational graphics. Along Frankfort Street, the streetscape will be improved with street trees, planting beds and individually articulated entrance stoops to boost street life and activity. The corner will be landscaped as it fronts a mostly transparent ground floor housing lobby and other amenity spaces. See Figures 6-18 through 6-24 for renderings, materials and proposed landscaping details.

Transit-oriented development

The site's proximity to the MBTA's Maverick Station will easily facilitate transit throughout the Greater Boston Area for the Project's future residents. In addition to the site's future residents, the development is within two miles of Boston Logan International Airport and will allow travel beyond Boston for residents and visitors.



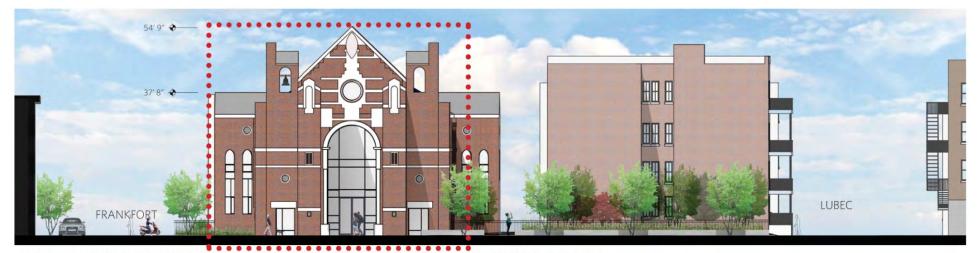


CHURCH BUILDING - WEST



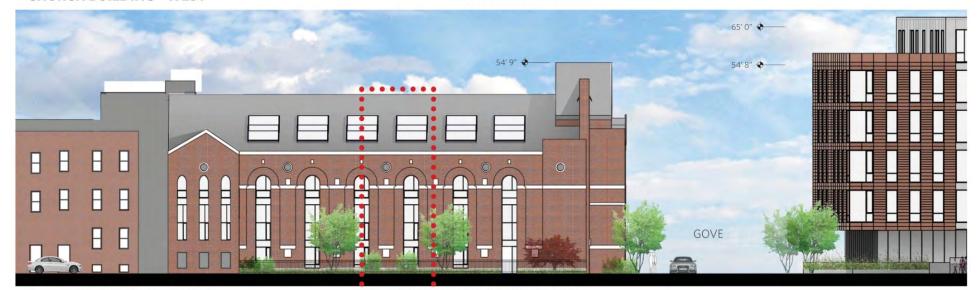
CHURCH BUILDING - NORTH





CHURCH BUILDING - WEST

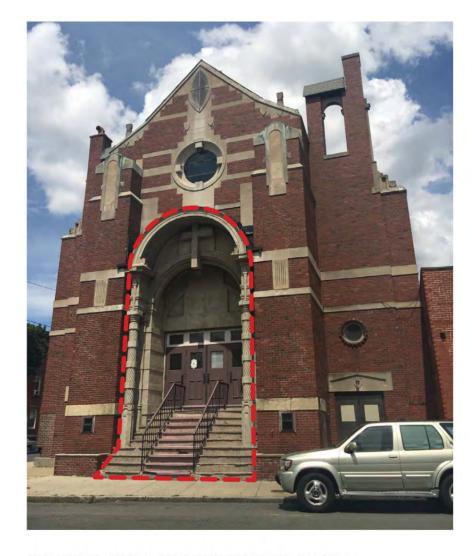
SEE ENLARGED ELEVATION



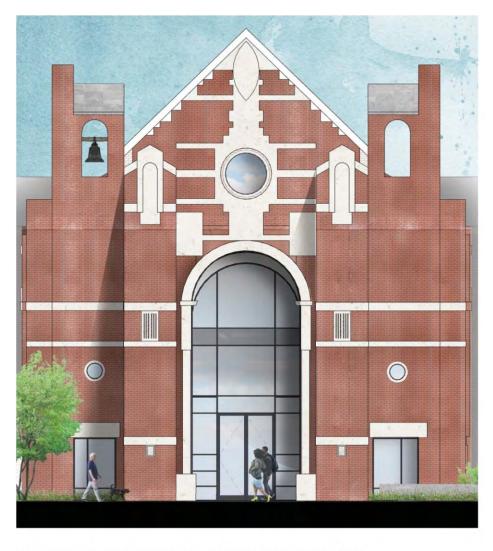
CHURCH BUILDING - NORTH

• • • • • • • • SEE ENLARGED ELEVATION

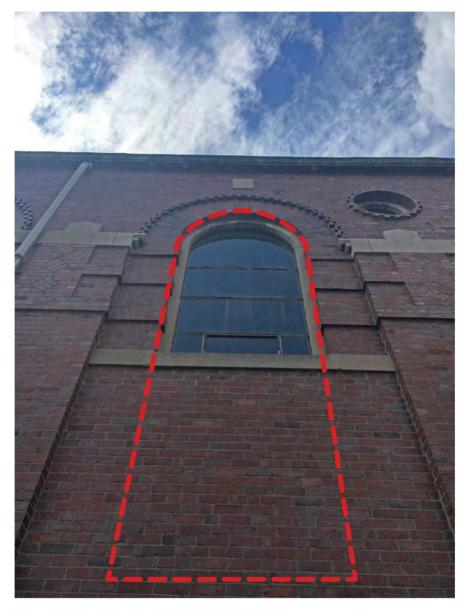




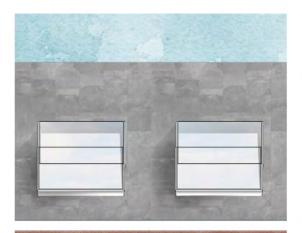
EXISTING STAIR AT CHURCH ENTRANCE



PROPOSED ENTRANCE TO CHURCH BUILDING



EXISTING WINDOW CONDITION





PROPOSED WINDOW CONDITION









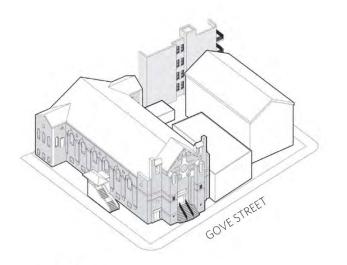


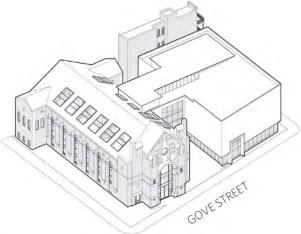
GOVE STREET CHURCH BUILDING 14 TOTAL UNITS

- ONE BEDROOM LOFT STYLE
- TWO BEDROOM LOFT STYLE

10 UNITS 4 UNITS (AVERAGE 800 SF)

TS (AVERAGE 1050 SF)







EXISTING

PREVIOUS WITH ADDITION

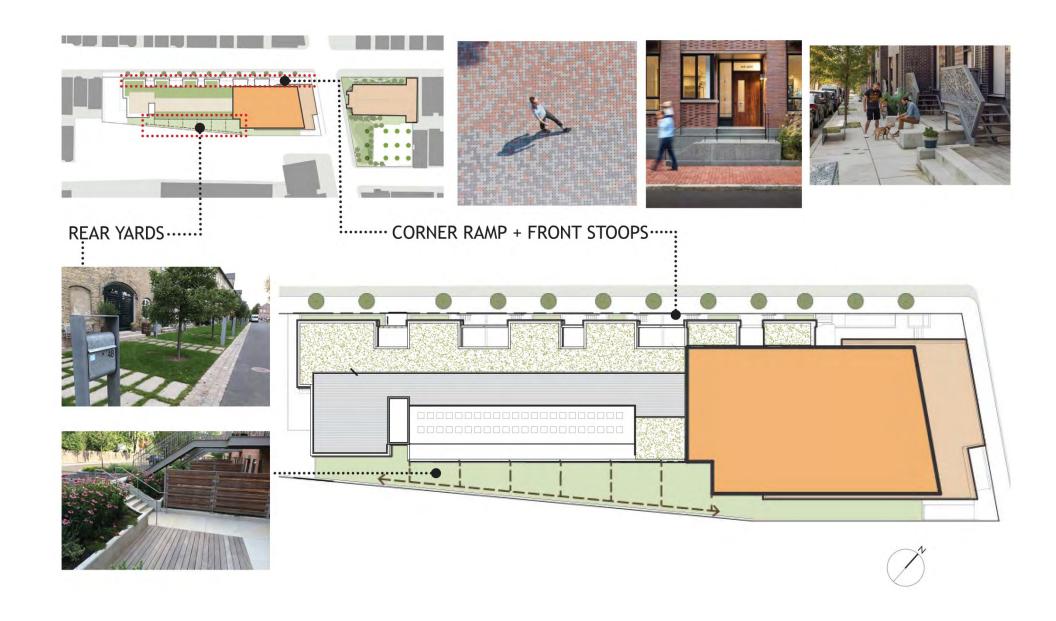
CURRENT WITH GREEN SPACE

CURRENT WITH GREEN SPACE

- REMOVE EXISTING FLOOR AND GOVE STREET ENTRY STAIR
- CREATE MAIN ENTRY OFF OF GOVE STREET
- RESTRUCTURE TO ACCOMMODATE 3 NEW RESIDENTIAL LEVELS
- ENLARGE WINDOW AREAS FOR SUFFICIENT NATURAL LIGHT
- INSTALL SKYLIGHTS FOR THIRD FLOOR UNITS
- RESPECT AND INTEGRATE THE EXISTING CHURCH CHARACTER
- CREATE REFLECTION SPACE AT CORNER OF CHURCH
- NEW GREEN SPACE WITH 13 INTEGRATED PARKING SPACES

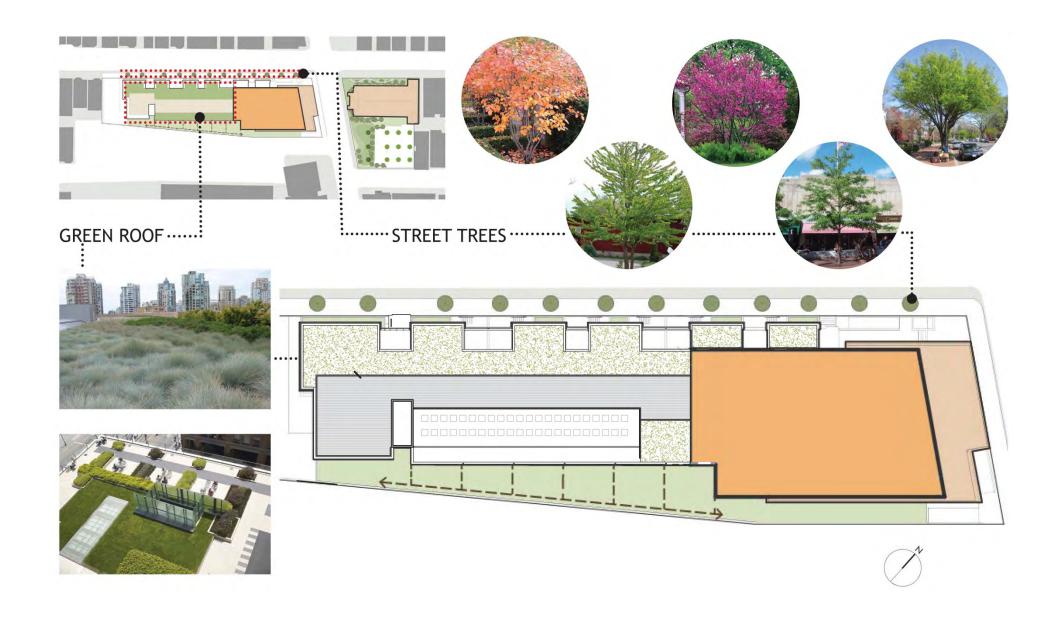
Frankfort Gove Street Housing

Boston, Massachusetts



Frankfort Gove Street Housing

Boston, Massachusetts



Frankfort Gove Street Housing

Boston, Massachusetts





Pennisetum Echinacea Perovskia

CHURCH BUILDING - SITE PLAN





Historic and Archaeological Resources

7.0 HISTORIC AND ARCHAEOLOGICAL RESOURCES

7.1 Introduction

This section describes the historic and archaeological resources located on the Project site and within the Project's vicinity. Reviews of the State and National Registers of Historic Places, as well as the Massachusetts Historical Commission's (MHC) Inventory of Historic and Archaeological Assets of the Commonwealth (the Inventory), were undertaken to identify historic and archaeological resources.

7.2 Historic Resources

7.2.1 Historic Resources on the Project Site

The approximately 1.13-acre Project site is located at the intersection of Frankfort and Gove streets in the neighborhood of East Boston. The site is located in the 115-146 Gove Street survey area (BOS.H) and the Our Lady of Mount Carmel Catholic Church Complex (BOS.W), listed in the Inventory. The site encompasses four parcels located at the intersection of Gove Street and Frankfort Street and is generally bound by Frankford Street to the west, Lubec Street to the east and existing residential and institutional properties to the north and south. The four parcels include the Our Lady of Mount Carmel Roman Catholic Church (BOS.26), the Our Lady of Mount Carmel Roman Catholic Rectory (BOS.15268) and the Our Lady of Mount Carmel Roman Catholic Convent (BOS.27).

Established in 1905, Our Lady of Mount Carmel Roman Catholic Church (BOS.26) is located in the northeast corner of the intersection of Frankfort Street and Gove Street. Constructed in two phases the lower church was designed by A. N. Rogers was built in 1907 and the Arts and Crafts-inspired upper brick church was designed by Matthew Sullivan in 1920. The church was founded as an Italian-speaking mission of St. Lazarus Church located at 61 Ashley Street in East Boston. In 1913, Rev. James Merighi was appointed the first resident pastor, serving until 1917. Our Lady of Mt. Carmel Catholic Church was officially closed in 2004. The three-bay wide façade of the brick and cast stone church features a projecting gabled entrance set within a segmental arched surround. The tower is flanked by brick bell towers and capped by a small square cupola. The six-bay wide side elevations feature round arched windows with brick round arch lintels.

The Our Lady of Mount Carmel Roman Catholic Rectory (BOS.15268) is located east of the Church on the northern side of Gove Street. Designed by Boston architect Thomas F. McDonough in 1955, the three-story brick building features one-over-one double hung replacement sash windows with cast stone sill and brick lintels with cast stone keystones. A central entrance features a one-bay projecting portico supported by Doric columns and capped by an iron balustrade. A one-story five-bay wide brick connector building,

containing the parish hall adjoins the rectory and the church. The two eastern bays of the north elevation have been bricked in, while stone lintels survive over the two western windows. The central entrance features a stone rounded-arched entry.

Constructed in 1949-1958 the Our Lady of Mount Carmel Roman Catholic Convent (BOS.27) is located on the southern side of Gove Street within the southeast corner of the intersection of Frankfort and Gove. The three-story, eight- bay by four-bay brick building is capped by a low-pitched hipped roof. The building's main entrances, located on the west and north elevations are enhanced by concrete surrounds and accessed by a set of brick and concrete steps. The regularly spaced fenestration features six-over-six double hung wood sash with brick lintel and cast stone sills. Four first floor windows of the north elevation feature keyed round-arched surrounds and paneled aprons.

7.2.2 Historic Resources in the Project Vicinity

In addition to the existing buildings located on the Project site, there are two resources listed in the State and National Registers within the Project vicinity. Table 7-1 identifies these resources within one-quarter mile of the Project site and corresponds to resources depicted in Figure 7-1.

Table 7-1 Historic Resources in the Vicinity of the Project Site

Map No	Name	Address	Designation
1	1 1000-10)
I	Theodore Lyman School	30 Gove Street	National Register
			Individual Property
2	Street Clock	9 Chelsea Street	Local Landmark

7.3 Archaeological Resources within the Project Site

The Project site consists of a previously developed urban parcel. As confirmed on September 24, 2018 there are no known archaeological resources listed in the State and National Registers of Historic Places or included in the Inventory within the Project Site.

7.4 Impacts to Historic Resources

7.4.1 Urban Design

The Project includes redevelopment of the site for approximately 120,430 sf residential space including the renovation of Our Lade of Mt. Carmel Catholic Church and the creation of two new building containing up to 112 residential units, 84 parking spaces, open space and public realm improvements. Architecturally, the Project will blend with the residential neighborhood and revitalize the Our Lady of Mt. Carmel Catholic Church.







The Our Lady of Mt. Carmel Catholic Church (Church Building) will be renovated for 14 residential units. The Gove Street entrance will be revitalized with the lowering of the existing entrance to grade and the installation of a two-story glass entry within the existing arched opening. The proposed Project design will respect the architectural integrity of the building. The existing rectory and church addition will be removed for parking and green space at the corner of Gove Street and Lubec Street.

The existing convent will be removed to accommodate the construction of the new Frankfort and Gove Street Building, a residential building containing 45 apartment units and 42 row house units. The portion of the building to contain apartment units will be six-stories and 65 feet in height and will be clad in terracotta and metal siding. The proposed rowhouses will be four-stories and 46 feet tall with brick facades, consistent with the existing heights and materials of the existing residences located between Frankfort and Maverick Streets which are generally two- and three -story red brick buildings.

The Project will redevelop and replace an underutilized site and provide market-rate and affordable units with a variety of unit sizes and styles. The design of the Project will complement the neighborhood with the use of brick, terracotta and modern materials such as metal siding.

7.4.2 Shadow Impacts to Historic Resources

A shadow impact analysis was conducted to investigate shadow impacts from the Project during three time periods (9:00 a.m., 12:00 noon, and 3:00 p.m.) during the vernal equinox (March 21), summer solstice (June 21), autumnal equinox (September 21), and the winter solstice (December 21). In addition, shadow studies were conducted for the 6:00 p.m. period during the summer solstice and autumnal equinox. The shadow analysis presents net new shadow from the Project, as well as the existing shadow, and illustrates the incremental impact of the Project. The analysis shows that the Project's impact will generally be limited to the immediately surrounding streets and sidewalks and the Project site itself. See shadow figures 4.2-1 – 4.2-14.

New shadows will largely be directed to the west and north toward Frankfort Street, the north and northeast toward Gove Street. Among the periods studied, the only potential new shadow impacts to the Our Lady of Mt. Carmel Catholic Church (Church Building) are on December 21st, 3:00 pm, September 21st at 6:00 pm, March 21st at 3:00 pm and 6:00 pm. At these times, the impacts will be mainly limited to the southern end of the building. The potential shadows will have no impact to the National Register and Local Landmark resources in the Project's vicinity.

7.5 Consistency with Other Historic Reviews

7.5.1 Boston Landmarks Commission Article 80 Review

The submission of this PNF initiates review of the Project by the BLC under the City's Article 80 Review process.

7.5.2 Boston Landmarks Commission Article 85 Review

The proposed demolition of the buildings on the Project site will be subject to review by the Boston Landmarks Commission under Article 85 of the Boston Zoning Code. An Article 85 Application will be submitted to the BLC for the proposed demolition of the Rectory (1955) and the Convent (1949-1958) buildings.

7.5.3 Massachusetts Historical Commission

The MHC has review authority over projects utilizing or requiring state or federal funding, licensing, permitting and/or approvals that may have direct or indirect impacts to properties listed in the State or National Registers of Historic Places. If the Project requires a state or federal action, the MHC review process will be initiated through the filing of an MHC Project Notification Form as prescribed in MHC's governing regulations.

Chapter 8

Infrastructure

8.1 Overview of Existing Utility Services

The Project site is located at the intersection of Frankfort and Gove Streets in East Boston. The site currently consists of four parcels of land (former Our Lady of Mount Carmel complex) including the Church Building, the Rectory Building, the Convent Building and a surface parking lot totaling 1.13 acres of land area. The Project proposes to raze the existing Convent and Rectory buildings and construct a new building with a subsurface parking garage. The Church Building will be rehabilitated as part of the Project.

As shown on Figures 8-1 and 8-2 there are existing utilities in each street. The existing infrastructure surrounding the site appears sufficient to handle the increase in service needs from the Proposed Project. The following sections describe the existing sewer, water, drainage systems and energy services surrounding the Project and provide an explanation for how these systems will service the Project.

8.2 Water Supply

8.2.1 Existing Water Infrastructure

BWSC owns, operates, and maintains the water distribution systems in the vicinity of the Project Site. According to available record plans from BWSC, there is a twelve inch water main in Frankfort Street and an eight inch main in Gove Street. The existing water distribution in the vicinity of the Project site is shown on Figure 8-1.

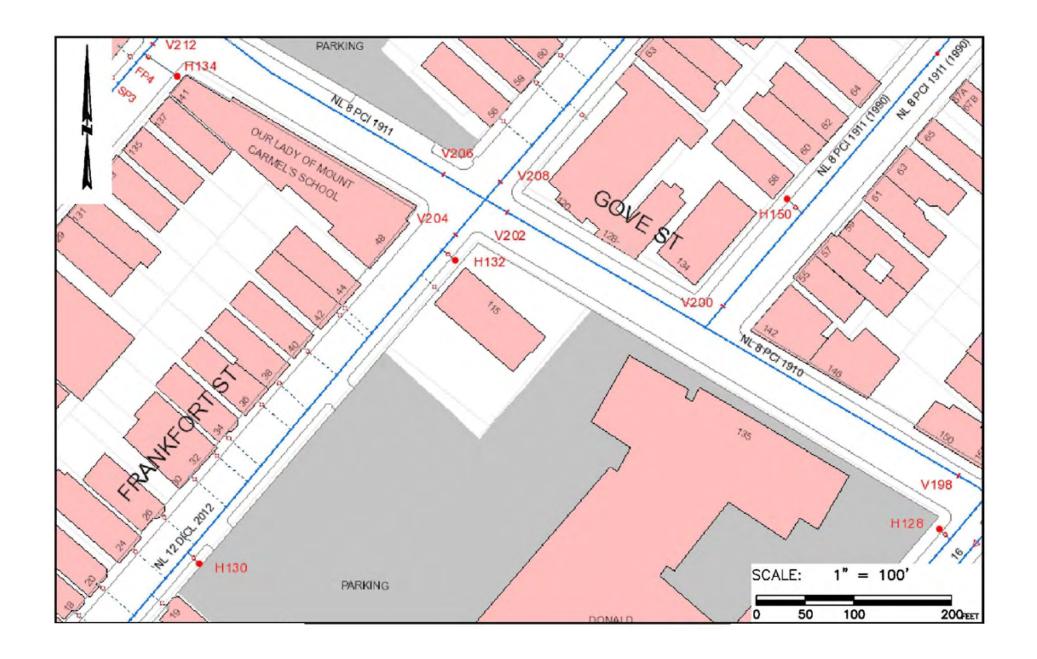
8.2.2 Water Consumption

The estimated proposed water demand for the Project is based on the estimated sanitary sewer flow (see Table 8-1), with a factor of 1.1 applied to account for consumption and other loses. Based on this formula, the Project's estimated additional peak water demand for domestic uses is 17,666-gallons per day. The proposed water demand calculation will be refined as the building program is further refined in future Article 80 filings with the BPDA and coordinated with BWSC. The domestic water will be supplied by the BWSC water system.

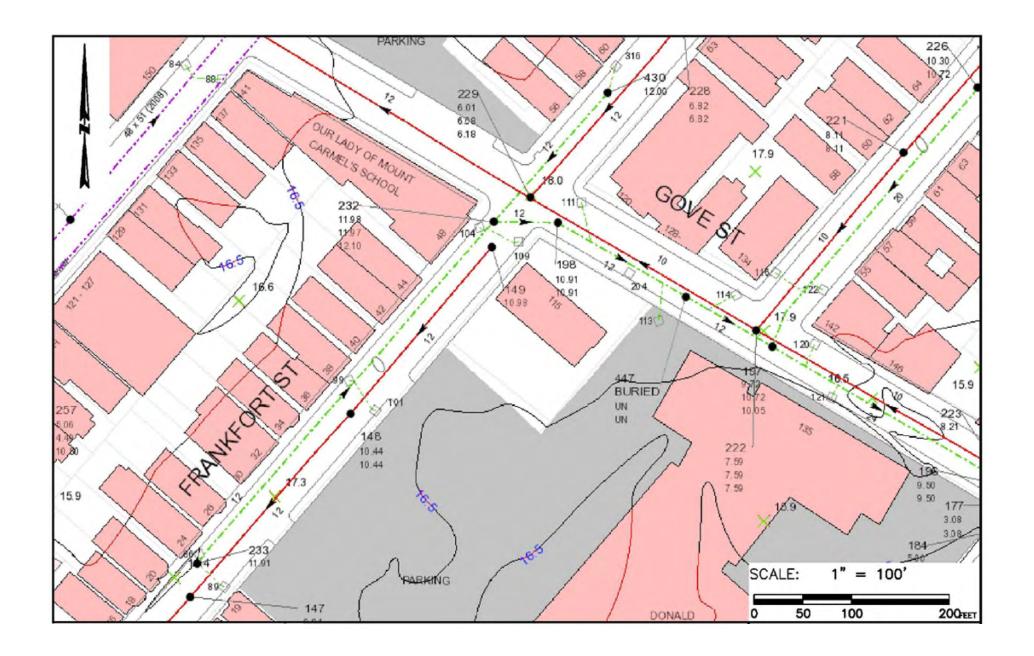
There are no anticipated water capacity concerns in the vicinity of the Project site.

8.2.3 Proposed Water Connections

The new building will require a new connection to the water main. As the design progresses, the Project will coordinate with BWSC for new water connections to the mains in accordance with all BWSC and ISD requirements. The existing Church Building to remain proposes to reuse the existing water service connections.









Further investigation is required to determine if the capacity and condition of the existing water service connections allow for reuse. All services will be in accordance with BWSC and ISD requirements.

8.2.4 Water Supply Conservation and Mitigation

The Project will be LEED certifiable in accordance with the BRA's Article 37 Green Building program. As such, various water conservation measures such as low-flow toilets and urinals, restricted flow faucets, and sensor operated sinks, toilets, and urinals may be incorporated in order to meet the LEED water conservation requirements. Specific water conservation measures to be included in the Project will be more fully described as the building designs develop.

8.2 Wastewater

8.2.1 Existing Wastewater

BWSC owns, operates, and maintains the sanitary and combined sewer mains on and in the vicinity of the Project Site. Per available record information from BWSC, there are sewer mains in the streets surrounding the Project Site. The existing sewer system in the vicinity of the Project site is shown on Figure 8-2. The sewers ultimately flow to the Massachusetts Water Resources Authority's (MWRA's) Deer Island Wastewater Treatment Plant, where it is treated and discharged to Massachusetts Bay.

8.2.2 Wastewater Generation

The Massachusetts Department of Environmental Protection (MassDEP) establishes sewer generation rates for various types of establishments in a section of the State Environmental Code Title V (Title V), 310 CMR 15.203. Based on an estimate of the Project's building program, Table 8-1 gives the estimated proposed sanitary sewer flows expected to be generated by the Project. Based on these Title V sewer generation rates, the Project is expected to produce approximately 16,060 gallons/day of additional sewer flow. The proposed sewer generation calculation will be refined as the building program is further refined in future Article 80 filings with the BPDA and coordinated with BWSC.

Table 8-1 Net New Sewer Generation

Frankfort Street			
Unit Type	Program	Sewer Generation Rate	Sewer Flow (gpd)
1 Bedroom	68 Units	110 gallons/day/room.	7,480
2 Bedroom	30 Units	110 gallons/day/room.	6,600
Total New Sewer Ge	neration at Frankfort Street		14,080
Gove Street			
Unit Type	Program	Sewer Generation Rate	Sewer Flow (gpd)
1 Bedroom	10 Units	110 gallons/day/room	1,100
2 Bedroom	4 Units	110 gallons/day/room	880
Total New Sewer Ge	neration at Gove Street		1,980
Grand Total New Sev	wer Generation for Frankfor	t and Gove Streets	16,060

The Project's engineer will coordinate final, proposed sewer flows and available capacity with BWSC during Project design to ensure the Project needs are met without disruption of service to the surrounding area.

8.2.3 Proposed Connection

The new building will require a new sanitary sewer service connection to the BWSC sewer main. The existing Church Building to remain proposes to reuse the existing sanitary sewer service connection. Further investigation is required to determine if the capacity and condition of the existing sanitary sewer connection allow for reuse. All sewer service connections will be designed and installed in accordance with BWSC requirements.

8.2.4 Sewer System Mitigation

As previously stated, the Project will be LEED certifiable in accordance with the BPDA's Article 37 Green Building program. As such, various measures for water conservation and wastewater reduction such as low-flow toilets and urinals, restricted flow faucets, and sensor operated sinks, toilets, and urinals may be incorporated to meet the LEED requirements. Specific water conservation and wastewater reduction measures to be included in the Project will be more fully described as the building designs develop.

8.4 Stormwater Management

8.4.1 Existing Conditions

The existing Project site consists of concrete sidewalks, the Church Building, Rectory Building, Convent Building with a grass yard and an asphalt parking lot. Runoff from the parking area and paved sidewalks likely sheet flows to catch basins located in Frankfort and Gove Streets that connect to the BWSC drainage system. Runoff from the roof of the existing buildings appears to be connected directly to the BWSC drainage system. The Project is in the process of determining the exact location of the connections.

8.4.2 Proposed Conditions

The Project is in the process of determining the exact existing location of connections to the BWSC drainage system. The stormwater runoff from the roof of the Church Building will maintain any existing connections. The new building areas will be designed to collect the roof runoff and be directed to a stormwater infiltration system (if feasible) to promote groundwater infiltration and mitigate for any additional impervious site area. The proposed drainage system design will be refined during the design process.

The Project will disturb more than one acre of land; therefore, construction will require the submittal of a Notice of Intent (NOI) for coverage under the Construction General Permit (CGP) as part of the Environmental Protection Agency's (EPA's) National Pollutant Discharge Elimination System (NPDES). Appropriate erosion and sedimentation (E&S) controls will be installed to prevent sediment laden stormwater runoff from leaving the site and entering the BWSC drainage system during construction. E&S controls will be maintained as necessary until all disturbed areas have been stabilized through the placement of pavement and structures and will conform to the Water Quality section of the City of Boston Environment Department Guidelines for Construction.

8.4.3 Water Quality Impacts

Erosion and sediment control measures will be implemented during construction to minimize the transport of site soils to off-site areas. During construction, existing storm drain inlets will be protected with filter fabric, straw bales and/or crushed stone, to provide for sediment removal from runoff. These controls will be inspected and maintained throughout the construction phase until the areas of disturbance have been stabilized through the placement of pavement, structure, or vegetative cover.

Depending on how the site work is split up, if it is all under one project it will be over one-acre of disturbance, which will trigger the National Pollutant Discharge Elimination System (NPDES) Construction General Permit. The Erosion and Sediment Controls will also conform to the Water Quality section of the City of Boston Environment Department Guidelines for Construction.

8.4.4 MassDEP Stormwater Management Policy Standards

In March 1997, MassDEP adopted a Stormwater Management Policy to address non-point source pollution. In 1997, MassDEP published the Massachusetts Stormwater Handbook as guidance on the Stormwater Policy, which was revised in February 2008. The Policy prescribes specific stormwater management standards for development projects, including urban pollutant removal criteria for projects that may impact environmental resource areas. Compliance is achieved through the implementation of Best Management Practices (BMPs) in the stormwater management design. The Policy is administered locally pursuant to MGL Ch. 131, s. 40.

A brief explanation of each Policy Standard and the system compliance is provided below:

Standard #1: No new stormwater conveyances (e.g. outfalls) may discharge untreated stormwater directly to or cause erosion in wetlands or waters of the Commonwealth.

Compliance: The proposed design will comply with this standard. The design will incorporate the appropriate stormwater treatment, and no new untreated stormwater will be directly discharged to, nor will erosion be caused to wetlands or waters of the Commonwealth as a result of stormwater discharges related to the Project.

Standard #2: Stormwater management systems shall be designed so that post-development peak discharge rates do not exceed pre-development peak discharge rates.

Compliance: The proposed design will comply with this standard to the maximum extent practicable. The pre-development stormwater discharge rates will be met or decreased as a result of the improvements associated with the Project.

Standard #3: For New Construction, loss of annual recharge to groundwater shall be eliminated or minimized through the use of infiltration measures including environmentally sensitive site design, low impact development techniques, stormwater best management practices, and good operation and maintenance. At a minimum, the annual recharge from the post-development site shall approximate the annual recharge from pre-development conditions based on soil type. The standard is met when the stormwater management system is designed to infiltrate the required recharge volume as determined in accordance with the Massachusetts Stormwater Handbook.

Compliance: The Project will comply with this standard to the maximum extent practicable.

Standard #4: Stormwater management systems shall be designed to remove 80% of the average annual post-construction load of Total Suspended Solids (TSS). This standard is met when: (a) Suitable practices for source control and pollution prevention are identified in a long-term pollution prevention plan, and thereafter are implemented and maintained; (b) Structural stormwater best management practices are sized to capture the required water

quality volume determined in accordance with the Massachusetts Stormwater Handbook; and (c) Pretreatment is provided in accordance with the Massachusetts Stormwater Handbook.

Compliance: The proposed design will comply with this standard. Within the Project's limit of work, there will be building roof, paved sidewalk, landscaped areas, and roadway areas. Runoff from paved areas that would contribute unwanted sediments or pollutants to the existing storm drain system will be collected by deep sump, hooded catch basins and conveyed through water quality units before discharging.

Standard #5: For Land Uses with Higher Potential Pollutant Loads (LUHPPL), source control and pollution prevention shall be implemented in accordance with the Massachusetts Stormwater Handbook to eliminate or reduce the discharge of stormwater runoff from such land uses to the maximum extent practicable. If through source control and/or pollution prevention all land uses with higher potential pollutant loads cannot me completely protected from exposure to rain, snow, snow melt, and stormwater runoff, the Proponent shall use the specific structural stormwater BMPs determined by the Department to be suitable for such uses as provided in the Massachusetts Stormwater Handbook. Stormwater discharges from land uses with higher potential pollutant loads shall also comply with the requirements of the Massachusetts Clean Waters Act, M.G.L. c.21 §§ 26-53 and the regulations promulgated thereunder at 314 CMR 3.00, 314 CMR 4.00 and 314 CMR 5.00.

Compliance: The proposed design will comply with this standard. The Project is anticipated to be a LUHPPL (per the Policy, Volume I, page 1-6) due to the average daily trips to the site.

Standard #6: Stormwater discharges within Zone II or Interim Wellhead Protection Area of a public water supply, and stormwater discharges near or to any other critical area, require the use of the specific source control and pollution prevention measures and the specific structural stormwater best management practices determined by the Department to be suitable for managing discharges to such areas, as provided in the Massachusetts Stormwater Handbook. A discharge is near a critical area if there is a strong likelihood of significant impact occurring to said area, taking into account site-specific factors. Stormwater discharges to Outstanding Resource Waters shall be removed and set back from the receiving water or wetland and receive the highest and best practical method of treatment. A "storm water discharge" as defined in 314 CMR 3.04(2)(a) or (b) to an Outstanding Resource Water or Special Resource Water shall comply with 314 CMR 3.00 and 314 CMR 4.00. Stormwater discharges to a Zone I or Zone A are prohibited unless essential to the operation of a public water supply.

Compliance: The proposed design will comply with this Standard to the maximum extent practicable. The Project will not discharge untreated stormwater to a sensitive area.

Standard #7: A redevelopment project is required to meet the following Stormwater Management Standards only to the maximum extent practicable: Standard 2, Standard 3, and the pretreatment and structural best management practice requirements of Standards 4, 5, and 6. Existing stormwater discharges shall comply with Standard 1 only to the maximum extent possible. A redevelopment project shall also comply with all other requirements of the Stormwater Management Standards and improve existing conditions.

Compliance: The Project will comply with this standard. The Project will comply with the Stormwater Management Standards as applicable to the redevelopment.

Standard #8: Erosion and sediment controls must be implemented to prevent impacts during construction or land disturbance activities.

Compliance: The Project will comply with this standard. Sedimentation and erosion controls will be incorporated as part of the design of the Project and employed during construction.

Standard #9: A Long-Term Operation and Maintenance (O&M) Plan shall be developed and implemented to ensure that stormwater management systems function as designed.

Compliance: The Project will comply with this standard. An O&M Plan including long-term BMP operation requirements will be prepared for the Project and will assure proper maintenance and functioning of the stormwater management system.

Standard #10: All illicit discharges to the stormwater management system are prohibited.

Compliance: The Project will comply with this standard. No illicit connections will be proposed with the Project.

8.5 Anticipated Energy Needs

8.5.1 Telecommunications Systems

The Proponent will select private telecommunication companies to provide telephone, cable, and data services. There are several potential candidates capable of providing service. Upon selection of a provider or providers, the Proponent will coordinate service connection locations and obtain appropriate approvals.

8.5.2 Electricity and Natural Gas

There are existing natural gas mains in Gove Street and overhead power line in both Gove and Frankfort streets. The Proponent will work with the natural gas and electric suppliers to confirm the systems have adequate capacity.

8.5.3 Utility Protection During Construction

Existing public and private infrastructure located within nearby public rights-of-way will be protected during Project construction. The installation of proposed utility connections within public ways will be undertaken in accordance with BWSC, Boston Public Works Department, the Dig-Safe Program, and applicable utility company requirements. Specific methods for constructing proposed utilities where they are near to, or connect with, existing water, sewer, and drain facilities will be reviewed by BWSC as part of its Site Plan Review process. All necessary permits will be obtained before the commencement of work.

The Proponent will continue to work and coordinate with the BWSC and the utility companies to ensure safe and coordinated utility operations in connection with the Project.

Coordination with Other Governmental Agencies

9.0 COORDINATION WITH OTHER GOVERNMENTAL AGENCIES

9.1 Architectural Access Board Requirements

The Project will comply with the requirements of the Architectural Access Board and will be designed to comply with the standards of the Americans with Disabilities Act. See Appendix E for the Accessibility Checklist.

9.2 Boston Landmarks Commission Article 85 Review

The proposed demolition of the buildings on the Project Site will be subject to review by the Boston Landmarks Commission under Article 85 of the Boston Zoning Code. An Article 85 Application will be submitted to the BLC for the proposed demolition of the Rectory (1955) and the Convent (1949-1958) buildings.

9.3 Boston Civic Design Commission Article 80 Review

The Project will comply with the provisions of Article 28 of the Boston Zoning Code. This PNF will be submitted to the Boston Civic Design Commission by the BPDA as part of the Article 80 process.

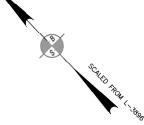
9.4 Massachusetts Historical Commission

The MHC has review authority over projects utilizing or requiring state or federal funding, licensing, permitting and/or approvals that may have direct or indirect impacts to properties listed in the State or National Registers of Historic Places. If the Project requires a state or federal action, the MHC review process will be initiated through the filing of an MHC Project Notification Form as prescribed in MHC's governing regulations.

Appendix A

Site Survey





LEGEND: LEGEND:
SEWER MANHOLE
DRAIN MANHOLE
DRAIN MANHOLE
HANDICAP RAMP
GAS SHUT OFF
BOSTON WATER VALVE
CATCH BASIN
LIGHT POLE
UTILITY POLE
SIGN
TRAFFIC SIGNAL
BITUMINOUS BIT. BITUMINOUS CONCRETE CHAIN LINK FENCE RIM ELEVATION INVERT ELEVATION SEWER DRAIN WATER GAS TELEPHONE OVERHEAD WIRE

REFERENCES

1. BOSTON PUBLIC WORKS DEPARTMENT LAYOUT PLAN L-3896.

2. BOSTON PUBLIC WORKS DEPARTMENT SURVEY FIELD NOTES BK 1232/138-141.

3. CITY OF BOSTON ASSESSING MAPA 1084 & 1085

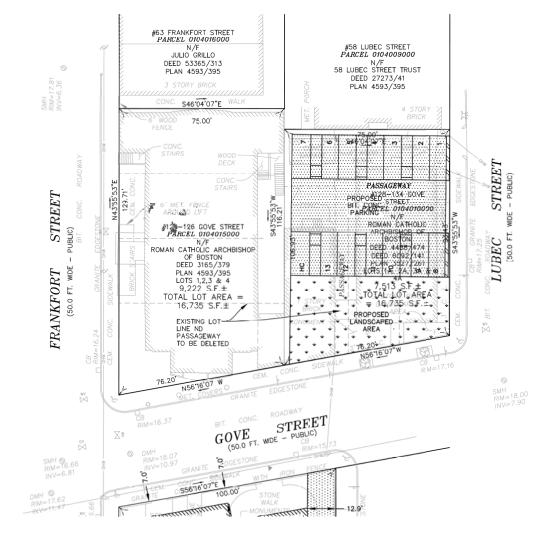
NOTES

1) ELEVATIONS REFER TO BOSTON CITY BASE.

2) UNDERGROUND UTILITIES ARE ONLY SHOWN FROM LIMITED RESEARCH ONLY. THERE IS EVIDENCE OF ADDITIONAL LINES IN THE AREA.

3) THE SURVEY IS BASED ON DEEDS AND PLANS OF RECORD AND ALSO THE TITLE COMMITMENT CASE #C22157 PREPARED BY COMMONWEALTH TITLE INSURANCE COMPANY EFFECTIVE DATE 7-1-15.

4) LOCIS IS NOT LOCATED WITHIN A FLOOD ZONE AS MARKED ON THE CURRENT FLOOD BASE MAPS PREPARED BY THE FEDERAL EMERGENCY MANAGEMENT AGENCY.
SEE MAP 25025C 0081G UPDATED THROUGH SEPTEMBER 25, 2009.



PLAN OF PROPOSED CONSTRUCTION 120-134 GOVE STREET BOSTON, MASSACHUSETTS (EAST BOSTON DISTRICT)



GREATER BOSTON SURVEYING AND ENGINEERING 19 FREDITH ROAD WETWOOTH, MA 02189 (781) 331-6189

CALC BY: PJT CHECK BY: DGM DATE: SEPTEMBER, 2018 SCALE: 1"=20'

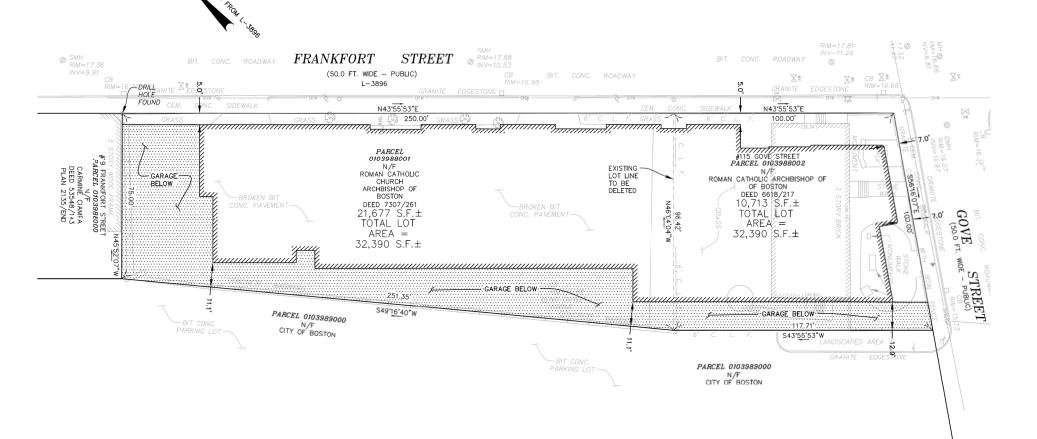
- REFERENCES
 1. BOSTON PUBLIC WORKS DEPARTMENT LAYOUT PLAN L-695, L-1168.
- 2. BOSTON PUBLIC WORKS DEPARTMENT SURVEY FIELD NOTES BK 1232/138-141

NOTES 1) ELEVATIONS REFER TO BOSTON CITY BASE.

- 2) UNDERGROUND UTILITIES ARE ONLY SHOWN FROM LIMITED RESEARCH ONLY. THERE IS EVIDENCE OF ADDITIONAL LINES IN THE AREA.
- 3) THE SURVEY IS BASED ON DEEDS AND PLANS OF RECORD AND ALSO THE TITLE COMMITMENT CASE #C22157 PREPARED BY COMMONWEALTH TITLE INSURANCE COMPANY EFFECTIVE DATE 7-1-15.
- 4) LOCUS IS NOT LOCATED WITHIN A FLOOD ZONE AS MARKED ON THE CURRENT FLOOD BASE MAPS PREPARED BY THE FEDERAL EMERGENCY MANAGEMENT AGENCY.

 SEE MAP 25025C 0081G UPDATED THROUGH SEPTEMBER 25, 2009.





LEGENU:

SEWER MANHOLE
DRAIN MANHOLE
CABLE TV MANHOLE
HANDICAP RAMP
GAS SHUT OFF
WATER SHUT OFF
WATER SHUT OFF
GASTON WATER VALVE
CATCH BASIN
LIGHT POLE
UTILITY POLE
SIGN
TRAFFIC SIGNAL
BITUMINOUS BITUMINOUS CONCRETE CHAIN LINK FENCE RIM ELEVATION INVERT ELEVATION CLF R= I= SEWER DRAIN WATER GAS TELEPHONE OVERHEAD WIRE

LEGEND:

PLAN OF PROPSOED COSNTRUCTION 21-47 FRANKFORT STREET BOSTON, MASSACHUSETTS (EAST BOSTON DISTRICT)



GREATER BOSTON SURVEYING AND ENGINEERING 19 FREDITH ROAD WE'MOUTH, MA 62189 (781) 331–342

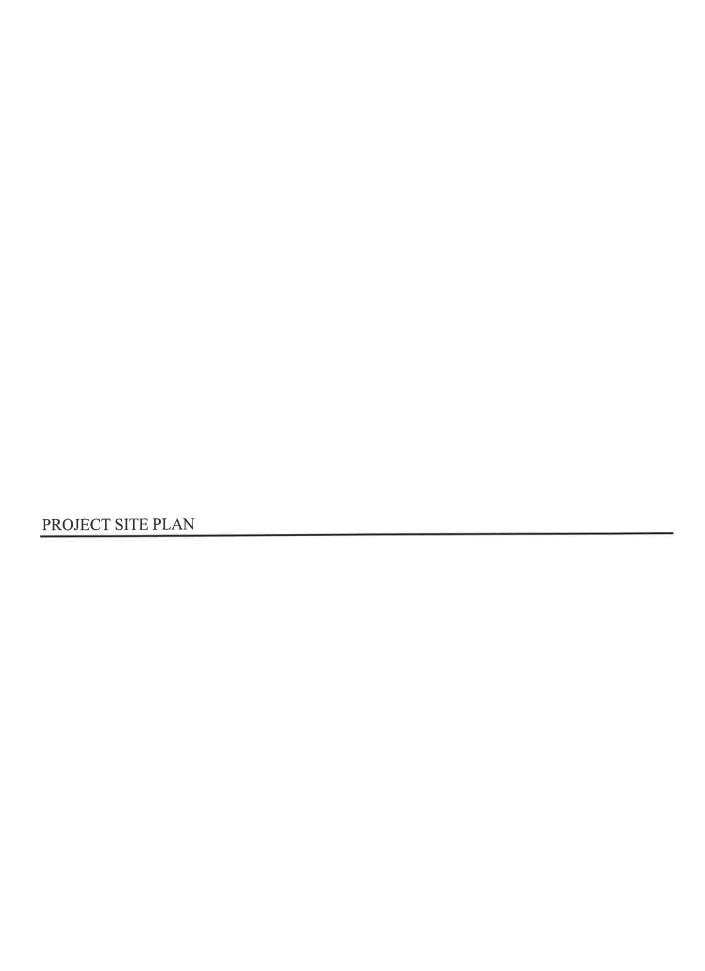
SCALE: 1"=20'

Appendix B

Transportation

APPENDIX

PROJECT SITE PLAN
MANUAL TURNING MOVEMENT COUNT DATA
SEASONAL ADJUSTMENT DATA
PUBLIC TRANSPORTATION SCHEDULES
EAST BOSTON GREENWAY LOCATION MAP
MASSDOT HIGH CRASH LOCATION MAP
GENERAL BACKGROUND TRAFFIC GROWTH
BACKGROUND DEVELOPMENT TRAFFIC-VOLUME NETWORKS
TRIP-GENERATION CALCULATIONS
BTD TRIP DISTRIBUTION
CAPACITY ANALYSIS WORKSHEETS



GOVE ST min. 5'-2' clear at hood of compact car for 3'-0' min. 5'-2' cleer at hood of compact car for 3'-0' SIME WAIT 53,-0, .0-,51 COME MATE 53,-0. 50,-0, .0-.81 Que no A BE BELL S3,-0, 50,-0. S0.-0a 18,-0. CONCENSE ! SHT-BATT entry level on grade meaning above... 18,-0, **P** 24.0, 17.02 J 10% slope OUT EL = 0'-0" EL = -5'-0" adojs %0L .0-,09

FRANKFORT ST

PREVIOUS STUDY - 82 SPACES THIS STUDY - 71 SPACES

33

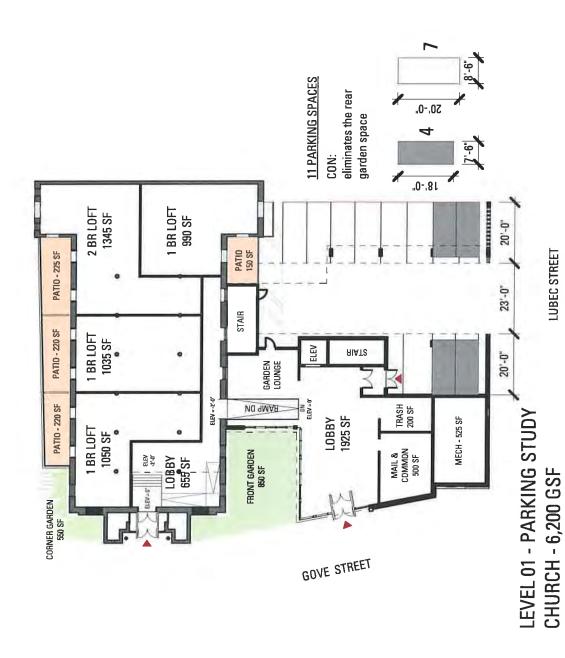
20.-0

32

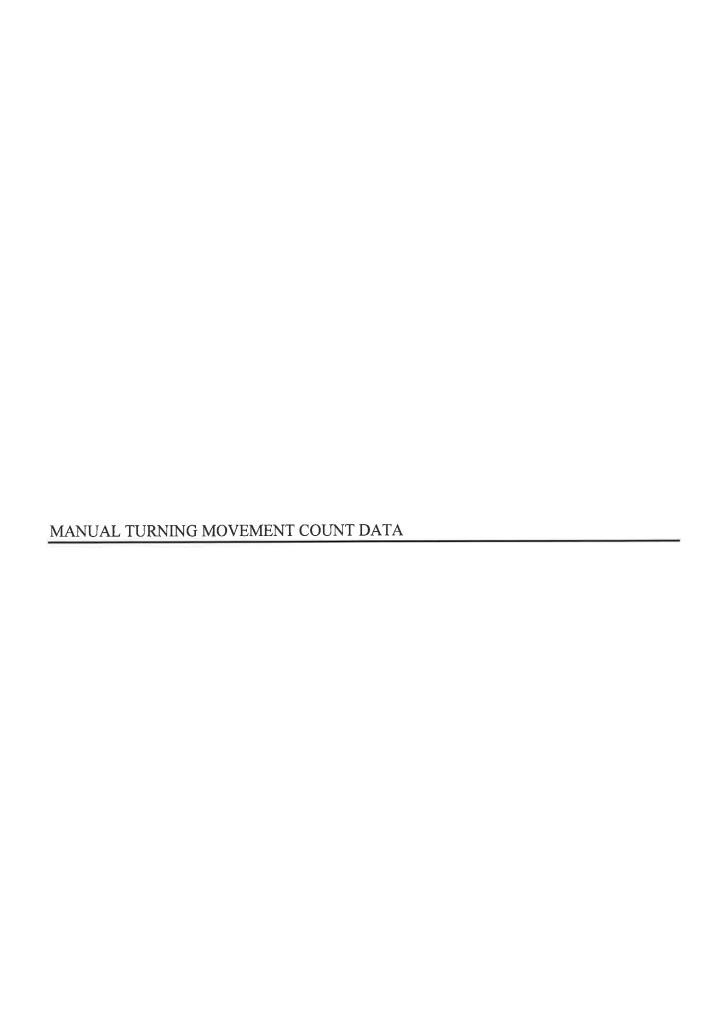
PARKING LEVEL Total GSF - 25,888 SF

SINGLE ENTRY OFF OF FRANKFORT NO TANDEM STUDY

SCALE IS 1:30



NEW BUILDING - 3,850 GSF



Accurate Counts 978-664-2565

File Name 75860001 Site Code 75860001 Start Date 1/25/2018 Page No 1

N/S Street: Frankfort Street E/W Street: Gove Street City/State: Boston, MA Weather: Clear

					Groups P	rinted- Cars -	Trucks						
	Fra	Frankfort St From North		~ [Gove St From Fast	Gove St From East		Frankfort St From South		ŭ.	Gove St From West		
Start Time	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Int. Total
07:00 AM	0	4	-	-	-	4	0	10	0	0	2	0	23
07:15 AM	т	c)	0	0	2	∞	~	Ø	7	-	5	-	37
07:30 AM	S	2	0	9	7	9	~	င	~	2	∞	_	42
07:45 AM	9	4	~	~	6	7	0	4	2	0	4	0	38
Total	14	15	2	«O	19	25	2	26	ro.	ю	19	2	140
08:00 AM	2	2	0	←	7	9	2	15	က	2	6	ю	52
08:15 AM	12	4	7	က	14	o,	2	17	-	-	80	0	99
08:30 AM	7	က	2	2	∞	4	-	10	-	~	ß	0	44
08:45 AM	~	←	0	_	2	7	0	ю	-	₩.	4	-	17
Total	22	10	0	7	31	21	S	39	9	Ω.	26	4	179
Grand Total	36	25	S	15	90	46	7	65	1	00	45	9	319
Apprch %	54.5	37.9	7.6	13.5	45	41.4	8.4	78.3	13.3	13.6	76.3	10.2	
Total %	11.3	7.8	1.6	4.7	15.7	14.4	2.2	20.4	3.4	2.5	14.1	1.9	
Cars	36	24	rC)	15	47	46	7	65	11	00	45	9	315
% Cars	100	96	100	100	94	100	100	100	100	100	100	100	98.7
Trucks	0	-	0	0	m	0	0	0	0	0	0	0	4
% Trucks	0	4	0	0	9	0	0	0	0	0	0	0	1.3

Accurate Counts 978-664-2565

File Name : 75860001 Site Code : 75860001 Start Date : 1/25/2018 Page No : 2

N/S Street: Frankfort Street E/W Street: Gove Street City/State: Boston, MA Weather: Clear

Peak Ho

Peak Ho

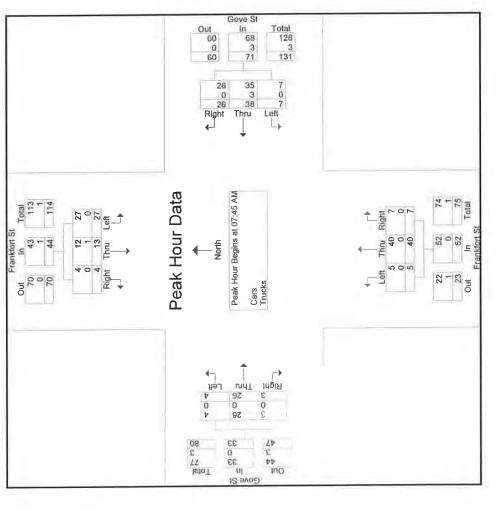
		Frankfort St	fort St			Gov	Gove St			Frankfort St	ort St			Gove St	e St		
		From North	North			From East	East			From South	South			From West	West		
Start Time	Left	Thru	Right	Thru Right App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Leff	Thru	Right	App. Total	Int. Total
c Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1	om 07:00 /	AM to 08:4	15 AM - P	eak 1 of 1													
Hour for Entire Intersection Begins at 07:45 AM	ntersection	Begins at	07:45 AN	5													
07:45 AM	9	4	*	7	-	6	7	17	0	4	2	9	0	4	0	4	38
08:00 AM	2	. 2	0	4	۳	7	9	14	2	15	ო	20	2	6	ო	41	52
08:15 AM	12	4	-	17	က	14	6	26	2	11	_	41	-	00	0	O	99
08:30 AM	7	က	2	12	2	∞	4	14	_	10	-	12	-	2	0	9	44
Total Volume	27	13	4	44	7	38	26	7.1	2	40	7	52	4	26	က	33	200
% App. Total	61.4	29.5	9.1		9.6	53.5	36.6		9.6	6.97	13.5		12.1	78.8	9.1		
PHF	.563	.813	.500	.647	.583	629	.722	.683	.625	799.	.583	.650	.500	.722	.250	.589	.758
Cars	27	12	4	43	7	35	26	89	5	40	7	25	4	26	ო	33	196
% Cars	100	92.3	100	7.78	100	92.1	100	95.8	100	100	100	100	100	100	100	100	98.0
Trucks	0	~	0	~	0	က	0	က	0	0	0	0	0	0	0	0	4
% Trucks	0	7.7	0	2.3	0	7.9	0	4.2	0	0	0	0	0	0	0	0	2.0

Accurate Counts 978-664-2565

File Name 75860001 Site Code 75860001 Start Date 1/25/2018 Page No 3

N/S Street: Frankfort Street E/W Street: Gove Street City/State: Boston, MA Weather: Clear





Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1 Peak Hour for Each Approach Begins at:

0	07:45 AM			0	07:30 AM			0	07:45 AM			J	07:30 AM			
+0 mins.	9	4	***	1	9	7	9	18	0	4	2	9	2	00	-	11
+15 mins.	7	2	0	4	_	6	7	17	2	15	ო	20	0	4	0	4
+30 mins.	12	4	+	17	τ-	7	9	14	2	7	_	4	2	6	က	14
+45 mins.	7	ო	2	12	က	4	თ	26	-	10	-	12	_	00	0	6
otal Volume	27	13	4	44	Ŧ	37	28	9/	co	40	7	52	5	29	4	38
% App. Total	61.4	29.5	9.1		14.5	48.7	36.8		9.6	6.97	13.5		13.2	76.3	10.5	

File Name : 75860001 Site Code : 75860001 Start Date : 1/25/2018 Page No : 5

Int. Total

23

37

N/S Street: Frankfort Street E/W Street: Gove Street City/State: Boston, MA Weather: Clear

	=	0	_	~	0	2	m	0	0	_	4	9	7
	Right												10.2
Gove St From West	Thru	2	5	00	4	19	Ø	00	2	4	26	45	76.3
Fo	Left	0	-	2	0	ю	2	,-	-	-	5	œ	13.6
	Right	0	2	~	7	c2	е	-	F	æ	ဖ	1	13.3
Frankfort St From South	Thru	10	6	¢,	4	26	15		10	ю	36	65	78.3
Fran Fron	Left	0	-	-	0	2	7	2	-	0	ιΩ	7	8.4
	Right	4	_∞	9	7	25	9	o	4	2	21	46	42.6
Gove St From East	Thru	-	2	7	o	19	7	11	_∞	2	28	47	43.5
Q F	Left	*	0	9	-	∞	/-	က	2	~	7	15	13.9
	Right	-	0	0	Ť	2	0	_	2	0	ന	S	7.7
Frankfort St From North	Thru	4	IJ	2	4	15	2	т	ო	~	თ	24	36.9
Frai	Left	0	က	21	9	14	2	12	7	_	22	36	55.4
-	Start Time	07:00 AM	07:15 AM	07:30 AM	07:45 AM	Total	08:00 AM	08:15 AM	08:30 AM	08:45 AM	Total	Grand Total	Apprch %

140

38

62

52

44 17 175

315

File Name : 75860001 Site Code : 75860001 Start Date : 1/25/2018 Page No : 9

N/S Street: Frankfort Street E/W Street: Gove Street City/State: Boston, MA Weather: Clear

					Group	Groups Printed- Trucks	cks						
	Ē ū	Frankfort St From North		- 11	Gove St From East			Frankfort St From South		Ē	Gove St From West		
Start Time	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Int. Total
07:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
07:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
07:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
07:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
Total	0	0	0	0	0	0	0	0	0	0	0	0	0
										,	1	,	•
08:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
08:15 AM	0	-	0	0	က	0	0	0	0	0	0	0	4
08:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
08:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
Total	0		0	0	က	0	0	0	0	0	0	0	4
Grand Total	0	←	0	0	ო	0	0	0	0	0	0	0	4
Apprch %	0	100	0	0	100	0	0	0	0	0	0	0	
Total %	0	25	0	0	75	0	0	0	0	0	0	0	

File Name : 75860001 Site Code : 75860001 Start Date : 1/25/2018 Page No : 13

N/S Street: Frankfort Street E/W Street: Gove Street City/State: Boston, MA Weather: Clear

								Groups	Groups Printed-Bikes Peds	Bikes Pe	sp								
		Frankfort St From North	ort St Jorth			Gove St From Eas	Gove St From East			Frankfort St From South	rt St outh			Gove St From West	St 'est				
Start Time	Left	Thru	Right	Peds	Left	Thru	Right	Peds	Left	Thru	Right	Peds	Left	Thru	Right	Peds	Exclu. Total Inclu. Total	Inclu. Total	Int. Total
07:00 AM	0	0	0	00	0	0	0	0	0	0	0	2	0	0	0	7	12	0	12
07:15 AM	0	0	0	7	0	0	0	-	0	0	0	ო	0	0	0	n	14	0	14
07:30 AM	0	0	0	9	0	0	0	9	0	0	0	თ	0	0	0	0	21	0	21
07:45 AM	0	0	0	00	0	-	0	2	0	0	0	17	0	0	0	ო	30	-	31
Total	0	0	0	29	0		0	o	0	0	0	31	0	0	0	∞	77	-	78
08:00 AM	0	0	0	60	0	0	0	12	0	0	0	33	0	0	0	o	62	0	62
08:15 AM	0	0	0	15	0	0	0	22	0	0	0	24	0	0	0	7	68	0	99
08:30 AM	0	0	0	7	0	-	0	9	0	0	0	10	_	0	0	ω	29	7	31
08:45 AM	0	0	0	7	0	0	0	4	0	0	0	4	0	0	0	2	20	0	20
Total	0	0	0	37	0	+	0	44	0	0	0	7.1	·	0	0	27	179	2	181
Grand Total	0	0	0	99	0	2	0	53	0	0	0	102	+	0	0	35	256	က	259
Apprch %	0	0	0		0	100	0		0	0	0		100	0	0				
Total %	0	0	0		0	2.99	0		0	0	0		33.3	0	0		98.8	1.2	

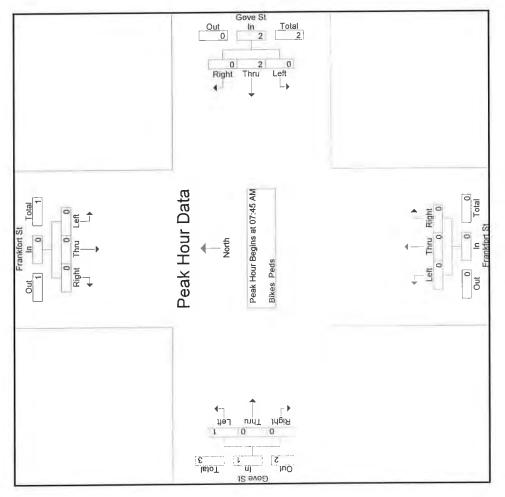
File Name: 75860001 Site Code: 75860001 Start Date: 1/25/2018 Page No: 14

N/S Street: Frankfort Street E/W Street: Gove Street City/State: Boston, MA Weather: Clear

Start Time			Frank	Frankfort St			Gov	Gove St			Frank	Frankfort St			Gove St	e St		
			From	North			From	East			From	South			From	West		
0 0 0 1 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Start Time	Left		Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
0 0 1 1 0 1 0 0 0 0 0 0 0 0 0 0 0 0 0 0	ak Hour Analysis Fr	70m 07:00	AM to 08:	45 AM - P	eak 1 of 1													
0 0	ak Hour for Entire Ir	ntersectio	n Begins at	t 07:45 AN	A													
0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	07:45 AM	0	0	0	0	0	-	0	•	0	0	0	0	0	0	0	0	
0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	08:00 AM	0	0	0	0	0	. 0	0	0	0	0	0	0	0	0	0	0	
0 0 0 0 0 0 0 0 1 0 0 1 0 0 0 0 0 0 0 0	08:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	O	0	
0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	08:30 AM	0	0	0	0	0	~	0	-	0	0	0	0	-	0	0	-	
0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Total Volume	0	0	0	0	0	2	0	2	0	0	0	0	-	0	0	-	
.000 .000 .000 .000 .000 .000 .000 .00	% App. Total	0	0	0		0	100	0		0	0	0		100	0	0		
	PHF	000	000	000	0000	000	.500	000.	.500	000.	000.	000	000.	.250	000.	000	.250	.375

File Name | 75860001 Site Code | 75860001 Start Date | 1/25/2018 Page No | 15

N/S Street: Frankfort Street E/W Street: Gove Street City/State: Boston, MA Weather: Clear



Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1

Peak Hour for Each Approach Begins at:

	07:00 AM			J	07:45 AM			0.1	07:00 AM			0	07:45 AM			
+0 mins.	0	0	0	0	0	_	0	-	0	0	0	0	0	0	0	0
+15 mins.	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
+30 mins.	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
+45 mins.	0	0	0	0	0	←	0	~	0	0	0	0	-	0	0	~
Total Volume	0	0	0	0	0	2	0	2	0	0	0	0	-	0	0	
% App. Total	0	0	0		0	100	0		0	0	0		100	0	0	

File Name | 75860001 Site Code | 75860001 Start Date | 1/25/2018 Page No | 1

N/S Street: Frankfort Street E/W Street: Gove Street City/State: Boston, MA Weather: Clear

	正正	Frankfort St From North		○ F	Gove St From East	Gove St From East		Frankfort St From South) <u>F</u>	Gove St From West		
Start Time	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Int. Total
04:00 PM	က	7	0	+	က	9	0	2	_	7	2	_	31
04:15 PM	—	က	2	2	S.	9	0	က	4	0	ო	0	29
04:30 PM	0	2	_	-	4	т	2	9	0	æ	ю	0	28
04:45 PM	~	Ø	2	4	က	12	0	2	~	~	4	-	39
Total	ß	23	co.	∞	15	27	2	16	9	9	12	7	127
05:00 PM	0	4	٠	0	S.	S	4	7-	0	0	5	~	23
05:15 PM	0	2	0	2	9	4	0	10	4	0	2	2	35
05:30 PM	က	4	2 :	0	4	2	0	ß	1	0	2	2	25
05:45 PM	←	4	7-	~	4	ß	0	7	0	2	~	2	23
Total	4	17	4	m	19	16	3 -	18	ro	2	10	7	106
Grand Total	თ	40	6	11	34	43	ю	34	11	œ	22	6	233
Apprch %	15.5	69	15.5	12.5	38.6	48.9	6.2	70.8	22.9	20.5	56.4	23.1	
Total %	3.9	17.2	3.9	4.7	14.6	18.5	1.3	14.6	4.7	3.4	9.4	3.9	
Cars	6	40	6	11	34	43	က	34	1	00	21	6	232
% Cars	100	100	100	100	100	100	100	100	100	100	95.5	100	9.66
Trucks	0	0	0	0	0	0	0	0	0	0	+-	0	
% Triicke	c	0	c	_	c	_	_	c	_	_	4.5	_	Δ Π

File Name: 75860001 Site Code: 75860001 Start Date: 1/25/2018 Page No: 2

N/S Street: Frankfort Street E/W Street: Gove Street City/State: Boston, MA Weather: Clear

Int. Total Right App. Total From West Gove St Thru Left Right App. Total Frankfort St From South Thr Left Right App. Total From East Gove St Thru Left Right App. Total Frankfort St From North Thru Left Start Time

750 7 9 200 30 0 ი ← 9 ∞ ന 375 4 0 9 25 9 2 16 299 8.3 250 0 0 13 00 19 563 9 က **12** 27 54 750 15 30 16 500 9 Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1 Peak Hour for Entire Intersection Begins at 04:00 PM 0 ~ ∟ 2 S 15.2 69.7 .719 23 S 3 15.2 417 품 04:00 PM 04:15 PM 04:30 PM 04:45 PM Total Volume % App. Total

39

99 20

0

31 28

വ വ

127 100

20

500

10

100

100

100

9 100

24 100

ဖ 100

 $^{\circ}$ 100

100

100 20

100 27

15 100

00 100

33

5 100

23

2 100 0

Cars

100

100

Trucks

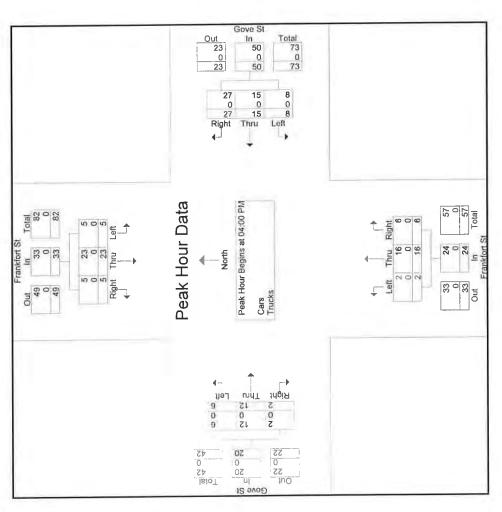
% Trucks

% Cars

0 0

File Name : 75860001 Site Code : 75860001 Start Date : 1/25/2018 Page No : 3

N/S Street: Frankfort Street E/W Street: Gove Street City/State: Boston, MA Weather: Clear



Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1

Peak Hour for Each Approach Begins at:

	9	9	9	4	22	
	0	-		2	4	18.2
		4				
4:30 PM	€	-	0	0	4	18.2
_	00	က	2	4	27	
	0	_	0	4	2	18.5
	9	2	-	10	19	70.4
4:30 PM	2	0	-	0	က	11.1
0	10	13	00	19	20	
		9				
	က	ĸ	4	က	15	30
4:00 PM	-	2	_	4	œ	16
ŏ	10	9	9		33	
	0	2	_	2	2	15.2
	7	60	5	œ	23	2'69
04:00 PM	ო	-	0	_	5	15.2
0	+0 mins.	+15 mins.	+30 mins.	+45 mins.	Total Volume	% App. Total

File Name : 75860001 Site Code : 75860001 Start Date : 1/25/2018 Page No : 5

N/S Street: Frankfort Street E/W Street: Gove Street City/State: Boston, MA Weather: Clear

		Frankfort St From North			Gove St From East			Frankfort St From South) Ľ	Gove St From West		
Start Time	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Int. Total
04:00 PM	က	7	0	-	က	9	0	5	_	2	2	-	31
04:15 PM	~	က	2	2	5	9	0	င	4	0	ю	0	29
04:30 PM	0	5	_	-	4	က	2	9	0	က	ო	0	28
04:45 PM	-	2 0	2	4	6	12	0	2	-	-	4	~	39
Total	S	23	ຸນ	00	5	27	7	16	g	9	12	2	127
05:00 PM	0	4	**	0	വ	2	~	-	0	0	2	~	23
05:15 PM	0	S	0	2	9	4	0	10	4	0	2	2	35
05:30 PM	က	4	2	0	4	2	0	S.	-	0	-	2	24
05:45 PM	_	4	-	-	4	£	0	2	0	2	-	2	23
Total	4	17	4	m	19	16	-	18	S	2	တ	7	105
Grand Total	o	40	თ	7	34	43	က	34	11	œ	21	6	232
Apprch %	15.5	69	15.5	12.5	38.6	48.9	6.2	70.8	22.9	21.1	55.3	23.7	
Total %	3.9	17.2	3.9	4.7	14.7	18.5	1.3	14.7	4.7	3.4	9.1	3.9	

File Name | 75860001 Site Code | 75860001 Start Date | 1/25/2018 Page No | 9

N/S Street: Frankfort Street E/W Street: Gove Street City/State: Boston, MA Weather: Clear

					Group	Groups Printed- Trucks						I	
	Fra	Frankfort St From North			Gove St From East		7.7 5.7	Frankfort St From South		Ē	Gove St From West		
Start Time	Left	Thru	Right	reff	Thru	Right	Left	Thru	Right	Left	Thru	Right	Int. Total
04:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
04:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
04:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
04:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
Total	0	0	0	0	0	0	0	0	0	0	0	0	0
05:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
05:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
05:30 PM	0	0	0	0	0	0	0	0	0	0	-	0	-
05:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
Total	0	0	0	0	0	0	0	0	0	0	-	0	-
Grand Total	0	0	0	0	0	0	0	0	0	0	~	0	
Apprch %	0	0	0	0	0	0	0	0	0	0	100	0	
Total %	0	0	0	0	0	0	0	0	0	0	100	0	

File Name : 75860001 Site Code : 75860001 Start Date : 1/25/2018 Page No : 13

N/S Street: Frankfort Street E/W Street: Gove Street City/State: Boston, MA Weather: Clear

Groups Printed- Bikes Peds

								GLOUD	Groups Printed- Bikes Peds	BIKES PE	SD								
		Frankfort St	t St			Gove St	St			Frankfort St	rt St			Gove St	St				
		From No	orth Th			From East	ast			From South	outh			From West	Vest				
Start Time	Left	Thru Rig	Right	Peds	Left	Thru	Right	Peds	Left	Thru	Right	Peds	Left	Thru	Right	Peds	Exclu, Total	Exclu, Total Inclu. Total	Int. Total
04:00 PM		0	0	10	0	0	0	e	0	0	0	_	0	0	0	3	19	0	19
04-15 PM	0	0	0	7	0	0	0	භ	0	0	0	~	0	0	0	4	19	0	19
04:30 PM	0	0	0	Ø	0	0	-	2	0	0	0	15	0	0	0	10	36	-	37
04.45 PM	0	0	0	7	0	0	0	0	0	0	0	10	0	0	0	7	19	0	19
Total	0	0	0	37	0	0	۳	œ	0	0	0	27	0	0	0	21	93	. -	94
05:00 PM	0	0	0	=======================================	0	0	0	ю	0	0	0	S	0	0	0	90	27	0	27
05:15 PM	0	0	0	15	0	0	0	ю	0	0	0	12	0	0	0	7	32	0	32
05:30 PM	0	0	0	o	0	0	0	Ø	0	0	0	9	0	0	0	Ŋ	28	0	28
05:45 PM	0	0	0	9	0	-	0	2	0	0	0	Ŋ	0	0	0	2	15	÷	16
Total	0	0	0	41	0		0	16	0	0	0	28	0	0	0	17	102		103
Grand Total	0	0	0	78	0	~	-	24	0	0	0	55	0	0	0	38	195	2	197
Apprch % Total %	0 0	0 0	0 0		0 0	50	50		0 0	0 0	0 0		0 0	0 0	0 0		66	+	

File Name | 75860001 Site Code | 75860001 Start Date | 1/25/2018 Page No | 14

N/S Street: Frankfort Street E/W Street: Gove Street City/State: Boston, MA Weather: Clear

		Frankfort St From North	ort St Jorth		0 Ę	Gove St From East			Frankfort St From South	ort St South			Gov	Gove St From West		
Start Time	Left	Thru:	Thru Right App. Total	l Left	Thru	L	Right App. Total	Left	Thru	Right	Right App. Total	Left	Thru	Right	Right App. Total	Int. Total
Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1	m 04:00	PM to 05:4	5 PM - Peak 1 of 1													
Peak Hour for Entire Intersection Begins at 04:00 PM	ersection	Begins at (04:00 PM													
04:00 PM	0	0	0	0	J	0 (0	0	0	0	0	0	0	0	0	
04:15 PM	0	0	0	0	J) 0	0	0	0	0	0	0	0	0	0	
04:30 PM	0	0	0	0 (J	1	•	0	0	0	0	0	0	0	0	
04:45 PM	0	0	0	0	_) 0	0	0	0	0	0	0	0	0	0	
Total Volume	0	0	0	0 () 1	-	0	0	0	0	0	0	0	0	
% App. Total	0	0	0	0	7	0 100		0	0	0		0	0	0		
HH	000	000	.000.	000.	000.	0 .250	.250	000.	000	000	000	000.	000.	000.	000	.250

File Name : 75860001 Site Code : 75860001 Start Date : 1/25/2018 Page No : 15

N/S Street: Frankfort Street E/W Street: Gove Street City/State: Boston, MA Weather: Clear

Gove St In Out 0 Total 1 Left Thru Right Peak Hour Data Peak Hour Begins at 04:00 PM Total Left Frankfort St Out In Out In Frankfort St Right Thru North Bikes Peds nuq_ O Might D Left JuO Total IS eve St onl

Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1 Peak Hour for Each Approach Begins at:

	04:00 PIM			0,	04:00 PM			Ö	04:00 PM			Ď	4:00 P.M			
+0 mins.	0	0	0	0	0	0	0		0	0	0	0	0			0
+15 mins.	0	0	0	0	0	0	0		0 0			0	0	0	0	0
+30 mins.	0	0	0	0	0	0			0	0		0	0			0
+45 mins.	0	0	0	0	0	0	0		0			0	0			0
Total Volume	0	0	0	0	0	0		-	0		0	0	0			0
% App. Total	0	0	0		0	0	100		0	0			0		0	

File Name 75860002 Site Code 75860002 Start Date 1/25/2018 Page No 1

N/S Street : Orleans Street E/W Street : Gove Street City/State : Boston, MA Weather : Clear

Confeans St Gove St Gove St From Rast From St From East From East From East From St From St From St From East From St From St </th <th></th> <th></th> <th></th> <th></th> <th></th> <th>Groups P</th> <th>rinted- Cars -</th> <th>Trucks</th> <th></th> <th></th> <th></th> <th></th> <th></th> <th></th>						Groups P	rinted- Cars -	Trucks						
Left Thru Right Left Thru Right Left <			Orleans St From North		Ů È	Sove St om East			Orleans St From South		Ľ.	Gove St From West		
1 6 0 2 0 2 0 1 9 11 1 0 5 0 0 1 15 0 1 0 5 0 0 4 14 0 5 0 18 0 0 1 14 0 5 0 7 0 0 0 11 0 1 0 4 0 0 17-3 81.8 0.9 1 0 44 0 0 19 90 1 20 0 4 0 0 19 90 1 0 1 0 0 0 19 88 1 10 95 0 11.8 0 0 10 97.8 10 95 0 11.8 0 <th>Start Time</th> <th></th> <th>Thru</th> <th>Right</th> <th></th> <th>Thru</th> <th>Right</th> <th>-</th> <th>Thru</th> <th>Right</th> <th>Left</th> <th>Thru</th> <th>Right</th> <th>Int. Total</th>	Start Time		Thru	Right		Thru	Right	-	Thru	Right	Left	Thru	Right	Int. Total
3 11 1 0 0 5 0 1 9 0 1 0 5 0 5 15 0 1 0 0 4 10 0 5 0 12 0 4 14 0 5 0 7 0 0 11 0 5 0 4 0 1 14 0 5 0 7 0 0 11 0 1 0 4 0 19 49 0 1 0 44 0 17.3 81.8 0.3 32.3 0 42 0 10 95 0 11.8 0 0 11.8 0 10 97.8 10 95.0 0 0 0 10 97.8 10 95.0 0 0 0	07:00 AM	~	9	0	2	0	2	0	Ξ	2	0	-	0	25
1 9 0 1 0 5 0	07:15 AM	т	-	~	0	0	S	0	22	7	0	0	0	49
5 15 0 3 0 6 0 10 41 1 6 0 18 0 4 10 0 5 0 7 0 1 14 0 5 0 7 0 0 11 0 1 0 4 0 19 49 0 1 0 4 0 17.3 81.8 0.3 32.3 0 67.7 0 19 88 11 19 0 44 0 100 97.8 100 95 0 95.2 0	07:30 AM	~	O	0	*	0	2	0	27	S	0	0	0	48
10 41 1 6 0 18 0 4 10 0 5 0 7 0 4 14 0 5 0 7 0 0 11 0 5 0 7 0 0 11 0 4 0 0 0 0 19 49 0 14 0 44 0 0 17.3 81.8 0.9 32.3 0 67.7 0 5.3 25.2 0.3 5.6 0 40 0 10 97.8 100 95 0 95.2 0 0 2 0 1 0 95.2 0 0 2 0 1 0 0 0	07:45 AM	5	15	0	က	0	9	0	18	4	0	0	0	51
4 10 0 5 0 7 0 4 14 0 5 0 12 0 1 14 0 3 0 44 0 0 11 0 1 0 44 0 19 49 0 14 0 24 0 17.3 81.8 0.9 32.3 0 67.7 0 5.3 25.2 0.3 5.6 0 11.8 0 100 97.8 100 95 0 95.2 0 0 2 0 1 0 95.2 0 0 2 0 1 0 0 0	Total	10	14	+	9	0	18	0	78	85	0	-	0	173
4 14 0 5 0 12 0 1 14 0 3 0 4 0 0 11 0 1 0 0 9 11 0 14 0 1 0 19 90 14 0 24 0 0 17.3 81.8 0.9 32.3 0 67.7 0 5.3 25.2 0.3 5.6 0 11.8 0 100 97.8 100 95 0 95.2 0 0 2 0 1 0 95 0 0	08:00 AM	4	10	0	ro	0	7	0	29	4	0	0	0	59
1 14 0 3 0 4 0 0 11 0 1 0 1 0 19 49 0 14 0 24 0 173 81.8 0.9 32.3 0 67.7 0 5.3 25.2 0.3 5.6 0 11.8 0 100 97.8 100 95 0 95.2 0 0 2 0 1 0 95 0	08:15 AM	4	4	0	2	0	12	0	19	9	0	0	0	09
0 11 0 1 0 1 0 9 49 0 14 0 24 0 17.3 81.8 0.9 32.3 0 67.7 0 5.3 25.2 0.3 5.6 0 11.8 0 19 88 1 19 0 40 0 100 97.8 100 95 0 95.2 0 0 2 0 1 0 2 0	08:30 AM	~	4	0	3	0	4	0	15	е	0	0	0	40
9 49 0 14 0 24 0 19 90 1 20 0 42 0 17.3 81.8 0.9 32.3 0 67.7 0 5.3 25.2 0.3 5.6 0 11.8 0 19 88 1 19 0 40 0 100 97.8 100 95 0 95.2 0 0 2 0 1 0 2 0	08:45 AM	0	7	0	~	0	-	0	11	-	0	0	0	25
19 90 1 20 0 42 0 17.3 81.8 0.9 32.3 0 67.7 0 5.3 25.2 0.3 5.6 0 11.8 0 19 88 1 19 0 40 0 100 97.8 100 95 0 95.2 0 0 2 0 1 0 2 0	Total	o	49	0	41	0	24	0	74	14	0	0	0	184
17.3 81.8 0.9 32.3 0 67.7 0 5.3 25.2 0.3 5.6 0 11.8 0 19 88 1 19 0 40 0 100 97.8 100 95 0 95.2 0 0 2 0 1 0 2 0	Grand Total	19	06	_	20	0	42	0	152	32	0	-	0	357
5.3 25.2 0.3 5.6 0 11.8 0 19 88 1 19 0 40 0 100 97.8 100 95 0 95.2 0 0 2 0 1 0 2 0	Apprch %	17.3	81.8	6.0	32.3	0	2.79	0	82.6	17.4	0	100	0	
19 88 1 19 0 40 0 100 97.8 100 95 0 95.2 0 0 2 0 1 0 2 0	Total %	5.3	25.2	0.3	5.6	0	11.8	0	45.6	თ	0	0.3	0	
100 97.8 100 95 0 95.2 0 2 0 1 0 2	Cars	19	88	-	19	0	40	0	149	32	0	-	0	349
0 2 0 1 0 2	% Cars	100	97.8	100	95	0	95.2	0	86	100	0	100	0	8.76
	Trucks	0	2	0	-	0	2	0	က	0	0	0	0	80
0 2.2 0 5 0 4.8	% Trucks	0	2.2	0	2	0	4.8	0	2	0	0	0	0	2.2

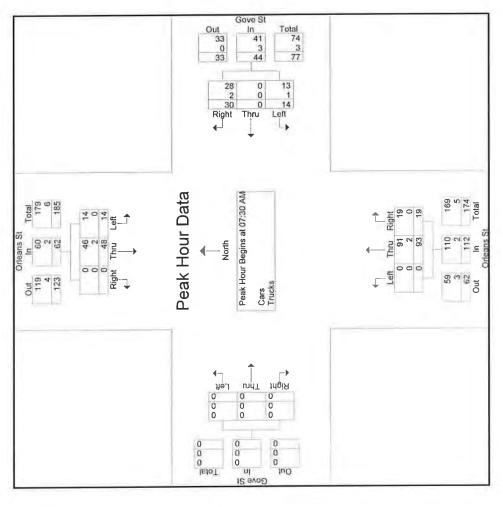
File Name | 75860002 Site Code | 75860002 Start Date | 1/25/2018 Page No | 2

N/S Street : Orleans Street E/W Street : Gove Street City/State : Boston, MA Weather : Clear

From West Gove St Thru Left Right App. Total From South Orleans St Thr Left Right App. Total From East Gove St Thru Left Thru Right App. Total From North Orleans St Left Start Time

File Name 75860002 Site Code 75860002 Start Date 1/25/2018 Page No 3

N/S Street : Orleans Street E/W Street : Gove Street City/State : Boston, MA Weather : Clear



Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1

Peak Hour for Each Approach Begins at:

5	07:45 AM			0	07:45 AM			10	07:15 AM			0	07:00 AM			
+0 mins.	S	15	0	20	က	0	9	O	0	22	7	29	0	_	0	_
+15 mins.	4	10	0	14	ιΩ	0	7	12	0	27	2	32	0	0	0	0
+30 mins.	4	14	0	18	2	0	12	17	0	18	4	22	0	0	0	0
+45 mins.	-	14	0	15	က	0	4	7	0	29	4	33	0	0	0	0
otal Volume	14	53	0	29	16	0	29	45	0	96	20	116	0	-	0	0
% App. Total	20.9	79.1	0		35.6	0	64.4		0	82.8	17.2		0	100	0	

File Name : 75860002 Site Code : 75860002 Start Date : 1/25/2018 Page No : 5

N/S Street: Orleans Street E/W Street: Gove Street City/State: Boston, MA Weather: Clear

	5	Orleans St		G	Gove St	Gloups Fillian-Cals		Orleans St			Gove St		
	4	From North		F	From East		Ē	From South		- 17	rom West		
Start Time	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Int. Total
07:00 AM	***	9	0	2	0	2	0	7	2	0	-	0	25
07:15 AM	က	7	-	0	0	S	0	22	7	0	0	0	49
07:30 AM	~	ō	0		0	22	0	26	5	0	0	0	47
07:45 AM	S	14	0	က	0	9	0	18	4	0	0	0	50
Total	10	40	~	9	0	18	0	77	18	0	-	0	171
08:00 AM	4	တ	0	5	0	2	0	29	4	0	0	0	58
08:15 AM	4	4	0	4	0	10	0	18	9	0	0	0	99
08:30 AM	-	14	0	က	0	4	0	41	က	0	0	0	39
08:45 AM	0	1	0	-	0	-	0	7	~	0	0	0	25
Total	თ	48	0	13	0	22	0	72	14	0	0	0	178
Grand Total	19	88	~	19	0	40	0	149	32	0	—	0	349
Apprch %	17.6	81.5	6.0	32.2	0	67.8	0	82.3	17.7	0	100	0	
Total %	5.4	25.2	0.3	5.4	0	11.5	0	42.7	9.5	0	0.3	0	

File Name | 75860002 Site Code | 75860002 Start Date | 1/25/2018 Page No | 9

N/S Street : Orleans Street E/W Street : Gove Street City/State : Boston, MA Weather : Clear

Corleans St From South Gove St From West Left Thru Right Left Thru 0 0 0 0 0 0 1 0 0 0 0 0 1 0 0 0 0 0 1 0 0 0 0 0 1 0 0 0 0 0 1 0 0 0 0 0 1 0 0 0 0 0 2 0 0 0 0 0 3 0 0 0 0 0 100 0 0 0 0 0 37.5 0 0 0 0						Group	Groups Printed- Trucks	ıcks						
Left Thru Right Right<		О Д	Orleans St		Ú	Sove St			rleans St om South		Ē	Gove St		
0 0	Start Time	Left	Thru	Right		Thru	Right	_	Thru	Right		Thru	Right	Int. Total
0 0	07:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
0 1 0	07:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
0 1 0	07:30 AM	0	0	0	0	0	0	0	-	0	0	0	0	_
0 1 0 0 0 0 1 0	07:45 AM	0	-	0	0	0	0	0	0	0	0	0	0	1
0 1 0	Total	0	-	0	0	0	0	0	-	0	0	0	0	2
0 0 1 0 1 0 1 0	08:00 AM	0	-	0	0	0	0	0	0	0	0	0	0	←
0 0	08:15 AM	0	0	0		0	2	0	-	0	0	0	0	4
0 0	08:30 AM	0	0	0	0	0	0	0	1	0	0	0	0	-
0 1 0 1 0 0 2 0 0 0 0 0 0 0 0 0 0 0 0 0	08:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
0 2 0 3 0 0 100 0 33.3 0 66.7 0 100 0 0 25 0 12.5 0 25 0 37.5 0	Total	0	~	0	-	0	7	0	2	0	0	0	0	9
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	Grand Total	0	2	0	~	0	2	0	က	0	0	0	0	00
0 25 0 12.5 0 25 0 37.5 0	Apprch %	0	100	0	33.3	0	2.99	0	100	0	0	0	0	
	Total %	0	25	0	12.5	0	52	0	37.5	0	0	0	0	

File Name 75860002 Site Code 75860002 Start Date 1/25/2018 Page No 13

N/S Street: Orleans Street E/W Street: Gove Street City/State: Boston, MA Weather: Clear

							Groups	Groups Printed-Bikes Peds	Bikes Per	ds								
	Orleans St From North	is St Jorth			Gove St From East	St			Orleans St From South	s St outh			Gove St From West	St est				
Left	Thru	Right	Peds	Left	Thru	Right	Peds	Left	Thru	Right	Peds	Left	Thru	Right	Peds	Exclu. Total Inclu. Total	Inclu. Total	Int. Total
0	0	0	10	0	0	0	4	0	0	0	6	0	0	0	7	30	0	30
0	0	0	4	0	0	0	က	0	0	0	9	0	0	0	5	18	0	18
0	0	0	19	0	0	0	7	0	0	0	~	0	0	0	0	31	0	31
0	0	0	21	0	~	0	12	0	0	0	9	0	0	0	4	43	-	44
0	0	0	54	0	-	0	30	0	0	0	22	0	0	0	16	122	F	123
0	0	0	20	0	0	0	6	0	0	0	21	0	0	0	21	65	0	65
0	0	0	31	0	0	0	ო	0	0	0	18	0	0	0	15	29	0	29
0	0	0	10	0	0	0	9	0	0	0	Ø	0	0	0	9	28	0	28
0	0	0	7	0	0	0	0	0	0	0	m	0	0	0	-	1	0	1
0	0	0	89	0	0	0	12	0	0	0	48	0	0	0	43	171	0	171
0	0	0	122	0	-	0	42	0	0	0	70	0	0	0	59	293		294
0	0	0		0	100	0		0	0	0		0	0	0				
0	0	0		0	100	0		0	0	0		0	0	0		99.7	0.3	

File Name : 75860002 Site Code : 75860002 Start Date : 1/25/2018 Page No : 14

N/S Street : Orleans Street E/W Street : Gove Street City/State : Boston, MA Weather : Clear

		Orlea	Orleans St			Gove St	e St			Orlea	Orleans St			Gove St	s St		
		From	From North			From East	East			From	From South			From West	West		
Start Time	Left		Thru: Right App. Total	\pp. Total	Left	Thru	Right	Right App. Total	Left	Thru	Right /	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1	om 07:00	AM to 08:4	45 AM - Pea	k 1 of 1	+												
Peak Hour for Entire Intersection Begins at 07:00 AM	ntersection	ι Begins at	t 07:00 AM														
07:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
07:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
07:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
07:45 AM	0	0	0	0	0	-	0	-	0	0	0	0	0	0	0	0	-
Total Volume	0	0	0	0	0	-	0	1	0	0	0	0	0	0	0	0	
% App. Total	0	0	0		0	100	0		0	0	0		0	0	0		
PHF	000	000	000.	000.	000	.250	000.	.250	000.	000	000	000.	000	000	000	000.	.250

File Name 75860002 Site Code 75860002 Start Date 1/25/2018 Page No 15

N/S Street : Orleans Street E/W Street : Gove Street City/State : Boston, MA Weather : Clear

Gove St In Out 0 Total 1 Thru Left Right Peak Hour Data Peak Hour Begins at 07:00 AM Total Total 0 f lef Out In Out In Right Thru North Bikes Peds nuq1 о Нејаћ tuO 1 latoT f IS avoð ni

Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1

Peak Hour for Each Approach Begins at:

	07:00 AM			07	07:00 AM			0	07:00 AM			0	07:00 AM			
+0 mins.	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
+15 mins.	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
+30 mins.	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
+45 mins.	0	0	0	0	0	-	0	-	0	0	0	0	0	0	0	0
Total Volume	0	0	0	0	0	-	0	-	0	0	0	0	0	0	0	0
% App. Total	0	0	0		0	100	0		0	0	0		0	0	0	

File Name: 75860002 Site Code: 75860002 Start Date: 1/25/2018 Page No: 1

N/S Street : Orleans Street E/W Street : Gove Street City/State : Boston, MA Weather : Clear

					Groups P	rinted- Cars -	Trucks						
		Orleans St		щ	Gove St From East			Orleans St From South		0 1	Gove St From West		
Start Time	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Int. Total
04:00 PM	-	7	0	m	0	က	0	15	-	0	0	0	30
04:15 PM	က	-	0	•	0	4	0	17	4	0	0	0	40
04:30 PM	က	18	0	0	0	7	0	24	2	0	0	0	54
04:45 PM	4	16	0	+	0	5	0	16	4	0	0	0	46
Total	#	52	0	ιΩ	0	19	o	72	11	0	0	0	170
05:00 PM		18	0	0	0	4	0	23	5	0	0	0	51
05:15 PM	2	16	0	0	0	7	7-	13	4	-	0	0	44
05:30 PM	က	16	0	0	0	9	0	19	~	0	0	0	45
05:45 PM	0	19	-	0	0	4	0	16	8	0	0	0	42
Total	9	69		0	0	21	5	7.1	12	+	0	0	182
Grand Total	17	121	1	5	0	40	~	143	23	~	0	0	352
Apprch %	12.2	87.1	0.7	11.1	0	88.9	9.0	85.6	13.8	100	0	0	
Total %	4.8	34.4	0.3	1.4	0	11.4	0.3	40.6	6.5	0.3	0	0	
Cars	16	121	_	5	0	39	-	142	23	-	0	0	349
% Cars	94.1	100	100	100	0	97.5	100	99.3	100	100	0	0	99.1
Trucks		0	0	0	0	-	0	*	0	0	0	0	က
% Trucks	5.9	0	0	0	0	2.5	0	0.7	0	0	0	0	6.0

File Name : 75860002 Site Code : 75860002 Start Date : 1/25/2018 Page No : 2

N/S Street: Orleans Street E/W Street: Gove Street City/State: Boston, MA Weather: Clear

S. evo. t.

		Orles	Orleans St			ס י	Gove St			Orlea	Orleans St			Gove St	s St		
		From	From North			Ϋ́	From East			From	From South			From west	vvest		
Start Time	Left	Thru		Right App. Total	l Left	. Thru	Right	t App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1	rom 04:00	PM to 05:4	45 PM - P	eak 1 of 1													
Peak Hour for Entire Intersection Begins at 04:30 PM	ntersection	า Begins at	t 04:30 PN	5													
04:30 PM	က	18	0	21	0	0	1~	7	0	24	2	56	0	0	0	0	54
04:45 PM	4	16	0	20	_	0		9	0	16	4	20	0	0	0	0	46
05:00 PM	_	18	0	19	0	0	7	4	0	23	ro	28	0	0	0	0	51
05:15 PM	2	16	0	18	0	0	. *	7	_	13	4	18	-	0	0	-	44
Total Volume	10	68	0	78	-1	0	23	3 24	-	92	15	92	-	0	0		195
% App. Total	12.8	87.2	0		4.2	0	95.8	~	1.1	82.6	16.3		100	0	0		
PHF	.625	.944	000	929	9 .250	000.	.821	1 .857	.250	.792	.750	.821	.250	000	000	.250	.903
Cars	10	89	0	78	3	0	1 23	3 24	_	75	15	91	-	0	0	-	194
% Cars	100	100	0	100	0 100) (100	0 100	100	98.7	100	98.9	100	0	0	100	99.5
Trucks	0	0	0	J	0 0	0		0 0	0	~	0	_	0	0	0	0	
% Trucks	0	0	0)) () (_	0 0	0	1.3	0	1.1	0	0	0	0	0.5

File Name 75860002 Site Code 75860002 Start Date 1/25/2018 Page No 3

N/S Street : Orleans Street E/W Street : Gove Street City/State : Boston, MA Weather : Clear

Total 49 0 49 Gove St In 24 0 24 Out 25 0 25 0 0 0 Thru 1 0 1 Left 23 0 23 Right Peak Hour Data Peak Hour Begins at 04:30 PM Right 15 0 161 Total Total 177 1 505 91 1 92 In Orleans St m 78 0 87 North 89 0 89 Thru 69 69 Out 0 0 0 Right 99 100 Cars Trucks лиц<u>т</u> о 0 0 0 1dgiЯ ¥-∏eµ 0 ino ino Tolal 2 0 i 10 Gove St

Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1 Pea

at:	
Begins	
Approach	
Each	
our for	
ak Ho	

J	04:30 PM			O	04:00 PM			Ò	04:15 PM				04:30 PM			
+0 mins.	က	18	0	21	ო	0	က	_{(O}	0	17	4	21	0	0	0	0
+15 mins.	4	16	0	20	_	0	4	ιΩ	0	24	2	26	0	0	0	0
+30 mins.	_	18	0	19	0	0	7	7	0	16	4	20	0	0	0	0
+45 mins.	2	16	0	18	-	0	S	9	0	23	ĸ	28	1	0	0	7
otal Volume	10	89	0	78	S	0	18	24	0	80	15	95	_	0	0	-
% App. Total	12.8	87.2	0		20.8	0	79.2		0	84.2	15.8		100	0	0	

File Name: 75860002 Site Code: 75860002 Start Date: 1/25/2018 Page No: 5

N/S Street : Orleans Street E/W Street : Gove Street City/State : Boston, MA Weather : Clear

					Group	Groups Printed- Cars	ars						
	У Ц	Orleans St From North		~ ii	Gove St From East			Orleans St From South		O P	Gove St From West		
Start Time	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Int. Total
04:00 PM	-	7	0	က	0	2	0	15	-	0	0	0	29
04:15 PM	ო	1	0	~	0	4	0	17	4	0	0	0	40
04:30 PM	ю	18	0	0	0	7	0	23	2	0	0	0	53
04:45 PM	4	16	0	_	0	ß	0	16	4	0	0	0	46
Total	17	52	0	Ω	0	18	0	7.1	11	0	0	0	168
05:00 PM	-	18	0	0	0	4	0	23	c,	0	0	0	51
05:15 PM	2	16	0	0	0	7		13	4	₩	0	0	44
05:30 PM	2	16	0	0	0	9	0	19	~	0	0	0	44
05:45 PM	0	19	~	0	0	4	0	16	7	0	0	0	42
Total	22	69	+	0	0	21	-	71	12	-	0	0	181
Grand Total	16	121	_	5	0	39	4	142	23	~	0	0	349
Apprch %	11.6	87.7	0.7	11.4	0	9.88	9.0	85.5	13.9	100	0	0	
Total %	4.6	34.7	0.3	1.4	0	11.2	0.3	40.7	9.9	0.3	0	0	

File Name 75860002 Site Code 75860002 Start Date 1/25/2018 Page No 9

N/S Street: Orleans Street E/W Street: Gove Street City/State: Boston, MA Weather: Clear

	P. P.	Orleans St From North		S. F.	Gove St From East		ŌΈ	Orleans St From South		O F	Gove St From West		
Start Time	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Int. Total
04:00 PM	0	0	0	0	0	-	0	0	0	0	0	0	
04:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
04:30 PM	0	0	0	0	0	0	0	-	0	0	0	0	
04:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	
Total	0	0	0	0	0	-	0	~	0	0	0	0	N
05:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
05:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	
05:30 PM		0	0	0	0	0	0	0	0	0	0	0	
05:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	
Total	≈	0	0	0	0	0	0	0	0	0	0	0	
Grand Total	4	0	0	0	0	_	0	~	0	0	0	0	
Apprch %	100	0	0	0	0	100	0	100	0	0	0	0	
Total %	33.3	0	0	0	0	33.3	С	33.3	0	C	C	0	

File Name : 75860002 Site Code : 75860002 Start Date : 1/25/2018 Page No : 13

N/S Street: Orleans Street E/W Street: Gove Street City/State: Boston, MA Weather: Clear

								Group	Groups Printed- Bikes Peds	Bikes Pe	ds								
		Orleans St	s St			Gove St	St			Orleans St	s St			Gove St	St				
		From North	orth			From East	ast			From South	outn			From M	esi				
Start Time	Left	Thru	Right	Peds	Left	Thru	Right	Peds	Left	Thru	Right	Peds	Left	Thru	Right	Peds	Peds Exclu. Total Inclu. Total	Inclu. Total	Int. Iotal
04:00 PM	0	0	0	7	0	0	0	ന	0	-	0	2	0	0	0	2	18	+	19
04:15 PM	0	0	0	7-	0	0	0	~	0	0	0	_	0	0	0	ю	16	0	16
04:30 PM	0	0	0	7	0	0	0	ო	0	0	0	o	0	0	0	4	27	0	27
04:45 PM	0	0	0	10	0	0	0	2	0	0	0	00	0	0	0	ro	25	0	25
Total	0	0	0	43	0	0	0	o	0	-	0	20	0	0	0	4	80	-	87
05:00 PM	0	0	0	12	0	0	0	2	0	-	0	o	0	0	0	4	27	*	28
05:15 PM	0	0	0	4	0	0	0	2	0	0	0	-	0	0	0	oo	35	0	35
05:30 PM	0	0	0	00	0	0	0	e	0	0	0	7	0	0	0	7	25	0	25
05:45 PM	0	0	0	7	0	0	0	2	0	0	0	S	0	0	0	4	18	0	18
Total	0	0	0	41	0	0	0	σ	0	æ	0	32	0	0	0	23	105		106
Grand Total	0	0	0	84	0	0	0	18	0	2	0	52	0	0	0	37	191	2	193
Apprch %	0	0	0		0	0	0		0	100	0		0	0	0				
Total %	0	0	0		0	0	0		0	100	0		0	0	0		66	1	

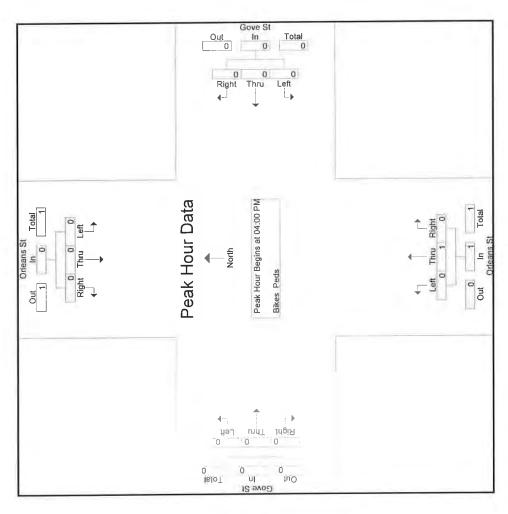
File Name : 75860002 Site Code : 75860002 Start Date : 1/25/2018 Page No : 14

N/S Street : Orleans Street E/W Street : Gove Street City/State : Boston, MA Weather : Clear

		Orleans St	ns St			Gove St	e St			Orles	Orleans St			Gov	Gove St		
		From North	North			From East	East			From	From South			From	From West		
Start Time	Left	Thru	Right	Right App. Total	Left	Thru	Right	Right App. Total	Left	Thru	Right	Right App. Total	Left	Thru	Right	Right App. Total	Int. Total
Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1	04:00 Pi	M to 05:4.	5 PM - Pe	3ak 1 of 1													
Peak Hour for Entire Intersection Begins at 04:00 PM	section E	degins at	04:00 PM	_													
04:00 PM	0	0	0	0	0	0	0	0	0	-	0	~	0	0	0	0	
04:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
04:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
04:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
Total Volume	0	0	0	0	0	0	0	0	0	-	0	-	0	0	0	0	
% App. Total	0	0	0		0	0	0		0	100	0		0	0	0		
PHF	000	000	000	000	000	000	000	000	000	.250	000	.250	000	000	000	000.	.250

File Name : 75860002 Site Code : 75860002 Start Date : 1/25/2018 Page No : 15

N/S Street : Orleans Street E/W Street : Gove Street City/State : Boston, MA Weather : Clear



Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1

Peak Hour for Each Approach Begins at:

٠	04:00 PM			70	04:00 PM			0	04:00 PM			8	04:00 PM			
+0 mins.	0	0	0	0	0	0	0	0	0	←	0	-	0	0	0	0
+15 mins.	0	0	0	0	0	0	0	0	0	0	0	0		0	0	0
+30 mins.	0	0	0	0	0	0	0	0	0	0	0	0		0	0	0
+45 mins.	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Fotal Volume	0	0	0	0	0	0	0	0	0	-	0	1		0	0	0
% App. Total	0	0	0		0	0	0		0	100	0		0	0	0	

N/S Street : Bremen Street E/W Street : Gove Street City/State : Boston, MA Weather : Clear

File Name : 75860003 Site Code : 75860003 Start Date : 1/25/2018 Page No : 1

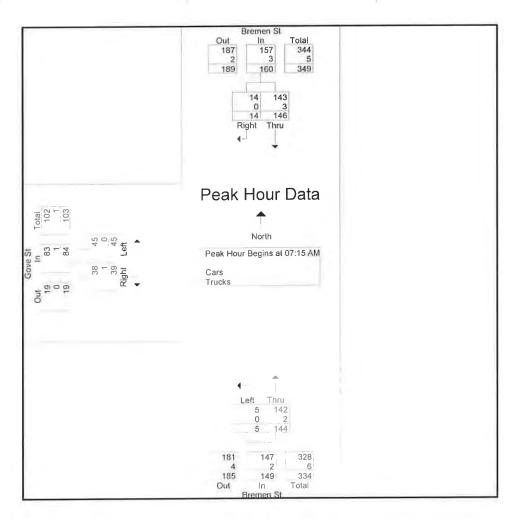
Groups Printed- Cars - Trucks

		Gove St From West		Bremen St From South		Bremen St	
Int. Tota	Right	Left	Thru	Left	Right	From North Thru	Start Time
82	5	4	33	1	3	36	07:00 AM
98	8	8	40	3	6	33	07:15 AM
90	9	13	34	1	3	30	07:30 AM
113	10	13	35	0	4	51	07:45 AM
383	32	38	142	5	16	150	Total
92	12	11	35	1	1	32	08:00 AM
9	7	8	37	0	7	32	08:15 AM
7	3	7	38	1	2	24	08:30 AM
62	3	5	25	1	3	25	08:45 AM
320	25	31	135	3	13	113	Total
70:	57	69	277	8	29	263	Grand Total
	45.2	54.8	97.2	2.8	9.9	90.1	Apprch %
	8.1	9.8	39.4	1.1	4.1	37.4	Total %
68	56	69	273	8	29	254	Cars
98	98.2	100	98.6	100	100	96.6	% Cars
14	1	0	4	0	0	9	Trucks
-:	1.8	0	1.4	0	0	3_4	% Trucks

N/S Street: Bremen Street E/W Street: Gove Street City/State : Boston, MA Weather : Clear

File Name : 75860003 Site Code : 75860003 Start Date : 1/25/2018 Page No : 2

		Bremen St From North			Bremen St rom South			Gove St From West		
Start Time	Thru	Right	App. Total	Left	Thru	App. Total	Left	Right	App. Total	Int. Total
Peak Hour Analysis From 07	7:00 AM to 0	8:45 AM - F	Peak 1 of 1			411				
Peak Hour for Entire Interse	ction Begins	at 07:15 A	M							
07:15 AM	33	6	39	3	40	43	8	8	16	98
07:30 AM	30	3	33	1	34	35	13	9	22	90
07:45 AM	51	4	55	0	35	35	13	10	23	113
08:00 AM	32	1	33	1	35	36	11	12	23	92
Total Volume	146	14	160	5	144	149	45	39	84	393
% App. Total	91.2	8.8		3.4	96.6		53.6	46.4		
PHF	.716	.583	.727	.417	.900	-866	.865	.813	.913	,869
Cars	143	14	157	5	142	147	45	38	83	387
% Cars	97.9	100	98.1	100	98.6	98.7	100	97.4	98.8	98.5
Trucks	3	0	3	0	2	2	0	1	1	6
% Trucks	2.1	0	1.9	0	1.4	1.3	0	2.6	1.2	1.5



N/S Street : Bremen Street E/W Street : Gove Street City/State : Boston, MA Weather : Clear

File Name: 75860003 Site Code: 75860003 Start Date: 1/25/2018 Page No: 4

Groups	Printed-	Cars
and a colonia and a later of		0.

	Bremen S From North	t n	Bremen St From South	h	Gove St From Wes	t	
Start Time	Thru	Right	Left	Thru	Left	Right	Int. Total
07:00 AM	34	3	1	33	4	5	80
07:15 AM	33	6	3	39	8	8	97
07:30 AM	29	3	1	34	13	9	89
07:45 AM	50	4	0	35	13	10	112
Total	146	16	5	141	38	32	378
08:00 AM	31	1	1	34	11	11	89
08:15 AM	30	7	0	36	8	7	88
08:30 AM	24	2	1	37	7	3	74
08:45 AM	23	3	1	25	5	3	60
Total	108	13	3	132	31	24	311
Grand Total	254	29	8	273	69	56	689
Apprch %	89.8	10.2	2.8	97.2	55.2	44.8	
Total %	36.9	4.2	1.2	39.6	10	8.1	

N/S Street : Bremen Street E/W Street : Gove Street City/State : Boston, MA Weather : Clear

File Name : 75860003 Site Code : 75860003 Start Date : 1/25/2018 Page No : 7

		Gove St		s Printed- Trucks Bremen St		Bremen St	
		From West		From South		From North	
Int. Total	Right	Left	Thru	Left	Right	Thru	Start Time
2	0	0	0	0	0	2	07:00 AM
1	0	0	1	0	0	0	07:15 AM
1	0	0	0	0	0	1	07:30 AM
1	0	0	0	0	0	1	07:45 AM
5	0	0	1	0	0	4	Total
•	aul	927	a)		Ÿ		1
3	1	0	1	0	0	1	08:00 AM
3	0	0	1	0	0	2	08:15 AM
1	0	0	1	0	0	0	08:30 AM
2	0	0	0	0	0	2	08:45 AM
9	1	0	3	0	0	5	Total
14	1	0	4	0	0	9	Crand Tatal
	100	0			1		Grand Total
		-	100	0	0	100	Apprch %
	7.1	0	28.6	0	0	64.3	Total %

N/S Street : Bremen Street E/W Street : Gove Street City/State : Boston, MA Weather : Clear

File Name : 75860003 Site Code : 75860003 Start Date : 1/25/2018 Page No : 10

Groups Printed-	Bikes	Peds	
Draman Ct	20,000		

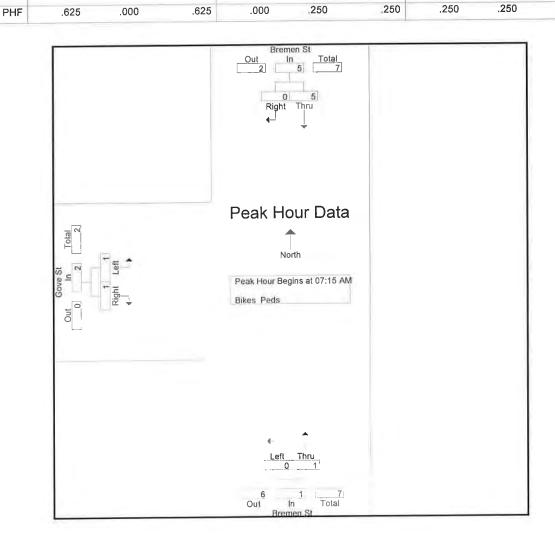
		remen St om North		Br	emen St om South			Gove St rom West				
Start Time	Thru	Right	Peds	Left	Thru	Peds	Left	Right	Peds	Exclu. Total	Inclu. Total	Int. Total
07:00 AM	0	0	14	0	0	7	0	0	7	28	0	28
07:15 AM	1	0	19	0	0	7	1	0	5	31	2	33
07:30 AM	1	0	19	0	0	3	0	0	10	32	1	33
07:45 AM	1	0	44	0	0	6	0	1	1	51	2	53
Total	3	0	96	0	0	23	1	1	23	142	5	147
08:00 AM	2	0	44	0	1	2	0	0	1	47	3	50
08:15 AM	0	0	30	0	2	7	0	0	0	37	2	39
08:30 AM	0	0	29	0	0	1	0	0	11	41	0	4
08:45 AM	0	0	14	0	0	2	0	0	5	21	0	2
Total	2	0	117	0	3	12	0	0	17	146	5	15
Grand Total	5	0	213	0	3	35	1	1	40	288	10	298
Apprch %	100	0		0	100		50	50				
Total %	50	0		0	30		10	10		96.6	3.4	

N/S Street : Bremen Street E/W Street : Gove Street City/State : Boston, MA Weather : Clear File Name : 75860003 Site Code : 75860003 Start Date : 1/25/2018 Page No : 11

.500

.667

		Bremen St From North			Bremen St From South		F	Gove St From West		
Start Time	Thru	Right	App. Total	Left	Thru	App. Total	Left	Right	App. Total	Int. Total
eak Hour Analysis From 0	7:00 AM to 0	8:45 AM - F	Peak 1 of 1			1				
eak Hour for Entire Interse	ction Begins	at 07:15 Al	М							
07:15 AM	1	0	1	0	0	0	1	0	1	2
07:30 AM	1	0	1	0	0	0	0	0	0	1
07:45 AM	1	0	1	0	0	0	0	1	1	2
08:00 AM	2	0	2	0	1	1	0	0	0	3
Total Volume	5	0	5	0	1	1	1	1	2	8
% App. Total	100	0		0	100		50	50		



N/S Street: Bremen Street E/W Street: Gove Street City/State : Boston, MA Weather : Clear

File Name : 75860003 Site Code : 75860003 Start Date : 1/25/2018 Page No : 1

0

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0.4

	Gove St From West		rinted- Cars - Trucks Bremen St From South		Bremen St From North			
Int. Total	Right	Left	Thru	Left	Right	Thru	Start Time	
83	7	9	33	0	4	30	04:00 PM	
83	6	7	35	0	4	31	04:15 PM	
96	7	12	41	2	2	32	04:30 PM	
96	9	10	45	0	2	30	04:45 PM	
358	29	38	154	2	12	123	Total	
108	13	7	49	0	4	35	05:00 PM	
88	7	9	33	2	4	33	05:15 PM	
94	6	7	39	0	3	39	05:30 PM	
70	9	4	25	0	2	30	05:45 PM	
360	35	27	146	2	13	137	Total	
718	64	65	300	4	25	260	Grand Total	
	49.6	50.4	98.7	1.3	8.8	91.2	Apprch %	
	8.9	9.1	41.8	0.6	3.5	36.2	Total %	
715	64	64	300	4	25	258	Cars	
99.6	100	98.5	100	100	100	99.2	% Cars	

0

0

0

0

2

0.8

Trucks

% Trucks

0

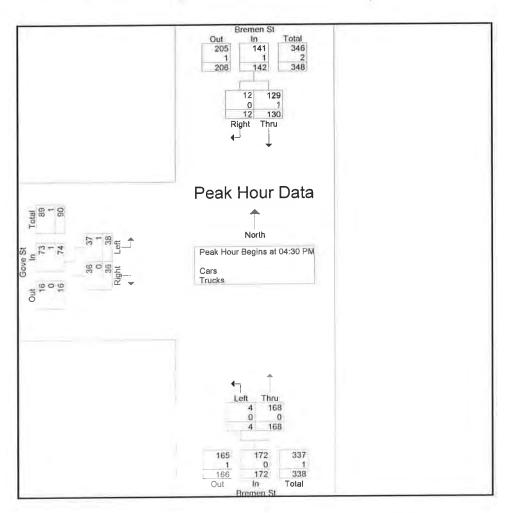
0

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1.5

N/S Street: Bremen Street E/W Street: Gove Street City/State : Boston, MA Weather : Clear

		Bremen St From North			Bremen St From South			Gove St From West		
Start Time	Thru	Right	App. Total	Left	Thru	App. Total	Left	Right	App. Total	Int. Total
Peak Hour Analysis From 04	4:00 PM to 0	5:45 PM - F	Peak 1 of 1							
Peak Hour for Entire Interse	ction Begins	at 04:30 Pf	M							
04:30 PM	32	2	34	2	41	43	12	7	19	96
04:45 PM	30	2	32	0	45	45	10	9	19	96
05:00 PM	35	4	39	0	49	49	7	13	20	108
05:15 PM	33	4	37	2	33	35	9	7	16	88
Total Volume	130	12	142	4	168	172	38	36	74	388
% App. Total	91.5	8.5		2.3	97.7		51.4	48.6		
PHF	.929	.750	.910	.500	.857	878	,792	.692	.925	_898
Cars	129	12	141	4	168	172	37	36	73	386
% Cars	99.2	100	99.3	100	100	100	97.4	100	98.6	99.5
Trucks	1	0	1	0	0	0	1	0	1	2
% Trucks	0.8	0	0.7	0	0	0	2.6	0	1.4	0.5



N/S Street : Bremen Street E/W Street : Gove Street City/State : Boston, MA Weather : Clear

West transfer of	and the same	1-12-14C	CO COLORS	
Groups	Prin	ed-	Cars	

	Bremen St From North		Bremen St From South	1	Gove St From Wes	t	
Start Time	Thru	Right	Left I	Thru	Left	Right	Int. Total
04:00 PM	30	4	0	33	9	7	83
04:15 PM	31	4	0	35	7	6	83
04:30 PM	32	2	2	41	12	7	96
04:45 PM	30	2	0	45	10	9	96
Total	123	12	2	154	38	29	358
05:00 PM	35	4	0	49	7	13	108
05:15 PM	32	4	2	33	8	7	86
05:30 PM	38	3	0	39	7	6	93
05:45 PM	30	2	0	25	4	9	70
Total	135	13	2	146	26	35	357
Grand Total	258	25	4	300	64	64	715
Apprch %	91.2	8.8	1.3	98.7	50	50	
Total %	36.1	3.5	0.6	42	9	9	

N/S Street : Bremen Street E/W Street : Gove Street City/State : Boston, MA Weather : Clear

		Grou	os Printed- Trucks				
	Bremen St		Bremen St		Gove St From West		
Start Time	From North Thru	Right	From South Left	Thru	Left	Right	Int. Total
04:00 PM	0	0	0	0	0	0	0
04:15 PM	0	0	0	0	0	0	0
04:30 PM	0	0	0	0	0	0	0
04:45 PM	0	0	0	0	0	0	0
Total	0	0	0	0	0	0	0
05:00 PM	0	o	0	0	0	0	0
05:15 PM	1	0	0	0	1	0	2
05:30 PM	1	0	0	0	0	0	1
05:45 PM	0	О	0	0	0	0	0
Total	2	0	0	0	1	0	3
Grand Total	2	0	0	0	1	0	3
Apprch %	100	0	0	0	100	0	
Total %	66.7	О	0	0	33.3	0	

N/S Street: Bremen Street E/W Street: Gove Street City/State : Boston, MA Weather : Clear

Groups	Printed-	Bikes	Peds	

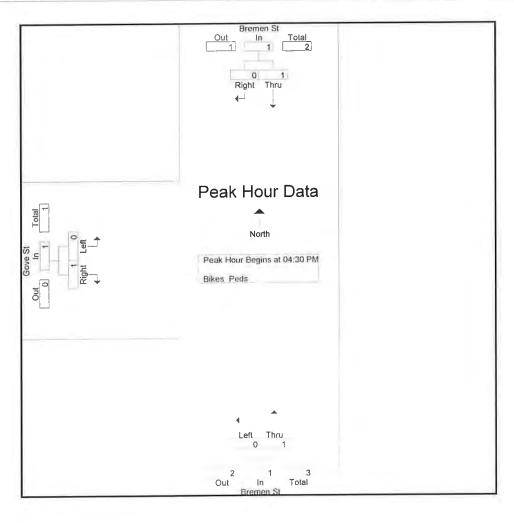
		remen St om North		Br	emen St om South			Gove St rom West				
Start Time	Thru	Right	Peds	Left	Thru	Peds	Left	Right	Peds		Inclu. Total	Int. Tota
04:00 PM	0	0	21	0	1	8	0	0	5	34	1	35
04:15 PM	0	0	13	0	0	3	0	0	3	19	0	19
04:30 PM	0	0	23	0	0	5	0	0	5	33	0	33
04:45 PM	0	0	25	0	0	6	0	0	5	36	0	36
Total	0	0	82	0	1	22	0	0	18	122	1	123
05:00 PM	0	0	18	0	1	8	0	0	1	27	1	28
05:15 PM	1	0	31	0	0	4	0	1	5	40	2	42
05:30 PM	0	0	15	0	0	0	0	0	4	19	0	19
05:45 PM	0	0	17	0	0	5	0	0	8	30	0	3
Total	1:	0	81	0	1	17	0	1	18	116	3	119
Grand Total	1	0	163	0	2	39	0	1	36	238	4	242
Apprch %	100	0		0	100		0	100				
Total %	25	0		0	50		0	25		98.3	1.7	

N/S Street : Bremen Street E/W Street : Gove Street City/State : Boston, MA Weather : Clear

File Name : 75860003 Site Code : 75860003 St

Start Date	: 1/25/201
age No	: 11

		Bremen St From North			Bremen St rom South			Gove St From West		
Start Time	Thru	Right	App. Total	Left	Thru	App. Total	Left	Right	App. Total	Int. Tota
eak Hour Analysis From 04	1:00 PM to 0	5:45 PM - F	Peak 1 of 1							
eak Hour for Entire Interse	ction Begins	at 04:30 P	M							
04:30 PM	0	0	0	0	0	0	0	0	0	(
04:45 PM	0	0	0	0	0	0	0	0	0	(
05:00 PM	0	0	0	0	1	1	0	0	0	•
05:15 PM	1	0	1	0	0	0	0	1	1	2
Total Volume	1	0	1	0	1	1	0	1	1	3
% App. Total	100	0		0	100		0	100		
PHF	.250	.000	250	.000	.250	.250	.000	.250	.250	.375



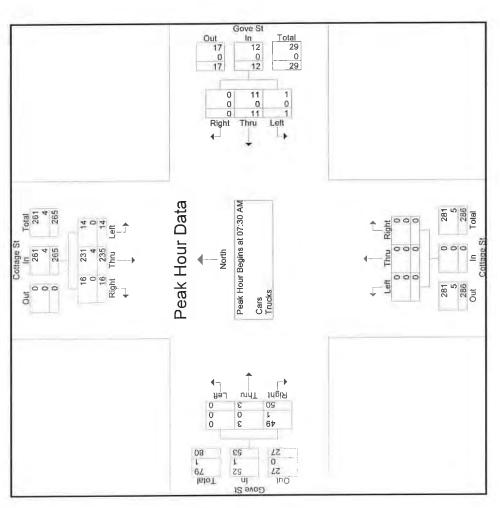
File Name 75860004 Site Code 75860004 Start Date 1/25/2018 Page No 1

	ပို့	Cottage St		വ [Gove St	e St		Cottage St			Gove St		
Start Time	Left	From North Thru	Right	Left	From East Thru	Right	Leff	Thru	Right	Left	From west	Right	Int. Total
07:00 AM		36	9	-	4	0	0	0	0	0	0	2	52
07:15 AM	2	31	5	_	2	0	0	0	0	0	ည	6	58
07:30 AM	ю	43	က	0	9	0	0	0	0	0	_	6	65
07:45 AM	4	64	5	←	-	0	0	0	0	0	0	10	85
Total	12	174	19	м	16	0	0	0	0	0	ω	30	260
08:00 AM	4	29	-	0	4	0	0	0	0	0	0	41	06
08:15 AM	က	61	7	0	0	0	0	0	0	0	2	17	06
08:30 AM	~	15	Ŋ	0	ю	0	0	0	0	0	0	4	28
08:45 AM	က	17	4	~	4	0	0	0	0	0	0	2	28
Total	-	160	17	-	ω	0	0	0	0	0	2	37	236
Grand Total	23	334	36	4	24	0	0	0	0	0	ю	29	496
Apprch %	5.9	85	9.5	14.3	85.7	0	0	0	0	0	10.7	89.3	
Total %	4.6	67.3	7.3	8.0	4.8	0	0	0	0	0	1.6	13.5	
Cars	23	326	36	4	24	0	0	0	0	0	∞	99	487
% Cars	100	97.6	100	100	100	0	0	0	0	0	100	98.5	98.2
Trucks	0	00	0	0	0	0	0	0	0	0	0	**	6
% Trucks	0	2.4	0	0	0	0	0	0	0	0	0	1.5	1.8

File Name : 75860004 Site Code : 75860004 Start Date : 1/25/2018 Page No : 2

		Cottage St From North	e St Iorth			Gove St From East	e St East			Cottage St From South	ge St South			Gove St From West	e St West		
Start Time L	Left	Thru	Right	Thru , Right , App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1	17:00 AM	1 to 08:45	3 AM - Pe	ak 1 of 1													
Peak Hour for Entire Intersection Begins at 07:30 AM	ection Be	egins at 0	7:30 AM														
07:30 AM	က	43	က	49	0	9	0	g	0	0	0	0	0	_	6	10	65
07:45 AM	4	64	5	73	~	-	0	2	0	0	0	0	0	0	10	10	85
08.00 AM	4	29	-	72	0	4	0	4	0	0	0	0	0	0	14	14	90
08:15 AM	ന	61	_	7.1	0	0	0	0	0	0	0	0	0	2	17	19	06
Total Volume	14	235	16	265	-	1	0	12	0	0	0	0	0	ო	20	53	330
% App. Total	5.3	88.7	9		8.3	91.7	0		0	0	0		0	5.7	94.3		
PHF .	.875	.877	.571	806.	.250	.458	000	.500	000	0000	000.	000.	000.	.375	.735	269.	.917
Cars	4	231	16	261	-	£	0	12	0	0	0	0	0	m	49	52	325
% Cars	100	98.3	100	98.5	100	100	0	100	0	0	0	0	0	100	98.0	98.1	98.5
Trucks	0	4	0	4	0	0	0	0	0	0	0	0	0	0	_	~	5
% Trucks	C	1.7	C	<u>т</u>	0	0	0	0	0	0	0	0	0	0	2.0	1.9	1.5

File Name 75860004 Site Code 75860004 Start Date 1/25/2018 Page No 3



Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1

gins at:
ргоасћ Ве
r Each Ap
eak Hour fo
X

	07:30 AM			0	07-00 AM			0.	07:00 AM			ö	07:30 AM			
+0 mins.	က	43	ო	49	-	4	0	ις	0	0	0	0	0	~	6	10
+15 mins.	4	64	5	73	-	2	0	9	0	0	0	0	0	0	10	10
+30 mins.	4	29	-	72	0	9	0	9	0	0	0	0	0	0	4	14
+45 mins.	6	61	7	7.1	-	-	0	2	0	0	0	0	0	2	17	19
otal Volume	14	235	16	265	က	16	0	19	0	0	0	0	0	က	50	53
% App. Total	5.3	88.7	9		15.8	84.2	0		0	0	0		0	5.7	94.3	

File Name : 75860004 Site Code : 75860004 Start Date : 1/25/2018 Page No : 5

						Groups Printed- Cars							
Thrui Right Left Thru Right Left Thru Right Left 33 6 1 4 0		Cottage St rom North			Gove St From East			offage St		Ī	Gove St From West		
33 6 1 4 0	Left	Thru	Right		Thru	Right	-	Thru	Right		Thru	Right	Int. Total
30 5 1 5 0	က	33	9	÷	4	0	0	0	0	0	0	2	49
43 3 0 6 0	2	30	Ŋ	÷	2	0	0	0	0	0	S	o,	22
63 5 1 1 0 0 0 0 0 0 169 19 3 16 0 0 0 0 0 60 7 0 4 0 0 0 0 0 15 5 0 3 0 0 0 0 0 17 4 1 1 1 0 0 0 0 0 157 17 1 8 0 0 0 0 0 0 84.7 9.4 14.3 85.7 0 0 0 0 0 66.9 7.4 0.8 4.9 0 0 0 0 0 0	က	43	ო	0	9	0	0	0	0	0	-	60	64
169 19 3 16 0 0 0 0 0 0 0 0 65 7 0 4 0 0 0 0 0 0 15 5 0 3 0 0 0 0 0 0 17 4 1 1 1 0 0 0 0 0 0 326 36 4 24 0 0 0 0 0 0 0 0 0 84.7 9.4 14.3 85.7 0	4	63	5	4-	~	0	0	0	0	0	0	10	84
65 1 0 4 0 0 0 0 0 60 7 0 0 0 0 0 0 15 5 0 3 0 0 0 0 17 4 1 1 1 0 0 0 0 157 17 1 8 0 0 0 0 0 84.7 9.4 14.3 85.7 0 0 0 0 0 66.9 7.4 0.8 4.9 0 0 0 0 0	12	169	6	m	16	0	0	0	0	0	O	29	254
3 60 7 0 0 0 0 0 0 0 1 15 5 0 3 0 0 0 0 0 3 17 4 1 1 0 0 0 0 0 11 157 17 1 8 0 0 0 0 0 23 326 36 4 24 0 0 0 0 0 0 4.7 66.9 7.4 0.8 4.9 0 0 0 0 0 0	4	65	_	0	4	0	0	0	0	0	0	4	88
15 5 0 3 0 0 0 0 0 17 4 1 1 0 0 0 0 0 157 17 1 8 0 0 0 0 0 326 36 4 24 0 0 0 0 0 0 66.9 7.4 0.8 4.9 0 0 0 0 0 0 0	က	09	7	0	0	0	0	0	0	0	2	17	88
3 17 4 1 1 0 0 0 0 0 11 157 17 1 8 0 0 0 0 0 23 326 36 4 24 0 0 0 0 0 6 84.7 9.4 14.3 85.7 0 0 0 0 0 4.7 66.9 7.4 0.8 4.9 0 0 0 0 0	-	15	ď	0	ო	0	0	0	0	0	0	4	28
11 157 17 1 8 0 0 0 0 23 326 36 4 24 0 0 0 0 0 6 84.7 9.4 14.3 85.7 0 0 0 0 4.7 66.9 7.4 0.8 4.9 0 0 0 0	က	17	4	←	~	0	0	0	0	0	0	2	28
23 326 36 4 24 0 0 0 0 6 84.7 9.4 14.3 85.7 0 0 0 0 4.7 66.9 7.4 0.8 4.9 0 0 0 0	: 1	157	17	-	∞	0	0	0	0	0	2	37	233
4.7 66.9 7.4 0.8 4.9 0 0 0 0 0	23	326	36	4 14.3	24 85.7	0	0	0	0	0	8 10.8	66	487
	4.7	6.99	7.4	0.8	4.9	0	0	0	0	0	1.6	13.6	

File Name : 75860004 Site Code : 75860004 Start Date : 1/25/2018 Page No : 9

Start Time						Group	Groups Printed- Trucks	UCKS						
Left Thru Right Left Thru Right Left Thru Right Left Thru 0 1 0		O IL	Cottage St		L.	Gove St rom East			ottage St om South		ű,	Gove St rom West		
	Start Time		Thru	Right		Thru	Right		Thru	Right	-	Thru	Right	Int. Total
	07:00 AM	0	က	0	0	0	0	0	0	0	0	0	0	က
	07:15 AM	0	~	0	0	0	0	0	0	0	0	0	0	_
0	07:30 AM	0	0	0	0	0	0	0	0	0	0	0	_	4
0 5 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	07:45 AM	0	~	0	0	0	0	0	0	0	0	0	0	
0 2 0	Total	0	5	0	0	0	0	0	0	0	0	0	+	9
0 1 0	08:00 AM	0	2	0	0	0	0	0	0	0	0	0	0	2
	08:15 AM	0	-	0	0	0	0	0	0	0	0	0	0	·
	08:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	Ü
0	08:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
0 8 0 0 0 0 0 0 0 0 100 0 0 0 0 0 0 0 0 88.9 0 0 0 0 0 0 0 0	Total	0	ю	0	0	0	0	0	0	0	0	0	0	က
0 0 0 0 0 0 0 0 0	Srand Total Apprch %	0 0	8 100	0 0	0 0	0 0	0	0 0	0 0	0 0	0 0	0	100	
	Total %	0	88.9	0	0	0	0	0	0	0	0	0	11.1	

File Name : 75860004 Site Code : 75860004 Start Date : 1/25/2018 Page No : 13

AN OFFICE COME OFFICE	Boston, MA	Clear
. 100 00 00.	City/State :	Veather :

								Group	Groups Printed- Bikes Peds	Sikes Pec	IS			0					
		Cottage St From North	s St			Gove St From Fast	St			Cottage St From South	ಪ್			Gove St From West	St est				
Start Time	Left	Thru	Right	Peds	Left	Thru	Right	Peds	Left	Thru	Right	Peds	Left	Thru	Right	Peds	Exclu. Total	Exclu. Total Inclu. Total	Int. Total
07:00 AM	0	0	0	2	0	0	0	0	0	0	0	~	0	0	0	4	7	0 2	7
07:15 AM	0	0	0	2	0	0	0	2	0	0	0	~	0	0	0	7	θ	0 9	9
07 30 AM	0	0	0	က	0	0	0	0	0	0	0	0	0	0	0	ю	9	0	9
07:45 AM	0	0	0	7	0	0	0	2	0	0	0	0	0	0	0	19	28	0	28
Total	0	0	0	41	0	o	0	4	0	0	0	2	0	0	0	27	47	0 2	47
08:00 AM	0	0	0	5	0	0	0	က	0	0	0	9	0	0	0	4	28	0	28
08 15 AM	0	0	0	7	0	0	0	4	0	0	0	2	0	0	0	26	42	2	42
08:30 AM	0	0	0	Ŋ	0	0	0	2	0	0	0	e	0	0	0	4	14	4 0	14
08-45 AM	0	0	0	0	0	0	0	2	0	0	0	0	0	0	0	41	16	0 9	16
Total	0	0	0	17	0	O	0	=	0	0	0	4	0	0	0	28	100	0	100
Grand Total	0	0	0	31	0	0	0	15	0	0	0	16	0	0	0	85	147	0 2	147
Apprch % Total %	0	0	0		0	0	0		0	0	0		0	0	0		100	0	

File Name : 75860004 Site Code : 75860004 Start Date : 1/25/2018 Page No : 14

N/S Street: Cottage Street E/W Street: Gove Street City/State: Boston, MA Weather: Clear

Start Time Left Thru Right App. Total Int. Total Volume Peak Hour Analysis From 07:00 AM 0			Cotts	Cottage St			Gove St	e St			Cotta	Cottage St			Go.	Gove St		
I Left Thru Right App. Total Thru Right App. Total Left Thru Right App. Total App. Total 0 </th <th></th> <th></th> <th>From</th> <th>North</th> <th></th> <th></th> <th>From</th> <th>East</th> <th></th> <th></th> <th>From</th> <th>South</th> <th></th> <th></th> <th>From</th> <th>West</th> <th></th> <th></th>			From	North			From	East			From	South			From	West		
	Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left		Right	App. Total	Left	Thru	Right	App. Total	Int. Total
	k Hour Analysis Fro	om 07:00	AM to 08:	45 AM - P	eak 1 of 1													
	k Hour for Entire Int	tersection	າ Begins a	t 07:00 AN	~													
	07:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
	07:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
	07:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
	07:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
% App. Total 0 0 0 0 0 0 0 0 0 0 0 0 0	Total Volume	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
	% App. Total	0	0	0		0	0	0		0	0	0		0	0	0		

000.

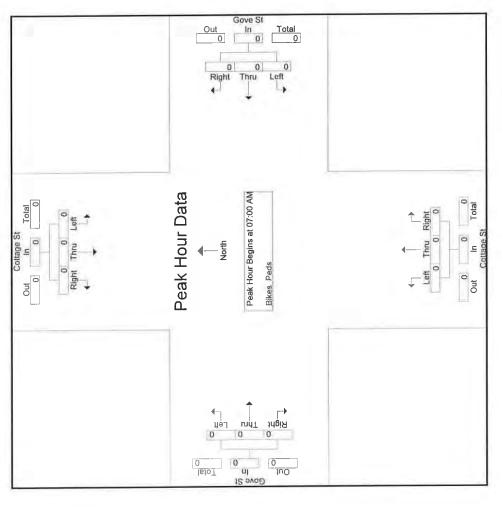
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File Name : 75860004 Site Code : 75860004 Start Date : 1/25/2018 Page No : 15

N/S Street: Cottage Street E/W Street: Gove Street City/State: Boston, MA Weather: Clear



Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1

Peak Hour for Each Approach Begins at:

.70	07:00 AM			07	07:00 AM			0.0	07:00 AM			0	7.00 AM			
+0 mins.	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
+15 mins.	0	0	0	0	0	0	0	0	0	0	0	0	0 0	0	0	0
+30 mins.	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
+45 mins.	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Total Volume	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
% App. Total	0	0	0		0	0	0		0	0	0		0	0	0	

File Name : 75860004 Site Code : 75860004 Start Date : 1/25/2018 Page No : 1

N/S Street: Cottage Street E/W Street: Gove Street City/State: Boston, MA Weather: Clear

	Left Fro	From North		1			, L			L	10000		
	iei.		- dei O	- Fro	From East	+4±i0	Pro	From South	Dight	#0	From West	Bight	Int Total
04:00 PM 04:15 PM 04:30 PM Total 05:00 PM		2	luği.	ם ט	niin	TIBLY I	רמו	n i	III (רפוו	0	TIPE CO.	10101
04:30 PM 04:30 PM 04:45 PM Total 05:00 PM	-	32	4	0	2	0	0	D	0	0	0	-	40
04:30 PM 04:45 PM Total 05:00 PM	-	37	က	2	4	0	0	0	0	0	~	2	55
04:45 PM Total 05:00 PM	е	37	g	-	0	0	0	0	0	0	0	7	54
Total 05:00 PM	7	42	5	-	8	0	0	0	0	0	~	က	62
05:00 PM	12	148	8	4	o	0	0	0	0	0	2	18	211
05:00 PM													
	7	39	9	_	7	0	0	0	0	0	-	4	09
05:15 PM	က	27	ĸ	0	4	0	0	0	0	0	2	7	48
05:30 PM	က	32	9	0	_	0	0	0	0	0	τ	4	47
05:45 PM	ю	35	4	-	က	0	0	0	0	0	0	2	48
Total	16	133	21	2	10	0	0	0	0	0	4	17	203
Grand Total	28	281	39	9	19	0	0	0	0	0	9	35	414
Apprch %	œ	80.7	11.2	24	92	0	0	0	0	0	14.6	85.4	
Total %	8.9	67.9	9.4	1.4	4.6	0	0	0	0	0	1.4	8.5	
Cars	28	280	39	9	19	0	0	0	0	0	9	34	412
% Cars	100	9.66	100	100	100	0	0	0	0	0	100	97.1	99.5
Trucks	0	-	0	0	0	0	0	0	0	0	0	-	2
% Trucks	0	9.0	0	0	0	0	0	0	0	0	0	2.9	0.5

File Name : 75860004 Site Code : 75860004 Start Date : 1/25/2018 Page No : 2

From West Gove St

N/S Street: Cottage Street E/W Street: Gove Street City/State: Boston, MA Weather: Clear

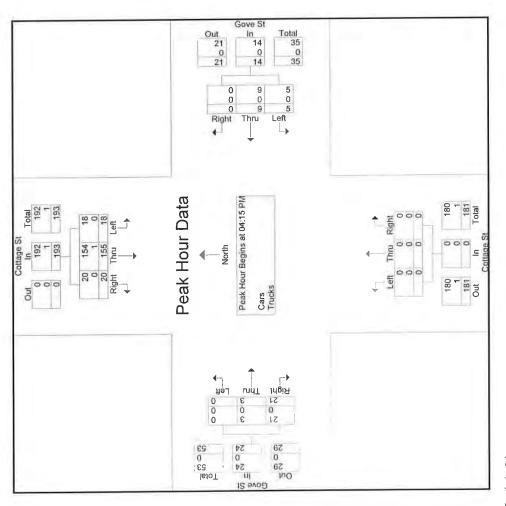
From South Cottage St From East Gove St From North Cottage St Peak Hour

5	
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במה ויטטו להיטט טו ועו ויטליטט וועו ויטלומון אמין ויטטו ויסלומון אמין ויסלו	Peak Hour for Entire Intersection Begins at 04:15 PM
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2	- 5
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Start Time	Left	Thru	Right	Thru Right App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1	om 04:00 F	PM to 05:4	15 PM - Pe	sak 1 of 1													
Hour for Entire Intersection Begins at 04:15 PM	ntersection	Begins at	04:15 PM	_1													
04:15 PM	~	37	က	41	2	4	0	9	0	0	0	0	0	-	7	00	22
04:30 PM	က	37	9	46	-	0	0	-	0	0	0	0	0	0	7	2	24
04:45 PM	7	42	2	54	_	ო	0	4	0	0	0	0	0	-	က	4	62
05:00 PM	7	39	9	52	τ-	2	0	က	0	0	0	0	0	-	4	വ	09
Total Volume	18	155	20	193	5	o	0	4	0	0	0	0	0	က	21	24	231
% App. Total	6.3	80.3	10.4		35.7	64.3	0		0	0	0		0	12.5	87.5		
PHF	.643	.923	.833	894	.625	.563	000°	.583	000	000	000	000.	000.	.750	.750	.750	.931
Cars	18	154	20	192	2	6	0	14	0	0	0	0	0	က	21	24	230
% Cars	100	99.4	100	99.5	100	100	0	100	0	0	0	0	0	100	100	100	9.66
Trucks	0	_	0	_	0	0	0	0	0	0	0	0	0	0	0	0	-
% Trucks	0	9.0	0	0.5	0	0	0	0	0	0	0	0	0	0	0	0	0.4

File Name 75860004 Site Code 75860004 Start Date 1/25/2018 Page No 3

N/S Street Cottage Street E/W Street Gove Street City/State Boston, MA Weather Clear



Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1

Peak Hour for Each Approach Begins at:

	04:15 PM			Ç	04:15 PM			J	04:00 PM			04	04:30 PM			
+0 mins.	-	37	က	41	8	4	0	9	0	0	0	0	0	0	7	7
+15 mins.	ന	37	9	46	_	0	0	-	0	0	0	0	0	-	က	4
+30 mins.	7	42	2	54	-	က	0	4	0	0	0	0	0	_	4	Ω
+45 mins.	7	39	9	25	-	2	0	က	0	0	0	0	0	2	7	6
otal Volume	18	155	20	193	ις	0	0	41	0	0	0	0	0	4	21	25
% App. Total	9.3	80.3	10.4		35.7	64.3	0		0	0	0		0	16	84	

File Name : 75860004 Site Code : 75860004 Start Date : 1/25/2018 Page No : 5

					Groun	Groups Printed- Cars	Cars						
		Cottage St			Gove St			Cottage St			Gove St		
		-rom North		u.	From East			From South		-	From West		
Start Time	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Int. Total
04:00 PM	+	32	4	0	2	0	0	0	0	0	0	~	40
04:15 PM	٠	36	က	2	4	0	0	0	0	0	₹	7	54
04:30 PM	8	37	9	~	0	0	0	0	0	0	0	7	54
04:45 PM	7	42	S	-	ო	0	0	0	0	0	~	ന	62
Total	12	147	4	4	o	0	0	0	0	0	2	92	210
05:00 PM	7	39	9	-	2	0	0	0	0	0	~	4	09
05:15 PM	က	27	ß	0	4	0	0	0	0	0	2	7	48
05:30 PM	က	32	9	0		0	0	0	0	0	-	m	46
05:45 PM	т	35	4	-	က	0	0	0	0	0	0	2	48
Total	16	133	21	2	10	0	0	0	0	0	4	16	202
Grand Total	28	280	39	9	19	0	0	0	0	0	9	34	412
Apprch %	8.1	80.7	11.2	24	92	0	0	0	0	0	15	85	
Total %	6.8	89	9.5	1.5	4.6	0	0	0	0	0	1.5	8.3	

File Name 75860004 Site Code 75860004 Start Date 1/25/2018 Page No 9

Int. Total

Right

N/S Street: Cottage Street E/W Street: Gove Street City/State: Boston, MA Weather: Clear

Left 0	From North		n Ę	Gove St From East	ot ISt	O [Cottage St From South		Ē	Gove St From West
0 0	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru
0	0	0	0	0	0	0	0	0	0	0
	~	0	0	0	0	0	0	0	0	0
04:30 PM	0	0	0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0	0	0	0
Total 0	-	0	0	0	0	0	0	0	0	0
05:00 PM 0	0	0	0	0	0	0	0	0	0	0
05:15 PM 0	0	0	0	0	0	0	0	0	0	0
05:30 PM 0	0	0	0	0	0	0	0	0	0	0
05:45 PM 0	0	0	0	0	0	0	0	0	0	0
Total 0	0	0	0	0	0	0	0	0	0	0
Grand Total 0	7	0	0	0	0	0	0	0	0	0
Apprch % 0	100	0	0	0	0	0	0	0	0	0
Total % 0	50	0	0	0	0	0	0	0	0	0

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File Name | 75860004 Site Code | 75860004 Start Date | 1/25/2018 Page No | 13

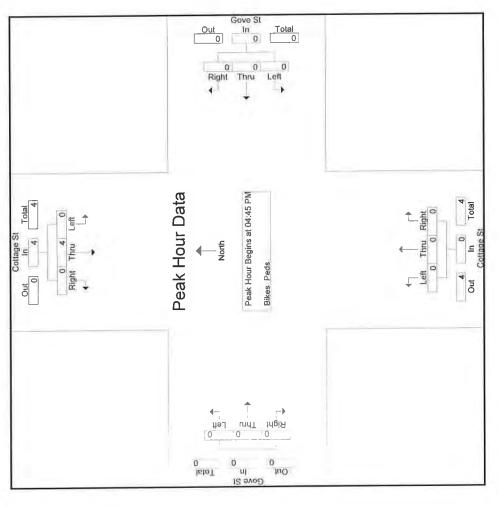
		al Int. Total	0 11	0 14	0 22	0 23	0 40	1 28	2 31	1 15	0 15	4 89	4 159		ı
		Inclu, Tota													
		Exclu, Total Inclu, Total	1	14	22	23	70	27	29	14	15	82	155		
		Peds	c)	ю	13	13	34	4	19	5	7	45	79		
	St Vest	Right	0	0	0	0	0	0	0	0	0	0	0	0	
	Gove St From West	Thru	0	0	0	0	0	0	0	0	0	0	0	0	
		Left	0	0	0	0	0	0	0	0	0	0	0	0	
		Peds	-	-	~	2	r.	22	ភេ	7	-	13	18		
S	ಕ್ಷ	Right	0	0	0	0	0	0	0	0	0	0	0	0	
sikes Pec	Cottage St From South	Thru	0	0	0	0	0	0	0	0	0	0	0	0	
Groups Printed- Bikes Peds		Left	0	0	0	0	0	0	0	0	0	0	0	0	
		Peds	+	ιΩ	-	4	Ξ	0	۳	2	2	5	16		
	Gove St From East	Right	0	0	0	0	0	0	0	0	0	0	0	0	
	Gove S From Es	Thru	0	0	0	0	0	0	0	0	0	0	0	0	
		Left	0	0	0	0	0	0	0	0	0	0	0	0	
		Peds	4	r.	7	4	20	00	4	2	2	22	42		
	ts ∓	ht	0	0	0	0	0	0	0	0	0	: 0	0	0	
	Cottage St From North	Thru	0	0	0	0	0	~	2	-	0	4	4	100	
		Left	0	0	0	0	0	0	0	0	0	0	0	0	
		Start Time	04:00 PM	04 15 PM	04:30 PM	04.45 PM	Total	05:00 PM	05:15 PM	05:30 PM	05:45 PM	Total	Grand Total	Apprch %	

File Name : 75860004 Site Code : 75860004 Start Date : 1/25/2018 Page No : 14

Left Thru Right App. Total Int.			Cottage St	le St			Gove St	St			Cotta	Cottage St			Gov	Gove St		
From 04:00 PM to 05:45 PM - Peak 1 of 1 Left			From N	Vorth			From	East			From (South			From	West		
0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Start Time	1	Thru	Right	App. Total	Left	Thru	Right A	\pp. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
1 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Hour Analysis From	04:00 P	M to 05:45	5 PM - Pe	ak 1 of 1													
0 1 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Your for Entire Inter-	section E	3egins at (04:45 PM														
0 1 0 1 0 0 1 0 0 0 0 0 0 0 0 0 0 0 0 0	04:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	_
0 1 2 0 2 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	05:00 PM	0	—	0	_	0	0	0	0	0	0	0	0	0	0	0	0	
0 4 0 6 4 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	05:15 PM	0	2	0	2	0	0	0	0	0	0	0	0	0	0	0	0	
0 4 0 4 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	05:30 PM	0	ı -	0	-	0	0	0	0	0	0	0	0	0	0	0	0	
0 100 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Total Volume	0	4	0	4	0	0	0	0	0	0	0	0	0	0	0	0	
000. 000. 000. 000. 000. 000. 000. 000. 000. 000. 000. 000. 000. 000. 000. 000.	% App. Total	0	100	0		0	0	0		0	0	0		0	0	0		
	PHF	000	.500	000	.500	000	000	000	000.	000	000	000	000.	000.	0000	000.	000.	.50

File Name 75860004 Site Code 75860004 Start Date 1/25/2018 Page No 15

N/S Street : Cottage Street E/W Street : Gove Street City/State : Boston, MA Weather : Clear



Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1

Peak Hour for Each Approach Begins at:

	04:45 PM			0	04:00 PM			ŏ	04:00 PM			ŏ	04:00 PM			
+0 mins.	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
+15 mins.	0	_	0	-	0	0	0	0	0	0	0	0	0	0	0	0
+30 mins.	0	·	0	6	0	0	0	0	0	0	0	0	0	0	0	0
+45 mins.	0	ı 	0	I —	0	0	0	0	0	0	0	0	0	0	0	0
Total Volume	0	4	0	4	0	0	0	0	0	0	0	0	0	0	0	0
% App. Total	0	100	0		0	0	0		0	0	0		0	0	0	

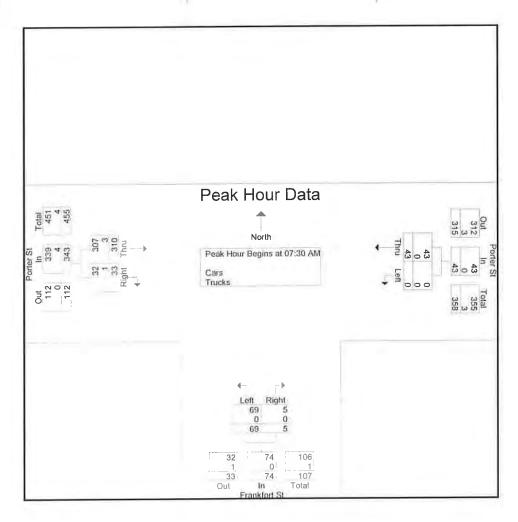
N/S Street : Frankfort Street E/W Street : Porter Street City/State : Boston, MA Weather : Clear

-	Profession 1	Dave :	Tanadan
Carpups	Printed-	Cars -	HUCKS

	Porter St From East		Frankfort S From South	1	Porter St From West		
Start Time	Left	Thru	Left	Right	Thru	Right	Int. Total
07:00 AM	0	7	17	1	44	6	75
07:15 AM	1	12	13	2	43	6	77
07:30 AM	0	17	12	1	57	8	95
07:45 AM	0	6	15	1	78	5	105
Total	1	42	57	5	222	25	352
08:00 AM	0	10	16	О	91	9	126
08:15 AM	0	10	26	3	84	11	134
08:30 AM	0	12	10	2	21	9	54
08:45 AM	0	8	5	1	17	1	32
Total	0	40	57	6	213	30	346
Grand Total	1	82	114	11	435	55	698
Apprch %	1.2	98.8	91.2	8.8	88.8	11.2	
Total %	0.1	11.7	16.3	1.6	62.3	7.9	
Cars	1	82	114	11	428	54	690
% Cars	100	100	100	100	98.4	98.2	98.9
Trucks	0	0	0	0	7	1	8
% Trucks	0	0	0	0	1.6	1.8	1.1

N/S Street: Frankfort Street E/W Street: Porter Street City/State : Boston, MA Weather : Clear

		Porter St From East			Frankfort St From South			Porter St From West		
Start Time	Left	Thru '	App. Total	Left	Right	App. Total	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 07	7:00 AM to 0	8:45 AM - F	Peak 1 of 1							
Peak Hour for Entire Interse	ction Begins	at 07:30 A	M							
07:30 AM	0	17	17	12	1	13	57	8	65	95
07:45 AM	0	6	6	15	1	16	78	5	83	105
08:00 AM	0	10	10	16	0	16	91	9	100	126
08:15 AM	0	10	10	26	3	29	84	11	95	134
Total Volume	0	43	43	69	5	74	310	33	343	460
% App. Total	0	100		93.2	6.8		90.4	9.6		
PHF	.000	.632	_632	.663	.417	.638	.852	.750	.858	.858
Cars	0	43	43	69	5	74	307	32	339	456
% Cars	0	100	100	100	100	100	99.0	97.0	98.8	99.1
Trucks	0	0	0	0	0	0	3	1	4	4
% Trucks	0	0	0	0	0	0	1.0	3.0	1.2	0.9



N/S Street : Frankfort Street E/W Street : Porter Street City/State : Boston, MA Weather : Clear

		Grou	ups Printed- Cars			
	Porter St From East		Frankfort S From Sout		Porter St From West	
Start Time	Left	Thru	Left	Right	Thru	Righ
07:00 AM	0	7	17	1	41	
07:15 AM	1	12	13	2	42	
07:30 AM	0	17	12	1	57	

	From East		From South	1	From Wes	t	
Start Time	Left	Thru	Left	Right	Thru	Right	Int. Tota
07:00 AM	0	7	17	1	41	6	72
07:15 AM	1	12	13	2	42	6	76
07:30 AM	0	17	12	1	57	8	9
07:45 AM	0	6	15	1	78	5	10
Total	1	42	57	5	218	25	34
08:00 AM	0	10	16	0	89	9	12
08:15 AM	0	10	26	3	83	10	13
08:30 AM	0	12	10	2	21	9	5
08:45 AM	0	8	5	1	17	1	3
Total	0	40	57	6	210	29	34
Grand Total	1	82	114	11	428	54	69
Apprch %	1.2	98.8	91.2	8.8	88.8	11.2	
Total %	0.1	11.9	16.5	1.6	62	7.8	

N/S Street : Frankfort Street E/W Street : Porter Street City/State : Boston, MA Weather : Clear

File Name : 75860005 Site Code : 75860005 Start Date : 1/25/2018 Page No : 7

		G	Groups Printed- Tr	ucks			
	Porter S From Eas	t	Frankfo From S	ort St	Porte From	er St West	
Start Time	Left	Thru	Left	Right	Thru	Right	Int. Total
07:00 AM	0	0	0	0	3	0	3
07:15 AM	0	0	0	0	1	0	1
07:30 AM	0	0	0	0	0	0	0
07:45 AM	0	0	0	0	0	0	0
Total	0	0	0	0	4	0	4
		ų.		,			
08:00 AM	0	0	0	0	2	0	2
08:15 AM	0	0	0	0	1	1	2
08:30 AM	0	0	0	0	0	0	0
08:45 AM	0	0	0	0	0	0	0
Total	0	0	0	0	3	1	4
Grand Total	0	0	0	0	7	1	8
Apprch %	0	0	0	0	87.5	12.5	
Total %	0	0	0	0	87.5	12.5	

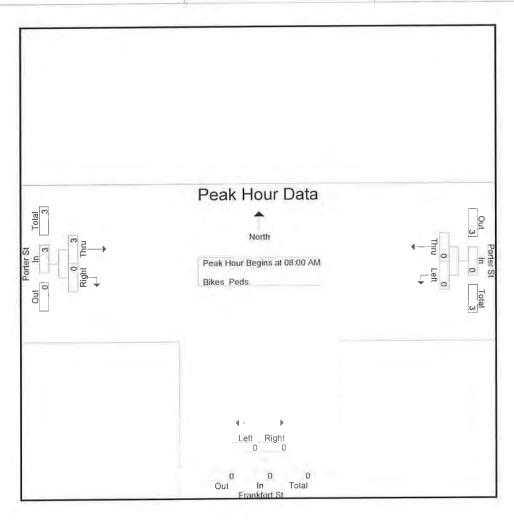
N/S Street : Frankfort Street E/W Street : Porter Street City/State : Boston, MA Weather : Clear

Groups	Printed-	Bikes	Peds	
rankfort	St			

		orter St om East		Fr: Er:	ankfort St om South		F	Porter St rom West				
Start Time	Left	Thru	Peds	Left	Right	Peds	Thru	Right	Peds	Exclu. Total	Inclu. Total	Int. Tota
07:00 AM	0	0	0	0	0	2	0	0	0	2	0	
07:15 AM	0	0	1	0	0	2	0	0	0	3	0	
07:30 AM	0	0	1	0	0	2	0	0	1	4	0	
07:45 AM	0	0	0	0	0	0	0	0	3	3	0	
Total	0	0	2	0	0	6	0	0	4	12	0	1
08:00 AM	0	0	0	0	0	16	2	0	5	21	2	2
08:15 AM	0	0	3	0	0	13	0	0	4	20	0	2
08:30 AM	0	0	1	0	0	4	0	0	0	5	0	
08:45 AM	0	0	0	0	0	3	1	0	1	4	1	
Total	0	0	4	0	0	36	3	0	10	50	3	
Grand Total	0	0	6	0	0	42	3	0	14	62	3	6
Apprch %	0	0		0	0		100	0				
Total %	0	0		0	0		100	0		95.4	4,6	

N/S Street : Frankfort Street E/W Street : Porter Street City/State : Boston, MA Weather : Clear

		Porter St From East			Frankfort St From South			Porter St From West		
Start Time	Left	Thru	App. Total	Left	Right	App. Total	Thru	Right	App. Total	Int. Tota
Peak Hour Analysis From 0	7:00 AM to 0	8:45 AM - F	Peak 1 of 1							
eak Hour for Entire Interse	ction Begins	at 08:00 A	М							
08:00 AM	0	0	0	0	0	0	2	0	2	2
08:15 AM	0	0	0	0	0	0	0	0	0	0
08:30 AM	0	0	0	0	0	0	0	0	0	0
08:45 AM	0	0	0	0	0	0	1	0	1	1
Total Volume	0	0	0	0	0	0	3	0	3	3
% App. Total	0	0		0	0		100	0		
PHF	.000	.000	.000	.000	.000	-000	.375	.000	.375	.375



N/S Street : Frankfort Street E/W Street : Porter Street City/State : Boston, MA Weather : Clear

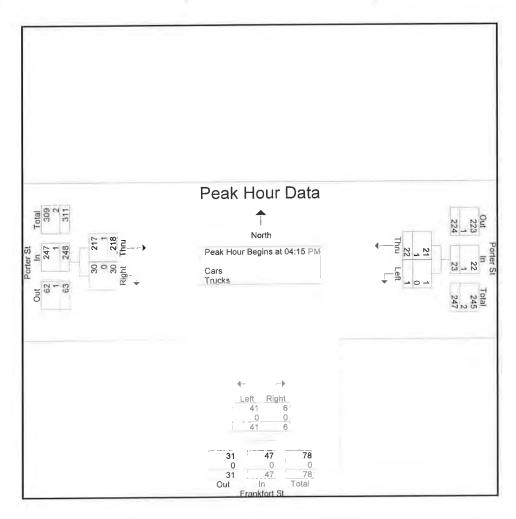
File Name : 75860005 Site Code : 75860005 Start Date : 1/25/2018 Page No : 1

Groups Printed- Cars - Trucks

		Porter St From West		Frankfort St From South		Porter St	
Int. Total	Right	Thru	Right	Left	Thru	From East Left	Start Time
68	6	47	1	9	5	0	04:00 PM
72	7	48	1	12	4	0	04:15 PM
81	13	56	O	11	1	0	04:30 PM
94	7	59	3	13	12	0	04:45 PM
315	33	210	5	45	22	0	Total
71	3	55	2	5	5	1	05:00 PM
70	5	40	1	13	9	2	05:15 PM
6	8	48	2	4	3	0	05:30 PM
64	9	42	1	10	2	0	05:45 PM
270	25	185	6	32	19 !	3	Total
585	58	395	11	77	41	3	Grand Total
	12.8	87.2	12.5	87.5	93.2	6.8	Apprch %
	9.9	67.5	1.9	13.2	7	0.5	Total %
582	58	394	11	77	39	3	Cars
99.5	100	99.7	100	100	95.1	100	% Cars
3	0	1	0	0	2	0	Trucks
0.5	0	0.3	0	0	4.9	0	% Trucks

N/S Street : Frankfort Street E/W Street : Porter Street City/State : Boston, MA Weather : Clear

		Porter St			Frankfort St			Porter St		
		From East			From South			From West		
Start Time	Left	Thru	App. Total	Left	Right	App. Total	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 04	4:00 PM to 0	5:45 PM - F	Peak 1 of 1							
Peak Hour for Entire Interse	ction Begins	at 04:15 P	M							
04:15 PM	0	4	4	12	1	13	48	7	55	72
04:30 PM	0	1	1	11	0	11	56	13	69	81
04:45 PM	0	12	12	13	3	16	59	7	66	94
05:00 PM	1	5	6	5	2	7	55	3	58	71
Total Volume	1	22	23	41	6	47	218	30	248	318
% App. Total	4.3	95.7		87.2	12.8		87.9	12.1		
PHF	.250	.458	.479	.788	.500	-734	.924	.577	.899	.846
Cars	1	21	22	41	6	47	217	30	247	316
% Cars	100	95.5	95.7	100	100	100	99.5	100	99.6	99.4
Trucks	0	1	1	0	0	0	1	0	1	2
% Trucks	0	4.5	4.3	0	0	0	0.5	0	0.4	0.6



N/S Street : Frankfort Street E/W Street : Porter Street City/State : Boston, MA Weather : Clear

File Name : 75860005 Site Code : 75860005 Start Date : 1/25/2018 Page No : 4

		Porter St From West		Frankfort St From South		Porter St From East	
Int. Tota	Right	Thru	Right	Left	Thru	Left	Start Time
6	6	47	1	9	5	0	04:00 PM
7	7	47	1	12	4	0	04:15 PM
8	13	56	0	11	1	0	04:30 PM
9	7	59	3	13	11	0	04:45 PM
31	33	209	5	45	21	0	Total
7	3	55	2	5	5	1	05:00 PM
7	5	40	1	13	9	2	05:15 PM
6	8	48	2	4	2	0	05:30 PM
6	9	42	1	10	2	0	05:45 PM
26	25	185	6	32	18	3	Total
58	58	394	11	77	39	3	Grand Total
	12.8	87.2	12.5	87.5	92.9	7.1	Apprch %
	10	67.7	1.9	13.2	6.7	0.5	Total %

N/S Street : Frankfort Street E/W Street : Porter Street City/State : Boston, MA Weather : Clear

File Name : 75860005 Site Code : 75860005 Start Date : 1/25/2018 Page No : 7

				s Printed- Trucks	Group		
		Porter St From West		Frankfort St From South		Porter St From East	
Int. Total	Right	Thru	Right	Left	Thru	Left	Start Time
0	0	0	0	0	0	0	04:00 PM
1	0	1	0	0	0	0	04:15 PM
0	0	0	0	0	0	0	04:30 PM
1	0	0	0	0	1	0	04:45 PM
2	0	1	0	0	1	0	Total
0	0	0		•	- 1		
U	U	U	0	0	0	0	05:00 PM
0	0	0	0	0	0	0	05:15 PM
1	0	0	0	0	1	0	05:30 PM
0	0	0	0	0	0	0	05:45 PM
1	0	0	0	0	1	0	Total
3	0	1	0	0	2	0	Grand Total
	О	100	0	0	100	0	Apprch %
	0	33.3	0	0	66.7	0	Total %

N/S Street : Frankfort Street E/W Street : Porter Street City/State : Boston, MA Weather : Clear

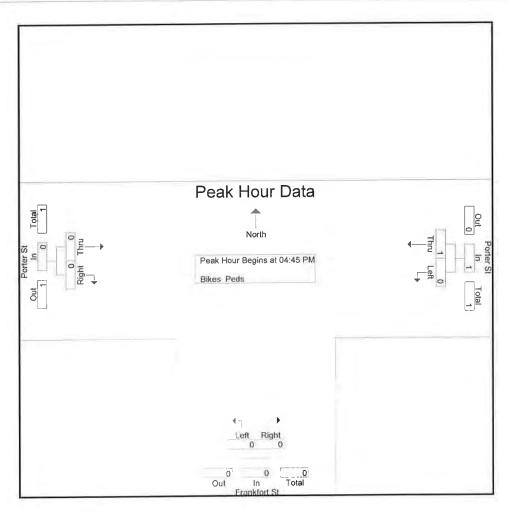
File Name : 75860005 Site Code : 75860005 Start Date : 1/25/2018 Page No : 10

Groups Printed- Bikes Peds

		orter St om East		Fr	ankfort St om South		Fr	Porter St rom West				
Start Time	Left	Thru	Peds	Left	Right	Peds	Thru	Right	Peds	Exclu. Total	Inclu. Total	Int. Total
04:00 PM	0	0	0	0	0	6	0	0	1	7	0	7
04:15 PM	0	0	1	0	0	3	0	0	0	4	0	4
04:30 PM	0	0	1	0	0	4	0	0	0	5	0	5
04:45 PM	0	0	0	0	0	4	0	0	1	5	0	5
Total	0	0	2	0	0	17	0	0	2	21	0	21
05:00 PM	0	0	1	0	0	3	0	0	0	4	0	4
05:15 PM	0	0	2	0	0	2	0	0	0	4	0	4
05:30 PM	0	1	2	0	0	6	0	0	1	9	1	10
05:45 PM	0	0	0	0	0	1	0	0	1	2	0	2
Total	0	1	5	0	0	12	0	0	2	19	1	20
Grand Total	0	1	7	0	0	29	0	0	4	40	1	41
Apprch %	0	100		0	0		0	0				
Total %	0	100		0	0		0	0		97.6	2.4	

N/S Street: Frankfort Street E/W Street: Porter Street City/State : Boston, MA Weather : Clear

		Porter St From East			Frankfort St From South			Porter St From West		
Start Time	Left	Thru	App. Total	Left	Right	App. Total	Thru	Right	App. Total	Int. Tota
Peak Hour Analysis From 04	1:00 PM to 0	5:45 PM - F	Peak 1 of 1							
Peak Hour for Entire Interse	ction Begins	at 04:45 PI	М							
04:45 PM	0	0	0	0	0	0	0	0	0	C
05:00 PM	0	0	0	0	0	0	0	0	0	(
05:15 PM	0	0	0	0	0	0	0	0	0	(
05:30 PM	0	1	1	0	0	0	0	0	0	1
Total Volume	0	1	1	0	0	0	0	0	0	1
% App. Total	0	100		0	0		0	0		
PHF	.000	.250	-250	.000	.000	.000	.000	.000	.000	.250



N/S Street : Orleans Street E/W Street : Porter Street City/State : Boston, MA Weather : Clear

File Name : 75860006 Site Code : 75860006 Start Date : 1/25/2018 Page No : 1

978-664-2

					Groups P	Groups Printed- Cars - Trucks					i		
	- ш	Orleans St From North		— ш	Porter St From Fast		ō ₽	Orleans St From South		i.	From West		
Start Time	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Int. Total
07:00 AM	_	0	1	0	26	0	12	0	0	_	48	80	26
07:15 AM	0	~	12	0	32	0	25	_	0	0	49	15	135
07:30 AM	-	0	0	0	33	0	28	0	0	8	59	13	136
07:45 AM	0	0	0	0	29	0	28	0	0	0	88	27	172
Total	2	+	5	0	120	0	93	-	0	က	244	63	540
08:00 AM	0	+	+	0	28	0	32	0	_	0	26	15	175
08:15 AM	0	0	+	0	40	0	32	-	7	-	92	24	193
08:30 AM	0	-	F	2	24	0	19	0	-	0	59	24	101
08:45 AM	0	÷	÷	۳	16	0	-	0	0	2	21	18	7.1
Total	0	က	4	က	108	0	94	1	4	ю	239	81	540
Grand Total	2	4	17	က	228	0	187	2	4	9	483	144	1080
Apprch %	8.7	17.4	73.9	1.3	98.7	0	96.9	-	2.1	0.9	76.3	22.7	
Total %	0.2	0.4	1.6	0.3	21.1	0	17.3	0.2	4.0	9.0	44.7	13.3	
Cars	2	4	17	ო	228	0	182	2	4	ဖ	474	142	1064
% Cars	100	100	100	100	100	0	97.3	100	100	100	98.1	98.6	98.5
Trucks	0	0	0	0	0	0	ις.	0	0	0	6	2	16
% Trucks	0	0	0	0	0	0	2.7	0	0	0	1.9	1.4	1.5

File Name : 75860006 Site Code : 75860006 Start Date : 1/25/2018 Page No : 2

N/S Street : Orleans Street E/W Street : Porter Street City/State : Boston, MA Weather : Clear

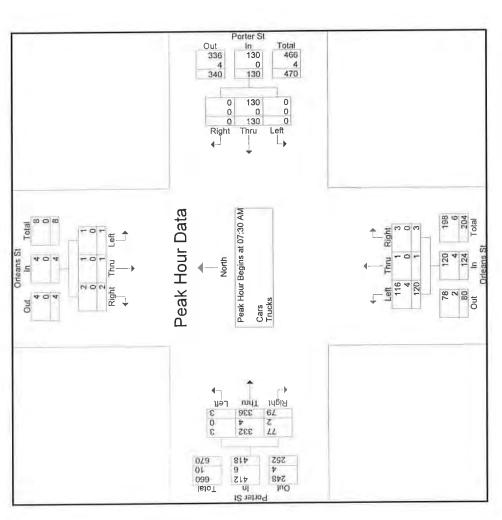
Int. Total Thru Right App. Total From West Porter St Left Right App. Total From South Orleans St Thru Left Thru Right App. Total From East Porter St Left Left Thru Right App. Total From North Orleans St Left Start Time Peak Hoo

Peak h

		136	172	175	193	929		.876	999	98.5	10	1.5
		74	115	112	117	418		.893	412	98.6	9	1.4
		13	27	15	24	79	18.9	.731	77	97.5	2	2.5
		59	88	26	92	336	80.4	998.	332	98.8	4	1.2
		2	0	0	-	က	0.7	375	က	100	0	0
		28	28	33	35	124		988.	120	8.96	4	3.2
		0	0	_	2	က	2.4	.375	က	100	0	0
		0	0	0	-		8.0	.250	-	100	0	0
		28	28	32	32	120	8.96	.938	116	2.96	4	3.3
		33	59	28	40	130		.813	130	100	0	0
		0	0	0	0	0	0	000.	0	0	0	0
		33	29	28	40	130	100	.813	130	100	0	0
		0	0	0	0	0	0	000	0	0	0	0
1 of 1		_	0	7	_	4		.500	4	100	0	0
AM - Peak	7:30 AM	0	0	-	-	2	90	.500	2	100	0	0
M to 08:45	Begins at 0	0	0	~	0	~	25	.250	-	100	0	0
n 07:00 A	tersection E	-	. 0	0	0	-	25	.250	-	100	0	0
ik Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1	ik Hour for Entire Intersection Begins at 07:30 AM	07:30 AM	07:45 AM	08:00 AM	08:15 AM	Total Volume	% App. Total	PHF	Cars	% Cars	Trucks	% Trucks

File Name 75860006 Site Code 75860006 Start Date 1/25/2018 Page No 3

N/S Street : Orleans Street E/W Street : Porter Street City/State : Boston, MA Weather : Clear



Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1

Peak Hour for Each Approach Begins at:

J	07:00 AM			07	7:30 AM			0	07:30 AM			.0	07:30 AM			
+0 mins.	-	0	~	2	0	33	0	33	28	0	0	28	7	29	13	74
+15 mins.	. 0	-	12	13	0	29	0	59	28	0	0	28	0	88	27	115
+30 mins.	_	0	0	_	0	28	0	28	32	0	-	33	0	26	15	112
+45 mins.	0	0	0	0	0	40	0	40	32	~	2	35	←	92	24	117
otal Volume	2	_	13	16 :	0	130	0	130	120	•	က	124	က	336	79	418
% App. Total	12.5	6.2	81.2		0	100	0		96.8	8.0	2.4		0.7	80.4	18.9	

File Name 75860006 Site Code 75860006 Start Date 1/25/2018 Page No 5

	0 1	Orleans St From North			Porter St From East			Orleans St From South		- <u>F</u>	Porter St From West		
Start Time	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Int. Total
07:00 AM	-	0	-	0	26	0	12	0	0	-	45	00	94
07:15 AM	0	~	12	0	32	0	25	-	0	0	48	15	134
07:30 AM	,	0	0	0	33	0	27	0	0	2	59	13	135
07:45 AM	0	0	0	0	29	0	28	0	0	0	87	26	170
Total	2	-	13	0	120	0	92	~	0	ю	239	62	533
08:00 AM	0	-	-	0	28	0	32	0	-	0	95	14	172
08:15 AM	0	0	+	0	40	0	29	~	2	-	91	24	189
08:30 AM	0	←	+	2	24	0	18	0	-	0	29	24	100
08:45 AM	0	-	***	-	16	0	+	0	0	2	20	8	20
Total	0	ю	4	m	108	0	06		4	m	235	80	531
Grand Total	2	4	17	က	228	0	182	7	4	9	474	142	1064
Apprch %	8.7	17.4	73.9	1.3	98.7	0	96.8	1:	2.1	-	76.2	22.8	
Total %	0.2	0.4	1.6	0.3	21.4	0	17.1	0.2	0.4	9.0	44.5	13.3	

File Name 75860006 Site Code 75860006 Start Date 1/25/2018 Page No 9

	ŌĒ	Orleans St From North		Q T	Porter St From Fast	it		Orleans St From South		— ii	Porter St From West		
Start Time	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Teff	Thru	Right	Int. Total
07:00 AM	0	0	0	0	0	0	0	0	0	0	က	0	e
07:15 AM	0	0	0	0	0	0	0	0	0	0	~	0	_
07:30 AM	0	0	0	0	0	0	٢	0	0	0	0	0	_
07:45 AM	0	0	0	0	0	0	0	0	0	0	۲	-	2
Total	0	0	0	0	0	0	-	0	0	0	ıs	•	2
08:00 AM	0	0	0	0	0	0	0	0	0	0	2	-	.,
08:15 AM	0	0	0	0	0	0	ю	0	0	0	-	0	7
08:30 AM	0	0	0	0	0	0	-	0	0	0	0	0	_
08:45 AM	0	0	0	0	0	0	0	0	0	0	~	0	`
Total	0	0	0	0	0	0	4	0	0	0	4	+	6
Grand Total	0	0	0	0	0	0	5	0	0	0	တ	2	16
Apprch %	0	0	0	0	0	0	100	0	0	0	81.8	18.2	
Total %	0	0	0	0	0	0	31.2	0	0	0	56.2	12.5	

File Name : 75860006 Site Code : 75860006 Start Date : 1/25/2018 Page No : 13

				Groups	Groups Printed- Bikes Peds	3ikes Pe	ds					1			
		Porter St From East	St			Orleans St From South	s St outh			Porter St From West	St				
Peds	Left	Thru	Right	Peds	Left	Thru	Right	Peds	Left	Thru	Right	Peds	Peds Exclu. Total Inclu. Total	Inclu. Total	Int. Total
9	0	0	0	7	0	0	0	ന	0	0	0	2	5	0	13
4	0	0	0	•	0	0	0	-	0	0	0	-	7	0	7
Ø	0	0	0	+	0	0	0	4	0	0	0	0	14	0	14
12	0	0	0	S	0	0	0	-	0	0	0	0	18	0	6
31	0	0	0	σ	0	0	0	σ	0	0	0	m	52	0	52
9	0	0	0	7	0	0	0	2	0	0	0	4	36	0	36
6	0	0	0	7	0	0	0	2	0	0	0	-	33	0	33
7	0	0	0	Ŋ	0	0	0	2	0	0	0	•	15	0	15
5	0	0	0	0	0	0	0	2	0	0	0	•	60	0	00
20	0	0	0	27	0	0	0	ø0	0	0	0	7	92	0	92
20	0 0	0 0	0 0	36	0 0	0 0	0 0	17	0 0	0 0	0 0	10	144	0	144
													100	0	

File Name : 75860006 Site Code : 75860006 Start Date : 1/25/2018 Page No : 14

		From	From North			Porter St From East	r St East			Orleans St From South	ns St South			Porter St From West	er St West		
Start Time	Left	Thru	Thru Right App. Total	pp. Total	Left	Thru	Right	Right App. Total	Left	Thru	Right /	App. Total	Left	Thru	Right	Right App. Total	Int. Total
Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1	07:00 A	4M to 08:4	15 AM - Peal	k 1 of 1													
Peak Hour for Entire Intersection Begins at 07:00 AM	section	Begins at	07:00 AM														
07:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	_
07:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	_
07:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	_
07:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	_
Total Volume	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
% App. Total	0	0	0		0	0	0		0	0	0		0	0	0		
PHF	000	000	000	000	000	000	000	000	000	000	000	000.	000.	000	000	000.	000

File Name 75860006 Site Code 75860006 Start Date 1/25/2018 Page No 15

N/S Street : Orleans Street E/W Street : Porter Street City/State : Boston, MA Weather : Clear

Porter St Out 0 Total 0 Right Thru Left Peak Hour Data Peak Hour Begins at 07:00 AM Total Total Out In 0 0 Right Thu Out In Bikes Peds unitT ← Teff O o ingiA JuO lisioT 0 Porter St 0

Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1

Peak Hour for Each Approach Begins at:

	07:00 AM			0	7:00 AM			ی	17:00 AM			_	77:00 AM			
+0 mins.	0	0	0	0	0	0	0	0	0			0	0			0
+15 mins.	0	0	0	0	0	0	0	0	0			0	0			0
+30 mins.	0	0	0	0	0	0	0	0	0			0	0			0
+45 mins.	0	0	0	0	0	0	0	0	0			0	0			0
Total Volume	0	0	0	0	0	0	0	0	0 0	0	0	0	0 0	0	0	0
% App. Total	0	0	0		0	0	0		0				0			

File Name : 75860006 Site Code : 75860006 Start Date : 1/25/2018 Page No : 1

Corteans St From East From South Thru III Porter St From East From South III From South III Right III Left From East From South III Right III <						Groups P	Groups Printed- Cars - Trucks	Trucks						
Left Thru Right		ОЦ	Orleans St rom North		T [orter St om East			Orleans St		<u> </u>	Porter St From West		
1 0 1 1 18 0 27 0 0 0 2 0 17 0 26 0 0 0 0 16 0 33 0 1 1 25 1 23 1 0 1 0 10 33 0 1 2 76 1 109 1 0 0 1 0 10 30 0 1 2 7 10 0 1 0 1 0 0 1 1 10 0 15 0 15 0 1 2 1	Start Time	Left	Thru	Right		Thru	Right	-	Thru	Right	Left	Thru	Right	Int, Total
0 0 17 0 26 0 0 0 0 16 0 33 0 0 1 3 1 25 1 23 1 1 1 1 25 1 1 1 1 0 0 1 0 10 0 30 1 1 2 1 0 10 0 30 0 1 2 1 1 1 10 0 15 0 1 0 0 1 0 15 0 33 0 0 1 2 1	04:00 PM	-	0	-	-	18	0	27	0	0	0	48	00	104
0 0 16 0 33 0 0 1 3 1 25 1 23 1 1 1 1 1 1 1 1 1 1 1 1 1 2 76 1 109 1<	04:15 PM	0	0	2	0	17	0	26	0	0	9	56	15	122
0 1 3 1 25 1 23 1 1 1 1 2 76 1 109 1 0 0 1 0 10 0 30 0 1 2 1 1 10 0 33 0 0 0 0 1 0 15 0 15 0 1 2 1 1 1 0 15 0 15 0 1 2 1 1 1 0 15 0 15 0 1 2 1 1 1 1 1 2 1 1 1 1 2 1	04:30 PM	0	0	0	0	16	0	33	0	0	0	70	21	140
1 1 6 2 76 1 109 1 0 0 1 0 10 0 30 0 1 2 1 1 1 10 0 33 0 1 2 1 1 1 10 0 33 0 1 2 1	04:45 PM	0	~	က	-	25	-	23	~	0	-	99	17	139
0 0 1 0 10 0 30 0 1 2 1 0 19 2 24 0 1 2 1 1 10 0 33 0 0 0 1 1 1 10 33 0 1 2 1 1 1 1 15 2 0 1 2 5 1 54 2 102 2 2 3 11 3 130 3 211 3 12.5 18.8 68.8 2.2 95.6 2.2 98.1 1.4 0.2 0.3 11 0.3 13.4 0.3 21.7 0.3 100 100 100 100 0 0.3 1.0 0.3 1.0 0 0 0 0 0 0 0 0 0 0 0	Total	~	-	9	2	92	-	109	~	0	7	240	61	505
0 0 1 0 10 0 30 0 1 2 1 0 19 2 24 0 1 2 1 1 10 0 33 0 0 0 1 0 15 0 15 0 1 2 1 1 1 15 0 1 2 1 2 5 1 54 2 102 2 2 3 11 3 11 3 11 3 12.5 18.8 68.8 2.2 95.6 2.2 98.1 1.4 0.2 0.3 1.1 0.3 13.4 0.3 21.7 0.3 100 100 100 0 0 0 0 0 0 0 0 0 2 3 11 3 21 3 10 0 0 </td <td></td>														
0 0 19 2 24 0 1 2 1 1 10 33 0 0 0 1 1 10 15 0 0 1 2 1 0 15 0 15 0 2 3 1 0 1 54 2 102 2 2 3 11 3 130 3 211 3 14 12.5 18.8 68.8 2.2 95.6 2.2 98.1 1.4 0.2 0.3 1.1 0.3 13.4 0.3 21.7 0.3 100 100 100 100 98.5 100 99.5 100 0 0 0 0 0 0 0 0 0 0	05:00 PM	0	0	-	0	10	0	30	0	0	4	61	19	125
1 2 1 1 10 0 15 0 15 0 1 2 1 0 15 0 15 2 0 15 2 2 1 2 1 2 2 1 2 2 1 2 2 2 1 2 3 1 3 1 3 1 3 1 3 1 3 1 1 4 3 1 1 0 0 0 0 0 0 0 0 0 </td <td>05:15 PM</td> <td>0</td> <td>0</td> <td>2</td> <td>0</td> <td>19</td> <td>2</td> <td>24</td> <td>0</td> <td>0</td> <td>-</td> <td>46</td> <td>18</td> <td>112</td>	05:15 PM	0	0	2	0	19	2	24	0	0	-	46	18	112
0 0 15 0 15 2 1 2 5 1 54 2 102 2 2 3 11 3 130 3 211 3 12.5 18.8 68.8 2.2 95.6 2.2 98.1 1.4 0.2 0.3 1.1 0.3 13.4 0.3 21.7 0.3 2 3 11 3 128 3 21.7 0.3 100 100 100 98.5 100 99.5 100 0 0 0 0 0.5 0 0	05:30 PM	F	2	_	←	10	0	33	0	7	က	99	14	122
1 2 5 1 1 54 2 102 2 2 3 111 3 130 3 211 3 12.5 18.8 68.8 2.2 95.6 2.2 98.1 1.4 0.2 0.3 1.1 0.3 13.4 0.3 21.7 0.3 2 3 111 3 128 3 210 3 100 100 100 98.5 100 99.5 100 0 0 0 0 1.5 0 0.5 0	05:45 PM	0	0	-	0	15	0	15	7	0	0	55	20	108
2 3 11 3 130 3 211 3 12.5 18.8 68.8 2.2 95.6 2.2 98.1 1.4 0.2 0.3 13.4 0.3 21.7 0.3 2 3 11 3 128 3 210 3 100 100 100 98.5 100 99.5 100 0 0 0 0 1.5 0 0.5 0	Total	_	2	ıc	T.	54	2	102	2	V	∞	218	7.1	467
12.5 18.8 68.8 2.2 95.6 2.2 98.1 1.4 0.2 0.3 13.4 0.3 21.7 0.3 2 3 11 3 128 3 210 3 100 100 100 98.5 100 99.5 100 0 0 0 0 0 0.5 0	Grand Total	7	ო	17	က	130	n	211	ო	7	15	458	132	972
0.2 0.3 1.1 0.3 13.4 0.3 21.7 0.3 0.3 2 3 11 3 128 3 210 3 100 100 100 98.5 100 99.5 100 10 0 0 0 0 1.5 0 0.5 0	Apprch %	12.5	18.8	68.8	2.2	92.6	2.2	98.1	1.4	0.5	2.5	7.5.7	21.8	
2 3 11 3 128 3 210 3 100 100 100 98.5 100 99.5 100 10 0 0 0 0 0 0 1 0 10 0 0 0 0 0 0 0 0 0 0 0 0 0	Total %	0.2	0.3	1.1	0.3	13.4	0.3	21.7	0.3	0.1	1.5	47.1	13.6	
100 100 100 98.5 100 99.5 100 10 0 0 0 0 0 0 1 0 0 0 0 0 0 0 0 0	Cars	2	က	11	က	128	8	210	က	-	15	457	131	296
0 0 0 0 0 0 0 0.5 0	% Cars	100	100	100	100	98.5	100	99.5	100	100	100	8.66	99.2	99.5
0 0 0 0 0.5 0	Trucks	0	0	0	0	2	0	-	0	0	0		-	2
	% Trucks	0	0	0	0	1.5	0	0.5	0	0	0	0.2	0.8	0.5

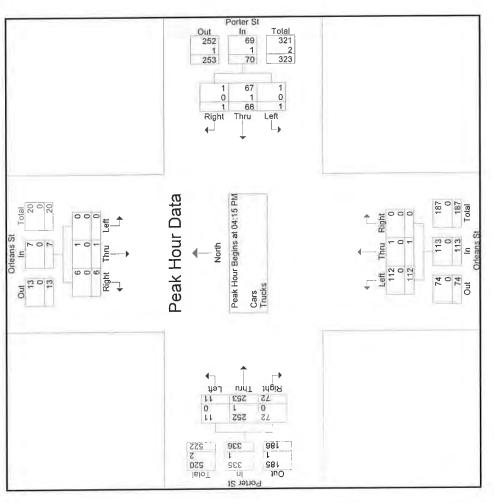
File Name : 75860006 Site Code : 75860006 Start Date : 1/25/2018 Page No : 2

-		Orleë	Orleans St From North			Port	Porter St From East			Orleans St From South	Orleans St rom South			Porter St From West	er St West		
Start Time		Thru	Righ	Left Thru Right App. Total	Left	Thru	Right	Right App. Total	Left	Thru	Right	Right App. Total	Left	Thru	Right	Thru Right App. Total	ij
Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1	rom 04:00	PM to 05:	:45 PM -	Peak 1 of 1													
Peak Hour for Entire Intersection Begins at 04:15 PM	ntersection	າ Begins a	at 04:15	PM													

-		From North	North			From East	East			From South	onth			From West	west		
Start Time		Thru	Left Thru Right App. Total	p. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
x Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1	rom 04:00 F	PM to 05:4;	5 PM - Peak	1 of 1													
Hour for Entire Intersection Begins at 04:15 PM	ntersection	Begins at i	04:15 PM														
04:15 PM	0	0	2	2	0	17	0	17	26	0	0	26	9	99	15	77	122
04:30 PM	0	0	0	0	0	16	0	16	33	0	0	33	0	70	21	91	140
04:45 PM	0	-	ო	4	_	25	-	27	23	_	0	24	_	99	17	84	139
05:00 PM	0	0	٢	-	0	10	0	10	30	0	0	30	4	61	19	84	125
Total Volume	0	~	9	7	-	68	-	70	112	-	0	113	1	253	72	336	526
% App. Total	0	14.3	85.7		1.4	97.1	4.1		99.1	6.0	0		3.3	75.3	21.4		
PHF	000	.250	.500	.438	.250	.680	.250	.648	.848	.250	000.	.856	.458	904	.857	.923	.939
Cars	0	-	9	7	-	29	-	69	112	-	0	113	1-	252	72	335	524
% Cars	0	100	100	100	100	98.5	100	98.6	100	100	0	100	100	9.66	100	2.66	9.66
Trucks	0	0	0	0	0	~	0	_	0	0	0	0	0	*	0	_	2
% Trucks	0	0	0	0	0	1.5	0	1,4	0	0	0	0	0	4.0	0	0.3	0.4

File Name : 75860006 Site Code : 75860006 Start Date : 1/25/2018 Page No : 3

N/S Street : Orleans Street E/W Street : Porter Street City/State : Boston, MA Weather : Clear



Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1

Peak Hour for Each Approach Begins at:

3	04:45 PM			0	04:00 PM			J	04:15 PM			_	04:15 PM			
+0 mins.	0	-	က	4	7	18	0	19	26	0	0	26	9	56	15	77
+15 mins.	0	0	_	_	0	17	0	17	33	0	0	33	0	70	21	91
+30 mins.	0	0	2	2	0	16	0	16	23	-	0	24	τ-	99	17	84
+45 mins.	~	2	-	4	-	25	-	27	30	0	0	30	4	61	19	84
otal Volume	۳	က	7	7	2	92	÷	6/	112	τ-	0	113	1	253	72	336
% App. Total	9.1	27.3	63.6		2.5	96.2	1.3		99.1	6.0	0		3.3	75.3	21.4	

File Name : 75860006 Site Code : 75860006 Start Date : 1/25/2018 Page No : 5

					Grou	Groups Printed- Cars	Cars						
	- u.	Orleans St From North		- ŭ	Porter St From East			Orleans St From South		- 正	Porter St From West		
Start Time	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Int. Total
04:00 PM	_	0	_	-	18	0	26	0	0	0	48	00	103
04:15 PM	0	0	2	0	17	0	26	0	0	9	55	15	121
04:30 PM	0	0	0	0	16	0	33	0	0	0	02	21	140
04:45 PM	0	_	9	~	24	_	23	-	0	~	99	17	138
Total	~	-	9	2	75		108	-	0	7	239	61	502
05:00 PM	0	0	~	0	10	0	30	0	0	4	61	19	125
05:15 PM	0	0	2	0	19	2	24	0	0	-	46	18	112
05:30 PM	-	2	7		o	0	33	0	-	က	56	13	120
05:45 PM	0	0	~	0	15	0	15	2	0	0	55	20	108
Total	+	2	5	· -	53	2	102	2	*	60	218	0.2	465
Grand Total	2	ю	11	က	128	က	210	က	_	15	457	131	296
Apprch %	12.5	18.8	68.8	2.2	95.5	2.2	98.1	1.4	0.5	2.5	75.8	21.7	
Total %	0.2	0.3	1.1	0.3	13.2	0.3	21.7	0.3	0.1	1.6	47.3	13.5	

File Name : 75860006 Site Code : 75860006 Start Date : 1/25/2018 Page No : 9

N/S Street: Orleans E/W Street: Porter S City/State: Boston, Weather: Clear

Street	reet	٧	
: Orleans Street	rter Stı	Boston, MA	75
ō	. Po	: Bo	· Cloar
I/S Street	:/W Street : Porter Street	ity/State	Josephor
2	\leq	Ě	-

Start Time Left From North Time Right Left From South Time Right Left Through Time Right Left Through Time Right Left Through Time Right Left Through Time Right Right <t< th=""><th></th><th>J</th><th>Orleans St</th><th></th><th>4</th><th>Porter St</th><th>1</th><th></th><th>rleans St</th><th></th><th>Т</th><th>Porter St</th><th></th><th></th></t<>		J	Orleans St		4	Porter St	1		rleans St		Т	Porter St		
Left Thru Right Left Thru Right 0		L.	rom North		ű.	rom East		Frc	om South		H	om West		
	Start Time		Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Int. Total
	04:00 PM	0	0	0	0	0	0	_	0	0	0	0	0	
	04:15 PM	0	0	0	0	0	0	0	0	0	0	-	0	-
0 0 0 1 0 0 1 0 0 0 0 0 0 0 0 0 0 0 0 0	04:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
0 0 0 1 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	04:45 PM	0	0	0	0	~	0	0	0	0	0	0	0	
0 0	Total	0	0	0	0	•	0	-	0	0	0	-	0	m
0 0	05:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
0 0	05:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
0 0	05:30 PM	0	0	0	0	_	0	0	0	0	0	0	-	20
0 0	05:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
0 0 0 0 2 0 1 0 0 0 0 0 100 0 0 0 0 0 0 0 40 0 20 0	Total	0	0	0	0	-	0	0	0	0	O	0	*	2
0 0 0 100 0 100 0 0 0 0 0 40 0 20 0	Grand Total	0	0	0	0	2	0	-	0	0	0	=	<u></u>	5
$0 \qquad 0 \qquad 0 \qquad 40 \qquad 0 \qquad 20 \qquad 0$	Apprch %	0	0	0	0	100	0	100	0	0	0	20	20	
	Total %	0	0	0	0	40	0	20	0	0	0	20	20	

File Name | 75860006 Site Code | 75860006 Start Date | 1/25/2018 Page No | 13

N/S Street : Orleans Street E/W Street : Porter Street City/State : Boston, MA Weather : Clear

From North From North Left Thru Right Peds Left Thru Right Thru Rinch Thru Right Thru Right Thru Right Thru Right Thru Right Thru R			Orlean	t			Porfer	T.	dnoio	Orleans Cillied Bires Leas	Orleans St	5.0			Porter St	Ť.				
Left Thru (Left) Right (Left) Peds (Left) Left (Left) Thru (Right) Right (Left) Right (Left) Thru (Right) Right (Left) Peds (Left) Thru (Right) Right (Left) Peds (Left) Thru (Right) Right (Left) Peds (Left) Thru (Right) Peds (Left) Thru (Right) Peds (Left) Thru (Right) Peds (Left) Thru (Left) Right (Left) Peds (Left) Thru (Right) Thru (Right) Peds (Left) Thru (Right) Peds (Left) Thru (Right)			From	lorth			From E	ast			From Sc	outh			From W	est				
0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	art Time	Left	Thru	Right	Peds	Left	Thru		Peds	Left	Thru	Right	Peds	Left	Thru	Right		Exclu, Total	Inclu. Total	Int. Total
0 0	4:00 PM	0	0	0	9	0	0	0	~	0	0	0	4	0	0	0	0	1	0	1
1	4:15 PM	0	0	0	9	0	0	0	~	0	0	0	6	0	0	0	0	10	0	10
0 0 0 0 0 28 0 0 0 0 0 0 0 0 0 0 0 0 0 0	4.30 PM	0	0	0	σ	0	0	0	0	0	0	0	7	0	0	0	m	19	0	19
0 0 0 0 11 0 0 0 0 0 0 0 0 0 0 0 0 0 0	4:45 PM	0	0	0	7	0	0	0	0	0	0	0	60	0	0	0	0	10	0	10
0 0	Total	0	0	0	28	0	0	0	2	0	0	0	17	0	0	0	6	20	0	50
0 0	5:00 PM	0	0	0	<u></u>	0	0	0	0	0	0	0	-	0	0	0	0	12	0	12
0 0 10 0 0 0 0 0 0 0 1 0 0 1 0 0 1 0 0 1 0	5:15 PM	0	0	0	60	0	0	0	0	0	0	0	ო	0	0	0	0	=======================================	0	7
0 0 0 15 0 0 0 0 0 1 0 0 0 0 0 0 0 0 0 0	5:30 PM	0	0	0	10	0	0	0	2	0	0	0	ю	0	0	7	0	15	-	16
0 0 0 0 44 0 0 0 3 0 0 0 0 1 1 1 2 0 0 0 0 0 0 0 5 0 0 0 0 0 50 9 0 0 0 0 0 0 50 8 9	5:45 PM	0	0	0	15	0	0	0	-	0	0	0	0	0	-	0	2	18		19
0 0 0 72 0 0 0 5 0 0 0 0 1 1 1 5 0 0 0 0 0 0 24 0 1 1 1 5 0 0 0 0 0 0 0 50 50 80 80 80 80 80 80 80 80 80 80 80 80 80	Total	0	0	0	44	0	0	0	е	0	0	0	7	0	+	÷	2	56	2	58
0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	ind Total	0	0	0	72	0	0	0	Ŋ	0	0	0	24	0	+	-	52	106	2	108
0 0 0 0 0 0 0 0 0 0 0	pprch %	0	0	0		0	0	0		0	0	0		0	20	20				
	Total %	0	0	0		0	0	0		0	0	0		0	20	20		98.1	1.9	

File Name 75860006 Site Code 75860006 Start Date 1/25/2018 Page No 14

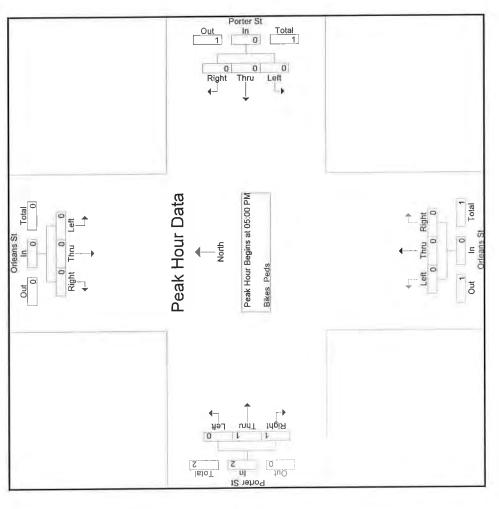
N/S Street: Orleans Street E/W Street: Porter Street City/State: Boston, MA Weather: Clear

	Crie	Orleans St			Porte	Porter St			Orleans St	Orleans St			Porter St	ar St	
	Ē	HOLL MOLLI			5	Edsi			11011	DOULL			2	N CON	
Left	Thru	Right	Right App. Total	Left	Thru	Right	Right App. Total	Left	Thru	Right /	App. Total	Left	Thru	Right	App. Total

		Orleans St				Porter St	in of			Orleans St	ns SI			TOT DOT	Porter St		
	_	From North	٦.			From East	East			From South	South			From	From West		
Start Time Lo	Left Th	Thru Right App. Total	ight A	pp. Total	Left	Thru	Right	Right App. Total	Left	Thr	Right	Right App. Total	Left	Thru	Right	Right App. Total	Int. Total
Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1	4:00 PM to	o 05:45 PN	M - Peal	c 1 of 1													
Peak Hour for Entire Intersection Begins at 05:00 PM	ction Beg	ins at 05:0)0 PM														
05:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
05:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
05:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	_	_	
05:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	~	0	1	•
Total Volume	0	0	0	0	0	0	0	0	0	0	0	0	0	-	-	2	
% App. Total	0	0	0		0	0	0		0	0	0		0	20	20		
PHF .0	000.	000.	000	000	000.	000	000.	000.	000	000	000	000	000	.250	.250	.500	.500

File Name 75860006 Site Code 75860006 Start Date 1/25/2018 Page No 15

N/S Street : Orleans Street E/W Street : Porter Street City/State : Boston, MA Weather : Clear



Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1

Peak Hour for Each Approach Begins at:

	04:00 PM			94	04:00 PM			J	04:00 PM			06	05:00 PM			
+0 mins.	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
+15 mins.	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
+30 mins.	0	0	0	0	0	0	0	0	0	0	0	0	0	0	-	-
+45 mins.	0	0	0	0	0	0	0	0	0	0	0	0	0	-	0	_
otal Volume	0	0	0	0	0	0	0	0	0	0	0	0	0	-	-	2
% App. Total	0	0	0		0	0	0		0	0	0		0	20	20	

File Name: 75860007 Site Code: 75860007 Start Date: 1/25/2018 Page No: 1

N/S Street: Bremen Street E/W Street: Porter Street City/State: Boston, MA Weather: Clear Groups Printed- Cars - Trucks

		Bremen St	n St			Porter St	्ट्र			Bremen St	St			Porter St	St		L	Porter PI	1		
	1 3/2 3	From North	lorth	The state of the s		From East	ast			From South	outh			From West			-	From Northwest	nwest		H
Start Time	Left	Thru	Right	HdRt	Left	Thru	BrRt	Right	Left	BrLt	밀	Right	Halt	Left	Dru L	Kight	Har.t	Bri	Brki	HOKE	Int. I otal
07:00 AM	35	36	0	-	0	0	21	18	0	14	20	4	0	∞	18	4	0	0	0	0	179
07:15 AM	35	42	0	0	က	0	32	32	0	4	29	9	2	4	22	9	0	0	0	0	227
07:30 AM	53	42	0	0	0	0	31	28	0	20	23	4	_	2	24	9	0	0	0	0	237
07:45 AM	56	56	0	0	-	0	27	22	0	12	4	က	0	~	31	13	0	0	0	0	236
Total	179	176	0	-	4	0	111	100	0	09	98	17	ო	18	95	29	0	0	0	0	879
08:00 AM	09	38	0	0	•	0	24	23	0	18	24	6	7	0	26	rc	0	0	0	0	229
08:15 AM	72	28	0	0	7	0	32	38	0	10	31	12	0	2	35	o	0	0	0	0	271
08:30 AM	22	31	0	2	2	0	17	25	0	က	22	6	-		20	9	0	0	0	0	161
08:45 AM	26	27	0	~	A pril	0	17	12	0	19	17	Ŋ	F	-	7	7	0	0	0	0	140
Total	180	124	0	က	9	0	06	86	0	20	94	35	m	4	95	22	0	0	0	0	801
Grand Total	359	300	0	4	10	0	201	198	0	110	180	52	9	22	187	51	0	0	0	0	1680
Apprch %	54.1	45.2	0	9.0	2.4	0	49.1	48.4	0	32.2	52.6	15.2	2.3	8.3	70.3	19.2	0	0	0	0	
Total %	21.4	17.9	0	0.2	9.0	0	12	11.8	0	6.5	10.7	3.1	0.4	1.3	11.1	က	0	0	0	0	
Cars	353	294	0	4	10	0	201	194	0	110	176	52	9	22	182	51	0	0	0	0	1655
% Cars	98.3	98	0	100	100	0	100	86	0	100	97.8	100	100	100	97.3	100	0	0	0	0	98.5
Trucks	9	9	0	0	0	0	0	4	0	0	4	0	0	0	ro	0	0	0	0	0	25
% Trucks	1.7	2	0	0	0	0	0	2	0	0	2.2	0	0	0	2.7	0	0	0	0	0	1.5

File Name : 75860007 Site Code : 75860007 Start Date : 1/25/2018 Page No : 2

		App. Total Int Total	
_	west	l HdRt	
Porter PI	rom Northwest	BrRI	
	Fron	BrLt	
		HdLt	
Porter St	From West	HdLi Left Thru Right App. Total	
Bremen St	From South	Left BrLI Thm Right App. Total	
Porter St	From East	Leff Thru BrRI Right App. Total	c1 of 1
Bremen St	From North	Start Time Left Thru Right HdRt App. Total	Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1
		Start Time	eak Hour Analy

		Ą	Bremen St	St			Д.	Porter St	<u></u>			西	Bremen St	#			ď	Porter St				S.	Porter PI			
		F	From North	₽			Ē	From East	st			포	From South	£			Fro	From West				From	From Northwest	est		
ime	Start Time Left Thru	Thru	Right HdRt		App. Total	Left	Thru	BrRI	Right	Right App. Total	Leff	BrLt	Thru	Right App. Total	pp. Total	HdL1	Left	Thr. R	Right App. Total	Total	HdLt	BrLt	BrRI	HdRt	App. Total In	Int. Total
ır Analy	sis Fro	m 07:00	D AM to	08:45 /	Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1	k 1 of 1																				
ur for Ei	ntire Int	ersectic	ın Begi	Peak Hour for Entire Intersection Begins at 07:30 AM	:30 AM																					
07:30 AM	53	42	0	0	92	0	0	31	28	29	0	20	23	4	47	-	ro	24	9	36	0	0	0	0	0	237
07:45 AM	99	26	0	0	112		0	27	22	20	0	12	4	က	53	0	_	31	13	45	0	0	0	0	0	236
08:00 AM	09	38	0	0	98	-	0	24	23	48	0	18	24	თ	21	-	0	56	2	32	0	0	0	0	0	229
08:15 AM	72	28	0	0	100	2	0	32	38	72	0	10	31	12	53	0	2	35	6	46	0	0	0	0	0	271
Total Volume	241	164	0	0	405	4	0	114	111	229	0	09	92	28	180	7	œ	116	33	159	0	0	0	0	0	973
	59.5	40.5	0	0		1.7	0	49.8	48.5		0	33.3	51.1	15.6		1.3	2	73	20.8		0	0	0	0		
PHF	.837	.732	000.	000	.904	.500	000	.891	.730	795	000	.750	.742	.583	.849	.500	.400	.829	635	.864	.000	.000	.000	000	000.	888.
Cars	237	160	0	0	397	4	0	114	108	226	0	09	90	28	178	2	80	114	33	157	0	0	0	0	0	958
% Cars	98.3	97.6	0	0	0.86	100	0	100	97.3	98.7	0	100	87.8	100	6.86	100	100	98.3	100	28.7	0	0	0	0	0	98.5
Trucks	4	4	0	0	00	0	0	0	3	က	0	0	2	0	2	0	0	7	0	2	0	0	0	0	0	15
% Trucks	1.7	2.4	0	0	2.0	0	0	0	2.7	1.3	0	0	2.2	0	1.	0	0	1.7	0	1.3	0	0	0	0	0	1.5

File Name 75860007 Site Code 75860007 Start Date 1/25/2018 Page No 3

N/S Street : Bremen Street E/W Street : Porter Street City/State : Boston, MA Weather : Clear

Porter St In 226 3 229 Out 379 6 385 Total 605 9 614 108 3 111 Right 4 0 4 Left 114 0 114 BrRt 0 0 0 Thru Right 28 0 28 Peak Hour Data Peak Hour Begins at 07:30 AM Total 603 13 616 160 Bremen St In 397 8 8 North 197 4 201 Out Out 206 5 000 Cars Trucks 000 Left 116 2 116 116 LIDH S 0 S 8 0 1997 33 Right 0 0 0 and and 12 Toher SI 01 727 2 159 1610T 721 2 2

Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1 Peak Hour for Each Approach Begins at:

	0	0	0	0	0	
	0	0	0	0	0	0
	0	0	0	0	0	0
	0	0	0	0	0	0
07:00 AM	0	0	0	0	0	0
	36	45	32	46 0	159	
	9	13	2	თ	33	20.8
	24	31	26	35	116	73
	ĸ		0	2	œ	c)
07 30 AM	-	0	_	0	2	1.3
	47	29	51	53 0 2	180	
	4		6	12		15.6
	23	4	24	3	92	51.1
	20	12	18	10	09	33.3
07:30 AM	0	0	0	0 10	0	0
	29	20	48	72	229	
	28	22	23	38	111	48.5
	31	27	24	32	114	49.8
	0	0	0	0	0	0
07:30 AM	0	1	-	7	4	1.7 0
	92	112	98	100	405	
	0	0	0	0	0	0
	0	0	0	0	0	0
	42	26	38	28	164	40.5
07:30 AM	53	26	9	72	241	59.5
	+0 mins.	+15 mins.	+30 mins.	+45 mins.	Total Volume	% App. Total

File Name : 75860007 Site Code : 75860007 Start Date : 1/25/2018 Page No : 5

			HdRt Int. Total	0 176	0 225	0 234	0 234	0 869	0 225	0 265	0 157	0 139	0 786	0 1655	0	0
			BrRt Hd	0	0	0	0	0	0	0	0	0	0	0	0	•
	Porter PI	ŧ	BrLt B	0	0	0	0	0	0	0	0	0	0	0	0	
	1	- 1	HdLt	0	0	0	0	0	0	0	0	0	0	0	0	
		1	Right	4	9	9	13	29	2	6	9	7	22	51	19.5	
	St		Thru	17	21	24	31	93	24	35	19	7	83	182	69.7	
	Porter St	From West	Left	00	4	2	_	18	0	2	_	~	4	22	8.4	
			HdLt	0	2	~	0	ന	~	0	-	~	m	9	2.3	
			Right	4	9	4	က	17	6	12	6	2	35	52	15.4	
4- Cars	n St	onth	Thru	20	28	23	4	85	24	29	21	17	91	176	52.1	
Groups Printed- Cars	Bremen St	From South	BrLt	4	4	20	12	09	48	10	က	19	20	110	32.5	
Group			Left	0	0	0	0	0	0	0	0	0	0	0	0	
			Right	9	32	27	22	66	23	36	24	12	95	194	47.9	
	ır St	From East	BrRt	21	32	31	27	17	24	32	17	17	06	201	49.6	
	Porter St	From	Thru	0	0	0	0	0	0	0	0	0	0	0	0	
			Left	0	m	0	—	4	-	2	2	_	9	10	2.5	
			HdRt	-	0	0	0	~	0	0	2	_	n	4	9.0	
	Bremen St	North	Thru Right	0	0	0	0	0	0	0	0	0	0	0	0	
	Brem			36	42	40	55	173	38	27	30	26	121	294	45.2	
			Left	33	35	23	52	176	58	71	22	26	177	353	54.2	
			Start Time	07:00 AM	07:15 AM	07:30 AM	07:45 AM	Total	08:00 AM	08:15 AM	08:30 AM	08:45 AM	Total	Grand Total	Apprch %	

File Name : 75860007 Site Code : 75860007 Start Date : 1/25/2018 Page No : 9

		Bremen St	en St			Porter St	St		do	Bremen St	St			Porter St	75.		,	Porter PI	<u>-</u>		
	4	From North	ž	å	4	From E	From East	ide: O	40	Prom South	Thai	Diah	+ 71	From West	1111	Bight	1	Rrit Brei	Bret	HAR	Int Total
Start Time	Lell		ה ה	בי מבי בי בי ב	ם כ	3 0	2 0	5		1 0	3 0	Tings.		- L				1 0			
07:00 AM	7	Π	0	0	0	0	>	>	0	>	0	0	>	>	-	0	0	>	5	0	י
07:15 AM	0	0	0	0	0	0	0	0	0	0	~	0	0	0	_	0	0	0	0	0	2
07:30 AM	0	2	0	0	0	0	0	_	0	0	0	0	0	0	0	0	0	0	0	0	က
07:45 AM		-	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2
Total	ო	က	0	0	0	0	0	+	0	o	÷	0	0	0	2	0	0	0	0	0	10
08:00 AM	2	0	0	0	0	0	0	0	0	0	0	0	0	0	7	0	0	0	0	0	4
08:15 AM	_	*	0	0	0	0	0	2	0	0	2	0	0	0	0	0	0	0	0	0	9
08:30 AM	0	-	0	0	0	0	0	4	0	0	-	0	0	0	T	0	0	0	0	0	4
08:45 AM	0	-	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	7
Total	က	က	0	0	0	0	0	n	0	0	m	0	0	0	т	0	0	0	0	0	15
Grand Total	9	9	0	0	0	0	0	4	0	0	4	0	0	0	2	0	0	0	0	0	25
Apprch %	20	50	0	0	0	0	0	100	0	0	100	0	0	0	100	0	0	0	0	0	
Total %	24	24	0	0	0	0	0	16	0	0	16	0	0	0	20	0	0	0	0	0	

N/S Street: Bremen Street E/W Street: Porter Street City/State: Boston, MA Weather: Clear

Groups Printed- Bikes Peds

File Name : 75860007 Site Code : 75860007 Start Date : 1/25/2018 Page No : 13

Bremen St From North	Si				Port	Porter St From Fast				Bremen St From South	n St outh			— ш	Porter St From West	St est			From	Porter PI From Northwest	est				
Thru Right HdRt Peds	1dRt	ш		Left Thru BrRt Right Peds	hru B	F. R.	tht Pe		Left Bri	Lt Thri	BrLt Thru Right Peds HdLt	t Ped	s HdL	\perp	Left Thru Right Peds HdLt	Right	Peds	HdLt		BrRt HdRt Peds	HdR! F		Exclu Total Inclut Total Int. Total	te Total In	t. Total
0 0	0		-	0	0	0	0	-	0	0	0 0		1 0	0	0	0	00	0	0	0	0	0		0	7
0		0	0	0	0	0	0	2	0	0	0 0		1 0	0	~	0	4	0	0	0	0	0	7	-	∞
0		0	0	0	0	0	0	7	0	0	0 0	0	0	0	0	0	က	0	0	0	0	0	9	0	9
0		0	~	0	0	0	0	-	0	0	0 0		0	0 0	0	0	7	0	0	0	0	0	4	0	4
0		0	2	0	0	0	0	9	0	0	0 0		8	0 0	-	0	17	0	0	0	0	0	28	-	29
0		0	0	0	0	0	0	0	0	0	0	0	_	0 0	0	0	_	0	0	0	0	0	2	0	2
0		0	0	0	0	0	0	_	0	0	0	0	0	0 0	0	0	2	0	0	0	0	0	9	0	9
0		0	2	0	0	0	0	0	0	0	0	0	0	0 0	_	0	2	0	0	0	0	0	4	-	ις
0		0	2	0	0	0	0	0	0	0	0	0	-	0 0	0	0	-	0	0	0	0	0	4	0	4
0		0	4	0	0	0	0	-	0	0	0	0	2	0 0	_	0	o	0	0	0	0	0	16	-	17
0		0	9	0	0	0	0	7	0	0	0	0	22	0	2	0	26	0	0	0	0	0	44	7	46
0		0		0	0	0	0		0	0	0	0		0	100	0		0	0	0	0				
0		0		0	0	0	0		0	0	0	0	_	0 0	100	0		0	0	0	0		95.7	4.3	

File Name : 75860007 Site Code : 75860007 Start Date : 1/25/2018 Page No : 14

N/S Street: Bremen Street E/W Street: Porter Street City/State: Boston, MA Weather: Clear

From East Porter St From North **Bremen St**

App. Total Int. Total HdRt From Northwest Porter PI BrR BrLi HALL App. Total Right From West Porter St 맫 Left H App. Total Right From South **Bremen St** BrL! Lef App. Total Right BrRt Thro Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1 Left App. Total Righ HdRt Start Time Left Thru

	0	0	0	0	0		000
	0	0	0	0	0	0	000.
	0	0	0	0	0	0	000.
	0	0	0	0	0	0	000
	0	0	0	0	0	0	000.
	0	-	0	0	-		,250
	0	0	0	0	0	0	000
	0	-	0	0	-	100	.250
	0	0	0	0	0	0	000.
	0	0	0	0	0	0	000
	0	0	0	0	0		000.
	0	0	0	0	0	0	000.
	0	0	0	0	0	0	000
	0	0	0	0	0	0	000.
	0	0	0	0	0	0	000
	0	0	0	0	0		000.
	0	0	0	0	0	0	0000
	0	0	0	0	0	0	000.
	0	0	0	0	0	0	000
	0	0	0	0	0	0	000
00 AM	0	0	0	0	0		000
is at 07:	0	0	0	0	0	0	000.
n Begir	0	0	0	0	0	0	000.
ersection	0	0	0	0	0	0	000
intire Int	0	0	0	0	0	0	PHF .000 .000 .000 .000
Peak Hour for Entire Intersection Begins at 07:00 AM	07:00 AM	07:15 AM	07:30 AM	07:45 AM	Total Volume	% App. Total	HHL BHL

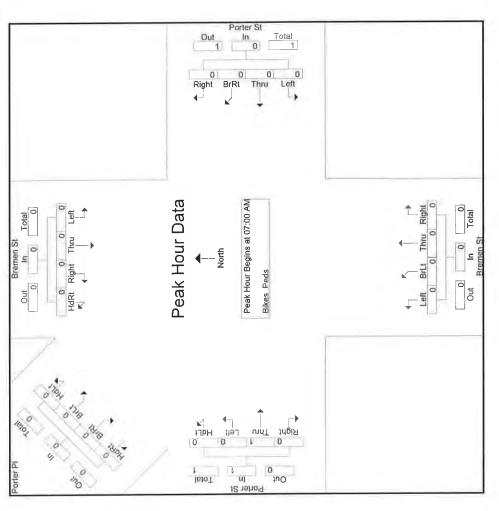
.250

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0

File Name 75860007 Site Code 75860007 Start Date 1/25/2018 Page No 15

N/S Street : Bremen Street E/W Street : Porter Street City/State : Boston, MA Weather : Clear



Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1

Peak Hour for Each Approach Begins at:

0	07:00 AM				0	7:00 AM				J	7:00 AM					07:00 AM					07:00 AM				
+0 mins.	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
+15 mins.	0	0	0	0	0	0	0	0	0	0	0 0	0	0	0	0	0 0	0	_	0	_	0	0	0	1 0 0 0 0 0	0
+30 mins.	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
+45 mins.	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
otal Volume	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0 0 1 0 1	0	~	0	-	0	0	0	0	0
) -	0	0	0	0		0	0	0	0		0	0	0	0		0	0	100	0		0	0 0 0	0	0	

File Name : 75860007 Site Code : 75860007 Start Date : 1/25/2018 Page No : 1

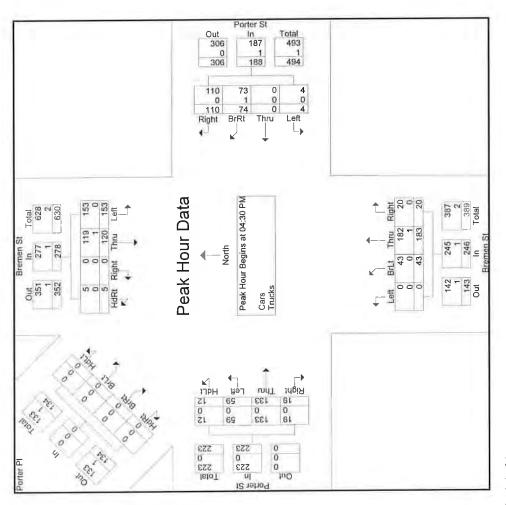
N/S Street: Bremen Street E/W Street: Porter Street City/State: Boston, MA Weather: Clear

File Name 75860007 Site Code 75860007 Start Date 1/25/2018 Page No 2

	Int. Total			247	237	239	212	935		.946	932	2.66	က	0.3
	App. Total			0	0	0	0	0		000	0	0	0	0
l west	HdRI			0	0	0	0	0	0	000	0	0	0	0
Porter Pl From Northwest	BrRt			0	0	0	0	0	0	000	0	0	0	0
From	BrLI			0	0	0	0	0	0	000	0	0	0	0
	HdLI			0	0	0	0	0	0	000	0	0	0	0
	pp. Total			22	71	48	49	223		.785	223	100	0	0
, t	Right App. Total			4	2	က	7	19	8.5	629.	19	100	0	0
Porter St From West				35	38	34	56	133	9.69	.875	133	100	0	0
모 문	Left			16	25	∞	10	29	26.5	.590	29	100	0	0
	HdLt			0	က	ന	9	12	5.4	.500	12	100	0	0
	App. Total			73	47	20	99	246		.842	245	9.66	_	0.4
+ 5	Right A			7	9	2	5	20	8.1	.714	20	100	0	0
Bremen St From South	J. Phr.			55	29	22	42	183	74.4	.803	182	99.5	-	0.5
P. P.	BrLt			=======================================	12	7	6	43	17.5	968.	43	100	0	0
	Left			0	0	0	0	0	0	000	0	0	0	0
	pp. Total			53	49	4	45	188		788.	187	99.5	~	0.5
	Right App. Total			30	28	24	28	110	58.5	.917	110	100	0	0
Porter St From East	BrRt			22	19	17	16	74	39.4	.841	73	98.6	~	4.1
٠ F	Thru			0	0	0	0	0	0	000	0	0	0	0
	Left	k 1 of 1		~	2	0	-	4	2.1	.500	4	100	0	0
	op. Total	M - Pea	30 PM	99	20	80	62	278		869	277	9.66	_	4.0
, c	IdRt AF	05:45 P	s at 04:	8	-	0	2	2	1.8	.625	2	100	0	0
Bremen St From North	Right F	PM to	n Begin	0	0	0	0	0	0	000	0	0	0	0
Bre	Thru	m 04:00	ersectio	24	33	35	28	120	43.2	.857	119	99.2	—	8.0
	Left	ysis Fro.	ntire Inte	40	36	45	32	153	55	.850	153	100	0	0
	Start Time Left Thru Right HdRt App. Tolal	Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1	Peak Hour for Entire Intersection Begins at 04:30 PM	04:30 PM	04:45 PM	05:00 PM	05:15 PM	Total Volume	% App. Total	PHF	Cars	% Cars	Trucks	% Trucks

File Name : 75860007 Site Code : 75860007 Start Date : 1/25/2018 Page No : 3

N/S Street : Bremen Street E/W Street : Porter Street City/State : Boston, MA Weather : Clear



Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1

Peak Hour for Each Approach Begins at:

		04 30 PM				04:30 PM	-			ò	04:00 PM			
1 0		0	11 5	55 7	73	0 8	16	35	4	22	0	0	0	0
66 2 0 19	28 49	0	12 2	29 6	4	7 3	25	38	2	7.1	0	0	0	0
70 0 0 17	24	0	11 5	57 2	2 70	3	00	34	3	48	0	0	0	0
0	3 28 45	0	9	42 5		9	10	56	7	49	0	0	0	0
4 0 74	110 188	0	43 183	3 20	246	5 12	59	133	19	223	0	0	0	0
2.1 0 39.4	58.5	0 17	17.5 74.4	4 8.1		5.4	26.5	9.69	8.5		0	0	0	0

File Name : 75860007 Site Code : 75860007 Start Date : 1/25/2018 Page No : 5

									Group	Groups Printed- Cars	L Cars										
		Bremen St	St			Porter St	.St			Bremen St	n St			Porter St	St			Porter Pl	PI		
i L	40	From N	orth	į.	40	From East	ast PrD+	Diah	Ho.	Brit Th	Thri	Picht	÷	I off Th	Thru	Richt	±	Brit BrR	BrRt	HAR	Int Total
04:00 PM	27	25	0	0	3	0	4	26	0	9	42	4	2	12	21	4	0	0	0	0	186
04:15 PM	43	40	0	0	~	0	19	20	0	∞	28	9	4	17	22	7	0	0	0	0	210
04:30 PM	40	24	0	7	_	0	22	30	0	1	55	7	0	16	35	4	0	0	0	0	247
04:45 PM	36	33	0	_	7	0	18	28	0	12	59	9	ო	25	38	S.	0	0	0	0	236
Total	146	122	0	က	7	0	73	104	0	37	154	23	o	70	116	15	0	0	0	0	879
05:00 PM	45	35	0	0	0	0	17	24	0	-	27	2	m	œ	34	n	0	0	0	0	239
05:15 PM	32	27	0	2	-	0	16	28	0	6	4	r.	9	10	26	7	0	0	0	0	210
05:30 PM	37	37	0	_	0	0	16	22	0	12	31	2	-	12	32	10	0	0	0	0	213
05:45 PM	37	33	0	-	2	0	=	20	0	17	19	2	2	∞	37	n	0	0	0	0	189
Total	151	132	0	4	m	0	09	94	0	43	148	-	15	38	129	23	0	0	0	0	851
Grand Total	297	254	0	7	10	0	133	198	0	80	302	34	24	108	245	38	0	0	0	0	1730
Apprch %	53.2	45.5	0	1,3	2.9	0	39	58.1	0	19.2	72.6	8.2	5.8	26	59	9.2	0	0	0	0	
Total %	17.2	14.7	0	4.0	9.0	0	7.7	11.4	0	4.6	17.5	2	1.4	6.2	14.2	2.2	0	0	0	0	

File Name : 75860007 Site Code : 75860007 Start Date : 1/25/2018 Page No : 9

									Groups	Groups Printed- Trucks	Trucks										
		Bremen St	n St			Porter St	S.			Bremen St	ıSt			Porter St	St			Porter Pl	Ы		
		From North	lorth			From East	ast			From South	outh			From West	est		L.	From Northwest	thwest		
Start Time	Left	Thru	Right	HdRt	Left	Thru	BrRt	Right	Left	BrLt	Thu	Right	HdLt	Leff	Thru	Right	보다	BrLt	BrRt	HdRt	Int, Total
04:00 PM	0	0	0	0	0	0	0	_	0	0	0	0	0	~	0	0	0	0	0	0	2
04:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	-	0	0	0	0	0	-
04:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
04:45 PM	0	0	0	0	0	0	~	0	0	0	0	0	0	0	0	0	0	0	0	0	_
Total	0	0	0	0	0	0	-	-	0	0	0	0	0	-	-	0	0	0	o	0	4
05:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
05:15 PM	0	~	0	0	0	0	0	0	0	0	٢	0	0	0	0	0	0	0	0	0	2
05:30 PM	+	~	0	0	0	0	-	-	0	0	0	0	0	0	0	0	0	0	0	0	4
05:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Total	-	2	0	0	0	0	-	*	0	0		0	0	0	0	0	0	0	0	0	9
Grand Total	~	2	0	0	0	0	2	2	0	0	~	0	0	7-	—	0	0	0	0	0	10
Apprch %	33.3	66.7	0	0	0	0	90	20	0	0	100	0	0	20	20	0	0	0	0	0	
Total %	10	20	0	0	0	0	20	20	0	0	10	0	0	10	10	0	0	0	0	0	

File Name : 75860007 Site Code : 75860007 Start Date : 1/25/2018 Page No : 13

N/S Street: Bremen Street E/W Street: Porter Street City/State: Boston, MA Weather: Clear

Groups Printed- Bikes Peds

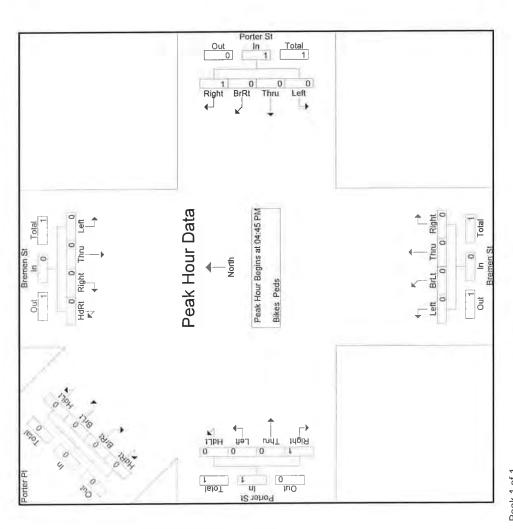
)	2		2												
		Bře	Bremen St	; <u>t</u>			ፈ	Porter St				Bren	Bremen St				Port	Porter St				Porter PI	_				
		Fro	From North	÷			Fre	From East				From	From South	-			From West	West			Fro	From Northwest	west				
Start Time	Left	Thru	Right	Left Thru Right HdRt Peds	Peds	Left	Left Thru BrRt Right Peds	BrRt	Right		Left	BrLt Thru Right Peds	hru R	Right F		HdLt L	eff Th	Left Thru Right Peds	ht Pec	Is HdLt		BrLt BrRt HdRt Peds	HdRt	Peds	Erctu Total Inclu Total Int. Total	Inch. Tetal	Int. Total
04:00 PM	0	0	0	0	_	0	0	0	0	-	0	0	0	0	-	0	0	0	0	0	0	0	0	0	7	0	7
04:15 PM	0	0	0	0	~	0	0	0	0	_	0	0	0	-	7	0	0	0	0	4 0	0	0	0	0	80	-	0
04:30 PM	0	0	0	0	2	0	0	0	0	S	0	0	0	0	4	0	0	0	0	3 0	0	0	0	0	14	0	4
04:45 PM	0	0	0	0	0	0	0	0	0	9	0	0	0	0	9	0	0	0	0	2 0	0	0	0	0	4	0	4
Total	0	0	0	0	4	0	0	0	0	5	0	0	0	-	13	0	0	0	0	17 0	0 0	0	0	0	47	-	84
05:00 PM	0	0	0	0	2	0	0	0	0	_	0	0	0	0	0	0	0	0	0	6	0	0	0	0	12	0	12
05:15 PM	0	0	0	0	0	0	0	0	0	က	0	0	0	0	0	0	0	0	_	4	0 0	0	0	0	7	-	60
05:30 PM	0	0	0	0	_	0	0	0		-	0	0	0	0	_	0	0	0	0	9	0 0	0 0	0	0	6	←	10
05:45 PM	0	0	0	0	ო	0	0	0	0	4	0	0	0	0	-	0	0	0	0	9	0 0	0	0	0	7	0	1
Total	0	0	0	0	9	0	0	0	_	O	0	0	0	0	2	0	0	0	-	22 (0 0	0	0	0	39	2	41
Grand Total	0	0	0	0	10	0	0	0	-	22	0	0	0	~	15	0	0	0	-	39 () C	0 0	0	0	86	က	89
Apprch %	0	0	0	0		0	0	0	100		0	0	0	100		0	0	0	100		0	0 0	0				
Total %	0	0	0	0		0	0	0	33.3		0	0	0	33.3		0	0	0 33	33.3	_	0	0 0	0		96.6	3.4	

File Name : 75860007 Site Code : 75860007 Start Date : 1/25/2018 Page No : 14

N/S Street: Bremen Street E/W Street: Porter Street City/State: Boston, MA Weather: Clear

-	. –	Bremen St From North	s St orth	,		Po	Porter St From East				Brem From	Bremen St From South				Porter St From West	St 'est			Fror	Porter Pi From Northwest	west		
Start Time Left Thru	Thru	Righ	HdRt	App Total	Leff	The	BrRt	Right App. Total		Left	BrL1 Th	Thru Right	jht App. Total	Halit	Left	Thru	Right	App. Total	HdLt	BrLt	BrRt	HdRt	App. Total Int. Total	Int, Tota
Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1	-rom 04	1:00 PM	to 05:45	PM - Pe	ak 1 of 1																			
Peak Hour for Entire Intersection Begins at 04:45 PM	Interse	ction Be	gins at 04	4:45 PM																				
04.45 PM 0	0	0	0	0	0	0	0	0	0	0	0	0) 0	0 0)	0	0	0	0	0	0	0	0	
05:00 PM 0	0	0	0	0	0	0	0	0	0	0	0	0) 0	0 0	-	0	0	0	0	0	0	0	0	
05:15 PM 0	0	0 (0	0	0	0	0	0	0	0	0	0	0	0	5	0	-	7	0	0	0	0	0	
05:30 PM 0	5	0 (0	0	0	0	0	-		0	0	0	0	0	_	0	0	0	0	0	0	0	0	
0 ominov letoT	0	0 (0	0	0	0	0	-	-	0	0	0	0	0 0	٥	0	-	1	0	0	0	0	0	2
	0) 0	0		0	0	0	100		0	0	0	0	J	_	0	100		0	0	0	0		
PHF .000	000.	000. (000. 000. 000. 000.		000. 000. 000. 000.	.000		.250 .2	.250 .0	000.	000.	000.	.000. 000.	000. 0	000.	000.	.250	.250	000.	000	000.	000.	000	.500

File Name : 75860007 Site Code : 75860007 Start Date : 1/25/2018 Page No : 15



Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1 Peak Hour for Each Approach Begins at:

	04:00 PM				ر	04 45 PM				_	04:00 PM					04:30 PM				_	04.00 PM				
+0 mins.	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	_
+15 mins.	0	0	0	0	0	0	0	0	0	0	0	0	0	-	τ-	0	0	0	0	0	0	0	0	0)
+30 mins.	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	J
+45 mins.	0	0	0	0	0	0	0	0	-	-	0	0	0	0	0	0	0	0	-	-	0	0	0	0	J
Total Volume	0	0	0	0	0	0	0	0	,-	+	0	0	0	-	-	0	0	0	-	-	0	0	0	0	
-4-F 4 %	0	0	0	0		0	0	0	100		0	0	0	100		0	0	0	100		0	0	0	0	

N/S Street : Cottage Street E/W Street : Porter Street City/State : Boston, MA Weather : Clear

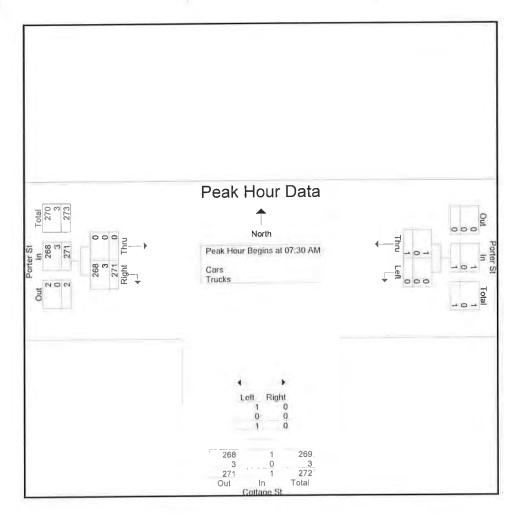
File Name : 75860008 Site Code : 75860008 Start Date : 1/25/2018 Page No : 1

Groups Printed- Cars - Trucks

		Porter St From West		Cottage St From South		Porter St From East	
Int. Total	Right	Thru	Right	Left	Thru	Left Last	Start Time
40	40	0	0	0	0	0	07:00 AM
40	40	0	0	0	0	0	07:15 AM
52	50	0	0	1	1	0	07:30 AM
73	73	0	0	0	0	0	07:45 AM
205	203	0	0	1	1	0	Total
75	75	0	o	0	0	0	08:00 AM
73	73	0	0	0	0	0	08:15 AM
19	19	0	0	0	0	0	08:30 AM
22	22	0	0	0	0	0	08:45 AM
189	189	0	0	0	0	0	Total
394	392	0	0	1	1	0	Grand Total
	100	0	0	100	100	0	Apprch %
	99.5	0	0	0.3	0.3	0	Total %
387	385	0	0	1	1	0	Cars
98.2	98.2	0	0	100	100	0	% Cars
7	7	0	0	0	0	0	Trucks
1.8	1.8	0	0	0	0	0	% Trucks

N/S Street : Cottage Street E/W Street : Porter Street City/State : Boston, MA Weather : Clear File Name : 75860008 Site Code : 75860008 Start Date : 1/25/2018 Page No : 2

		Porter St From East			Cottage St From South		ı	Porter St From West		
Start Time	Left	Thru	App. Total	Left	Right	App. Total	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 0	7:00 AM to 0	8:45 AM - F	Peak 1 of 1							
Peak Hour for Entire Interse	ction Begins	at 07:30 A	М							
07:30 AM	0	1	1	1	0	1	0	50	50	52
07:45 AM	0	Ö	0	0	0	0	0	73	73	73
08:00 AM	0	0	0	0	0	0	0	75	75	75
08:15 AM	0	0	0	0	0	0	0	73	73	73
Total Volume	0	1	1	1	0	1	0	271	271	273
% App. Total	0	100		100	0		0	100		
PHF	.000	.250	.250	.250	.000	.250	,000	.903	.903	.910
Cars	0	1	1	1	0	1	0	268	268	270
% Cars	0	100	100	100	0	100	0	98.9	98.9	98.9
Trucks	0	0	0	0	0	0	0	3	3	3
% Trucks	0	0	0	0	0	0	0	1.1	1.1	1.1



N/S Street : Cottage Street E/W Street : Porter Street City/State : Boston, MA Weather : Clear

File Name 75860008 Site Code 75860008 Start Date 1/25/2018 Page No 4

Groups	Printed- Cars
	Cottage St

			Groups Printed- (Cars			
	Porter From E	St Sast	Cotta From	South	From	er St West	
Start Time	Left	Thru	Left	Right	Thru	Right	Int. Total
07:00 AM	0	0	0	0	0	37	37
07:15 AM	0	0	0	0	0	39	39
07:30 AM	0	1	1	0	0	50	52
07:45 AM	0	0	0	0	0	72	72
Total	0	1	1	0	0	198	200
		0	o	0	0	73	73
08:00 AM	0	U	0	U	U	7.5	,,,
08:15 AM	0	0	0	0	0	73	73
08:30 AM	0	0	0	0	0	19	19
08:45 AM	0	0	0	0	0	22	22
Total	0	0	0	0	0	187	187
Grand Total	0	1	1	0	0	385	387
Apprch %	0	100	100	0	0	100	
	0	0.3	0.3	0	0	99.5	
Total %	U	0.5	0.5	•		00.0	L

N/S Street : Cottage Street E/W Street : Porter Street City/State : Boston, MA Weather : Clear

File Name : 75860008 Site Code : 75860008 Start Date : 1/25/2018 Page No : 7

	Groups Printed- Trucks Porter St Cottage St Porter St												
		From West		From South		Porter St From East							
Int. Total	Right	Thru	Right	Left	Thru	Left	Start Time						
3	3	0	0	0	0	0	07:00 AM						
1	1	0	0	0	0	0	07:15 AM						
0	0	0	О	0	0	0	07:30 AM						
1	1	0	0	0	0	0	07:45 AM						
5	5	0	0	0	0	0	Total						
2	2	0	٥	0	0	0	08:00 AM						
0	0	0	0	0	0	0	08:15 AM						
C	0	0	0	0	0	0	08:30 AM						
C	0	0	0	0	0	0	08:45 AM						
2	2	0	0	0	0	0	Total						
7	7	0	0	0	0	0	Grand Total						
	100	0	0	0	О	0	Apprch %						
	100	0	О	0	0	0	Total %						

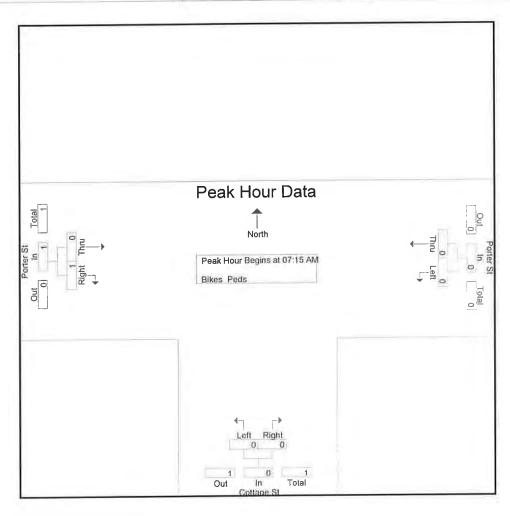
N/S Street : Cottage Street E/W Street : Porter Street City/State : Boston, MA Weather : Clear

File Name : 75860008 Site Code : 75860008 Start Date : 1/25/2018 Page No : 10

					Groups Prin	ted- Bikes	s Peds					
	Porter St From East			C	ottage St om South			Porter St From West				
Start Time	Left	Thru	Peds	Left	Right	Peds	Thru	Right	Peds	Exclu. Total	Inclu. Total	Int. Total
07:00 AM	0	0	6	0	0	4	0	0	1	11	0	11
07:15 AM	0	0	0	0	0	1	0	0	1	2	0	2
07:30 AM	0	0	2	0	0	0	0	0	0	2	0	2
07:45 AM	0	0	3	0	0	5	0	0	0	8	0	8
Total	0	0	11	0	0	10	0	0	2	23	0	23
			41			Y				1	400	
08:00 AM	0	0	2	0	0	2	0	1	2	6	1	7
08:15 AM	0	0	0	0	0	1	0	0	2	3	0	3
08:30 AM	0	0	0	0	0	4	0	0	1	5	0	5
08:45 AM	0	0	0	0	0	0	0	0	0	0	0	0
Total	0	0	2	0	0	7	0	1	5	14	1	15
						9	_			0.7	92	38
Grand Total	0	0	13	0	0	17	0	1	7	37	1	30
Apprch %	0	0		0	0		0	100				
Total %	0	0		0	0		0	100		97.4	2,6	

N/S Street : Cottage Street E/W Street : Porter Street City/State : Boston, MA Weather : Clear File Name : 75860008 Site Code : 75860008 Start Date : 1/25/2018 Page No : 11

		Porter St From East			Cottage St From South			Porter St From West		
Start Time	Left	Thru	App. Total	Left	Right	App. Total	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 07	7:00 AM to 08	B:45 AM - F	Peak 1 of 1		-					
Peak Hour for Entire Interse	ction Begins	at 07:15 Al	M							
07:15 AM	0	0	0	0	0	0	0	0	0	(
07:30 AM	0	0	0	0	0	0	0	0	0	(
07:45 AM	0	0	0	0	0	0	0	0	0	(
08:00 AM	0	0	0	0	0	0	0	1	1	
Total Volume	0	0	0	0	0	0	0	1	1	
% App. Total	0	0		0	0		0	100		
PHF	.000	.000	.000	.000	.000	.000	.000	.250	.250	.250



N/S Street : Cottage Street E/W Street : Porter Street City/State : Boston, MA Weather : Clear

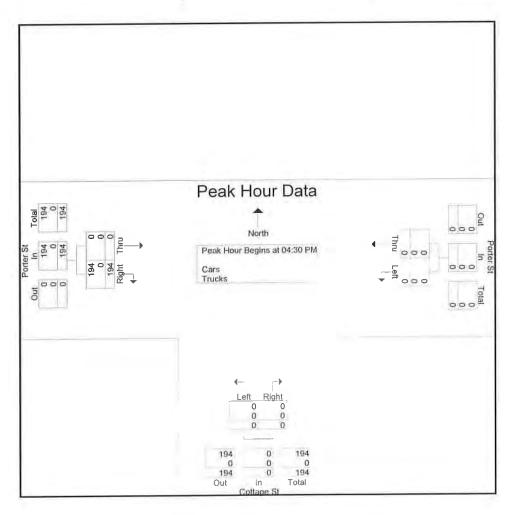
File Name: 75860008 Site Code: 75860008 Start Date: 1/25/2018 Page No: 1

Groups Printed- Cars - Trucks

		Porter St		Cottage St		Porter St	
let Tete	Di-la	From West	D: 11	From South		From East	
Int. Tota	Right	Thru	Right	Left	Thru	Left	Start Time
41	41	0	0	0	0	0	04:00 PM
39	39	0	0	0	0	0	04:15 PM
52	52	0	0	0	0	0	04:30 PM
51	51	0	0	0	0	0	04:45 PM
183	183	0	0	0	0	0	Total
50	50	0	0	0	О	0	05:00 PM
41	41	0	0	0	О	0	05:15 PM
42	42	0	0	0	0	0	05:30 PM
38	38	0	0	0	0	0	05:45 PM
171	171	0	0	0	0	0	Total
354	354	0	0	0	0	0	Grand Total
	100	0	0	0	0	0	Apprch %
	100	0	0	0	0	0	Total %
353	353	0	0	0	0	0	Cars
99.7	99.7	0	0	0	0		
	33.1					0	% Cars
1	1	0	0	0	0	0	Trucks
0.3	0.3	0	0	0	0	0	% Trucks

N/S Street : Cottage Street E/W Street : Porter Street City/State : Boston, MA Weather : Clear

		Porter St From East			Cottage St From South			Porter St From West		
Start Time	Left	Thru	App. Total	Left	Right	App. Total	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 04	1:00 PM to 0	5:45 PM - F	Peak 1 of 1							
Peak Hour for Entire Interse	ction Begins	at 04:30 Pl	M							
04:30 PM	0	0	0	0	0	0	0	52	52	52
04:45 PM	0	0	0	0	0	0	0	51	51	51
05:00 PM	0	0	0	0	0	0	0	50	50	50
05:15 PM	0	0	0	0	0	0	0	41	41	41
Total Volume	0	0	0	0	0	0	0	194	194	194
% App. Total	0	0		0	0		0	100		
PHF	.000	.000	-000	.000	.000	.000	-000	_933	.933	.933
Cars	0	0	0	0	0	0	0	194	194	194
% Cars	0	0	0	0	0	0	0	100	100	100
Trucks	0	0	0	0	0	0	0	0	0	0
% Trucks	0	0	0	0	0	0	0	0	0	0



N/S Street : Cottage Street E/W Street : Porter Street City/State : Boston, MA Weather : Clear

File Name : 75860008 Site Code : 75860008 Start Date : 1/25/2018 Page No : 4

		Grou	ups Printed- Cars				
	Porter St From East		Cottage St From South	ı	Porter St From West		
Start Time	Left	Thru	Left	Right	Thru	Right	Int. Total
04:00 PM	0	0	0	0	0	41	41
04:15 PM	0	0	0	0	0	38	38
04:30 PM	0	0	0	0	0	52	52
04:45 PM	0	0	0	0	0	51	51
Total	0	0	0	0	0	182	182
05:00 PM	0	0	0	О	0	50	50
05:15 PM	0	0	0	0	0	41	41
05:30 PM	0	0	0	0	0	42	42
05:45 PM	0	0	0	0	0	38	38
Total	0	0	0	0	0	171	171
Grand Total	0	0	0	0	0	353	353
Apprch %	0	0	0	0	0	100	
Total %	0	0	0	0	0	100	

N/S Street : Cottage Street E/W Street : Porter Street City/State : Boston, MA Weather : Clear

		Porter St		s Printed- Trucks Cottage St	7	Porter St	
		From West		From South		From East	
Int. Total	Right	Thru	Right	Left	Thru	Left	Start Time
0	0	0	0	0	0	0	04:00 PM
1	1	0	0	0	0	0	04:15 PM
0	0	0	0	0	0	0	04:30 PM
0	0	0	0	0	0	0	04:45 PM
1	1	0	0	0	0	0	Total
			. T	_	_1		
0	0	0	0	0	0	0	05:00 PM
0	0	0	0	0	0	0	05:15 PM
C	0	0	0	0	0	0	05:30 PM
C	0	0	0	0	0	0	05:45 PM
C	0	0	0	0	0	0	Total
1	1	0	0	0	0	0	Grand Total
	100	0	0	0	0	0	Apprch %
	100	0	0	0	0	0	Total %

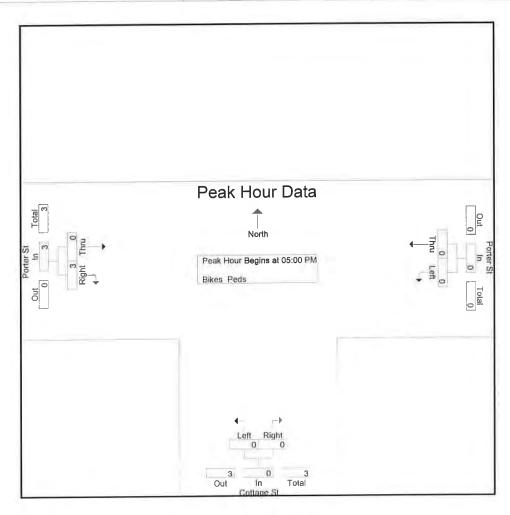
N/S Street : Cottage Street E/W Street : Porter Street City/State : Boston, MA Weather : Clear

Groups	Printed-	Bikes	Peds
--------	----------	-------	------

		orter St om East		Co Fro	ottage St om South		F	Porter St rom West				
Start Time	Left	Thru	Peds	Left	Right	Peds	Thru	Right	Peds	Exclu. Total	Inclu. Total	Int. Total
04:00 PM	0	0	0	0	0	7	0	0	0	7	0	7
04:15 PM	0	0	0	0	0	4	0	0	1	5	0	5
04:30 PM	0	0	0	0	0	8	0	0	2	10	0	10
04:45 PM	0	0	0	0	0	1	0	0	2	3	0	3
Total	0	0	0	0	0	20	0	0	5	25	0	25
05:00 DM	0	0	0	0	0	5	0	1	1	6	1	7
05:00 PM	0	0	0	U	U	3	· ·	'	•		•	•
05:15 PM	0	0	0	0	0	7	0	0	0	7	0	7
05:30 PM	0	0	0	0	0	4	0	1	0	4	1	5
05:45 PM	0	0	1	0	0	2	0	1	2	5	1	6
Total	0	0	1	0	0	18	0	3	3	22	3	25
Grand Total	0	0	1	0	0	38	0	3	8	47	3	50
Apprch %	0	0	<i>a</i>	0	0		0	100				
Total %	0	0		0	0		0	100		94	6	

N/S Street : Cottage Street E/W Street : Porter Street City/State : Boston, MA Weather : Clear

		Porter St From East			Cottage St From South			Porter St From West		
Start Time	Left	Thru	App. Total	Left	Right	App. Total	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 04	:00 PM to 0	5:45 PM - F	Peak 1 of 1							
Peak Hour for Entire Interse	ction Begins	at 05:00 P	M							
05:00 PM	0	0	0	0	0	0	0	1	1	1
05:15 PM	0	0	0	0	0	0	0	0	0	C
05:30 PM	0	0	0	0	0	0	0	1	1	1
05:45 PM	0	0	0	0	0	0	0	1	1	1
Total Volume	0	0	0	0	0	0	0	3	3	3
% App. Total	0	0		0	0		0	100		
PHF	.000	.000	.000	.000	.000	.000	.000	.750	.750	,750



File Name : 75860009 Site Code : 75860009 Start Date : 1/25/2018 Page No : 1

					The second secon	The state of the s							
	O II	Orleans St From North		∑⊔	Maverick St From East		O E	Orleans St From South		May	Maverick St From West		
Start Time	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Int. Total
07:00 AM	0	2	ις.	က	51	7	10	4	0	0	0	0	82
07:15 AM	0	7	9	က	63	5	14	23	0	0	0	0	121
07:30 AM	0	7	2	2	99	2	1	17	0	0	0	0	102
07:45 AM	0	7	9	2	62	2	6	4	0	0	0	0	107
Total	0	23	19	10	232	56	44	28	0	0	0	0	412
08:00 AM	0	7	4	Ŋ	92	o o	7	16	0	0	0	0	124
08:15 AM	0	o	2	2	88	1	9	15	0	0	0	0	138
08:30 AM	0	00	Ø	2	47	4	4	10	-	0	0	0	84
08:45 AM	0	7	80	~	44	т	10	7	0	0	0	0	80
Total	0	31	27	10	255	27	27	48	+	0	0	0	426
Grand Total	0	54	46	20	487	53	7.1	106	-	0	0	0	838
Арртсһ %	0	54	46	3.6	87	9.5	39.9	59.6	9.0	0	0	0	
Total %	0	6.4	5.5	2.4	58.1	6.3	8.5	12.6	0.1	0	0	0	
Cars	0	53	44	20	472	51	71	105	-	0	0	0	817
% Cars	0	98.1	95.7	100	6.96	96.2	100	99.1	100	0	0	0	97.5
Trucks	0	_	2	0	15	2	0	•	0	0	0	0	21
% Trucks	0	1.9	4.3	0	3.1	00	<u>_</u>	6.0	_	C	_	C	2.5

File Name 75860009 Site Code 75860009 Start Date 1/25/2018 Page No 2

N/S Street: Orleans Street E/W Street: Maverick Street City/State: Boston, MA Weather: Clear

Int. Total Right App. Total Maverick St From West Thru Left Right App. Total From South Orleans St Thru Left Right App. Total Maverick St From East Thru Left Thru Right App. Total Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1 From North Orleans St Left Start Time

Peak Hour for Entire Intersection Begins at 07:30 AM	tersection	Begins at	07:30 AM												
07:30 AM	0	7	2	0	2	56	7	99	7	17	0	28	0	0	0
07:45 AM	0	7	9	13	2	62	7	71	6	14	0	23	0	0	0
08:00 AM	0	7	4	17	5	9/	6	06	7	16	0	23	0	0	0
08:15 AM	0	6	7	16	2	88	1	101	9	15	0	21	0	0	0
Total Volume	0	30	19	49	1	282	34	327	33	62	0	92	0	0	0
% App. Total	0	61.2	38.8		3.4	86.2	10.4		34.7	65.3	0		0	0	0
PHF	000	.833	629.	992.	.550	.801	.773	808	.750	.912	000	.848	000	000.	000
Cars	0	29	17	46	11	275	33	319	33	61	0	94	0	0	0
% Cars	0	2.96	89.5	93.9	100	97.5	97.1	97.6	100	98.4	0	98.9	0	0	0
Trucks	0	~	2	က	0	7	~	80	0	+	0	_	0	0	0
% Trucks	0	3.3	10.5	6.1	0	2.5	2.9	2.4	0	1.6	0	1.1	0	0	0

97.5

12

0 0 0

.853 459

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102

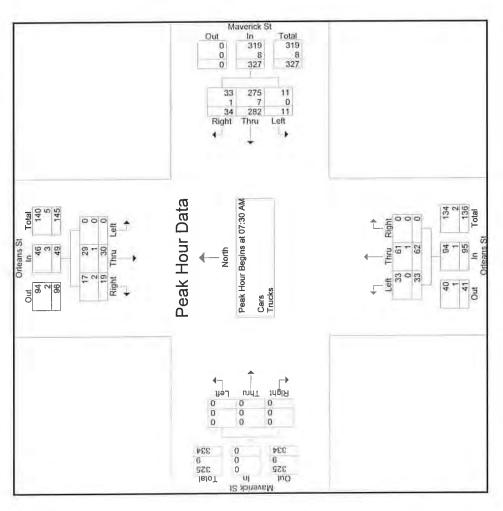
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File Name 75860009 Site Code 75860009 Start Date 1/25/2018 Page No 3

N/S Street: Orleans Street E/W Street: Maverick Street City/State: Boston, MA Weather: Clear



Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1

Peak Hour for Each Approach Begins at:

-	08:00 AM			0	07:30 AM				07:15 AM			_	07:00 AM			
+0 mins.	0	7	4	7	2	26	7	65			0	37	0	0	0	0
+15 mins.	0	o	7	16	2	62	7	74	11	17	0	28	0	0	0	0
+30 mins.	0	œ	œ	16	ĸ	9/	o	06			0	23	0	0	0	0
+45 mins.	0	7	œ	15	2	88	11	101			0	23	0	0	0	0
otal Volume	0	31	27	28	7	282	34	327			0	111	0	0	0	0
% App. Total	0	53.4	46.6		3.4	86.2	10.4				0		0	0	0	

File Name : 75860009 Site Code : 75860009 Start Date : 1/25/2018 Page No : 5

	C	Orleans St		Ž	Maverick St	Groups Filliged- Cars		deans St		Σ	Maverick St		
		rom North		ш	From East		도	From South		ű	From West		
Start Time	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Int, Total
07:00 AM	0	2	Ŋ	က	49	7	10	4	0	0	0	0	80
07:15 AM	0	7	9	3	09	S	14	23	0	0	0	0	118
07:30 AM	0	7	2	2	55	9	11	17	0	0	0	0	100
07:45 AM	0	7	5	2	58	2	o	14	0	0	0	0	102
Total	0	23	0	10	222	25	44	58	0	0	0	0	400
08:00 AM	0	9	4	S	75	6	7	16	0	0	0	0	122
08:15 AM	0	6	9	8	87	7	9	14	0	0	0	0	135
08:30 AM	0	œ	00	2	46	e	4	10	-	0	0	0	82
08:45 AM	0	7	æ	₩.	42	ю	10	7	0	0	0	0	78
Total	0	30	26	10	250	56	27	47	+	0	0	0	417
Grand Total	0	53	44	20	472	51	7.1	105	-	0	0	0	817
Apprch %	0	54.6	45.4	3.7	86.9	9.4	40.1	59.3	9.0	0	0	0	
Total %	0	6.5	5.4	2.4	57.8	6.2	8.7	12.9	0.1	0	0	0	

File Name : 75860009 Site Code : 75860009 Start Date : 1/25/2018 Page No : 9

					Group	Groups Printed- Trucks							
	0 [Orleans St From North		Ž Ē	Maverick St From East			Orleans St From South		≥ Щ	Maverick St From West		
Start Time	Teff	Thu	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Int. Total
07:00 AM	0	0	0	0	2	0	0	0	0	0	0	0	2
07:15 AM	0	0	0	0	က	0	0	0	0	0	0	0	е
07:30 AM	0	0	0	0	-	_	0	0	0	0	0	0	2
07:45 AM	0	0	7	0	4	0	0	0	0	0	0	0	5
Total	0	0	-	0	10	÷	0	0	0	0	0	0	12
08:00 AM	0	~	0	0	-	0	0	0	0	0	0	0	2
08:15 AM	0	0	~	0	+	0	0	~	0	0	0	0	ю
08:30 AM	0	0	0	0	٣	-	0	0	0	0	0	0	2
08:45 AM	0	0	0	0	2	0	0	0	0	0	0	0	2
Total	0	*	•	0	5	+	0	-	0	0	0	0	6
Grand Total	0	~	2	0	15	2	0	-	0	0	0	0	21
Apprch %	0	33.3	2.99	0	88.2	11.8	0	100	0	0	0	0	
Total %	0	4.8	9.5	0	71.4	9.5	0	4.8	0	0	0	0	

File Name 75860009 Site Code 75860009 Start Date 1/25/2018 Page No 13

		Orleans St From North	s St lorth			Maverick St From East	k St ast			Orleans St From South	s St outh			Maverick St From West	k St lest				
Start Time	Left	Thru	Right	Peds	Left	Thru	Right	Peds	Left	Thru	Right	Peds	Left	Thru	Right	Peds	Exclu. Total Inclu. Total	Inclu. Tota	Int. Total
07:00 AM	+	0	0	10	0	0	0	7	0	0	0	24	0	0	0	2	46		
07:15 AM	0	0	(+ i	#	0	0	0	4	0	0	0	17	0	0	0	m	35		
07:30 AM	0	**	0	15	0	0	0	ιΩ	0	0	0	18	0	0	0	1	49		
07:45 AM	0	0	0	23	0	0	0	7	0	0	0	30	0	0	0	ø0	68	J	0
Total	=	-	+-	29	0	0	0	23	0	0	0	88	0	0	0	27	198		m
08:00 AM	0	0	0	28	0	۳	0	ß	0	0	0	27	0	0	0	+	71	,	
08:15 AM	0	0	0	39	0	0	0	ნ	0	0	0	27	0	0	0	æ	87	J	0
08:30 AM	0	0	0	19	0	2	0	~	0	0	0	10	0	0	0	7	37		2
08:45 AM	0	0	0	15	0	0	0	S	0	0	0	13	0	0	0	7	40	J	0
Total	0	0	0	101	0	м	0	24	0	0	0	77	0	0	0	33	235		m
Grand Total	~	~	-	160	0	က	0	47	0	0	0	166	0	0	0	09	433		9
Apprch %	33.3	33.3	33.3		0	100	0		0	0	0		0	0	0				
Total %	16.7	16.7	16.7		0	20	0		0	0	0		0	0	0		98.6	14	-

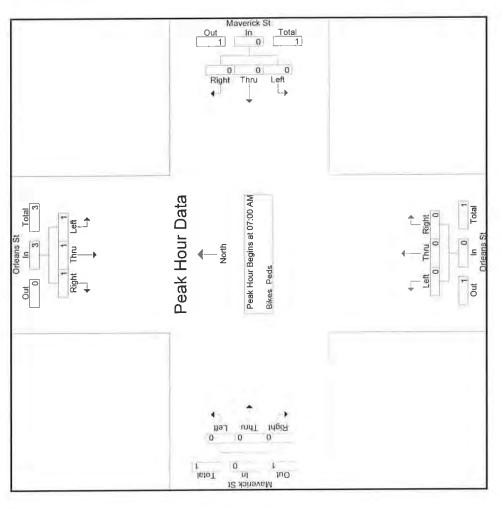
File Name : 75860009 Site Code : 75860009 Start Date : 1/25/2018 Page No : 14

		Orlea	Orleans St			Maver	Maverick St			Orleans St	ns St			Maverick St	ick St
		From North	North			From East	East			From South	South			From	From West
Start Time Left	Left	Thru	Thru Right App	App. Total	Left	Thru	Right A	App. Total	Left	Thru	Right	Right App. Total	Left	Thru	Right

Start Time Left Thru Right App. Total Left Thru Right App. Total Thru App. Total Thru App. Total Thru Right App. Total Thru Righ			From	Orleans of From North			From East	ick of East			From South	is or South			From West	West		
1 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
1 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	ak Hour Analysis Fi	rom 07:00 /	4M to 08:4	45 AM - P.	eak 1 of 1													
1 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	ak Hour for Entire In	ntersection	Begins at	: 07:00 AN	V													
0 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	07:00 AM	-	0	0	_	0	0	0	0	0	0	0	0	0	0	0	0	
1 1 1 1 3 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	07:15 AM	0	0	-	-	0	0	0	0	0	0	0	0	0	0	0	0	
1 1 1 1 1 3 3.3 3.3 3.3 3.3 3.5 50 750 700 700 700 700 700 700 700 700	07:30 AM	0	-	0	1	0	0	0	0	0	0	0	0	0	0	0	0	
14 1 1 1 3 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	07:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
33.3 33.3 33.3 0 0 0 0 0 0 0 0 0 0 0 0 0	Total Volume	-	-	-	ю	0	0	0	0	0	0	0	0	0	0	0	0	
.250 .250 .250 .750 .000 .000 .000 .000 .000 .000 .0	% App. Total	33.3	33.3	33.3		0	0	0		0	0	0		0	0	0		
	PHF	.250	.250	.250	750	000.	000	000.	000.	000	000.	000	0000	000	000	000	000.	

File Name 75860009 Site Code 75860009 Start Date 1/25/2018 Page No 15

N/S Street: Orleans Street E/W Street: Maverick Street City/State: Boston, MA Weather: Clear



Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1

Peak Hour for Each Approach Begins at:

	07:00 AM			J	7:45 AM			.0	07:00 AM			07	07:00 AM			
+0 mins.	-	0	0	~	0	0	0	0	0	0	0	0	0	0	0	0
+15 mins.	0	0	-	-	0	_	0	~	0	0	0	0	0	0	0	0
+30 mins.	0	-	0	+	0	0	0	0	0	0	0	0	0	0	0	0
+45 mins.	0	0	0	0	0	2	0	2	0	0	0	0	0	0	0	0
otal Volume	-	_	-	က	0	ო	0	က	0	0	0	0	0	0	0	0
% App. Total	33.3	33.3	33.3		0	100	0		0	0	0		0	0	0	

File Name : 75860009 Site Code : 75860009 Start Date : 1/25/2018 Page No : 1

					Groups P	Groups Printed- Cars - Trucks	Trucks				
	OL	Orleans St rom North		Ž Œ	Maverick St From East			Orleans St From South		Σπ	Maverick St From West
Start Time	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru
04:00 PM	0	6	9	4	42	7	7	7	0	0	0

					Groups P	Groups Printed- Cars - Irucks	Lucks						
	PO	Orleans St		Ma	Maverick St			Orleans St		2	Maverick St		
	Fro	From North		Ē	From East		Ē	From South		ш	From West		
Start Time	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Int. Total
04:00 PM	0	o	9	4	42	7	7	1	0	0	0	0	98
04:15 PM	0	7	4	က	39	9	9	15	0	0	0	0	80
04.30 PM	0	4	4	9	58	13	9	10	0	0	0	0	111
04:45 PM	0	7	4	0	52	7	6	10	0	0	0	0	88
Total	0	37		13	191	33	28	46	0	0	0	0	366
05:00 PM	0	12	m	←	38	12	10	16	0	0	0	0	92
05:15 PM	0	13	2	5	62	2	11	12	0	0	0	0	112
05:30 PM	0	10	က	က	41	4	17	15	0	0	0	0	87
05:45 PM	0	10	4	4	49	7	16	00	0	0	0	0	86
Total	0	45	12	13	190	30	48	51	0	0	0	0	389
Grand Total	0	82	30	26	381	63	92	26	0	0	0	0	755
Apprch %	0	73.2	26.8	5.5	81.1	13.4	43.9	56.1	0	0	0	0	
Total %	0	10.9	4	3.4	50.5	8.3	10.1	12.8	0	0	0	0	
Cars	0	82	30	26	370	63	92	26	0	0	0	0	744
% Cars	0	100	100	100	97.1	100	100	100	0	0	0	0	98.5
Trucks	0	0	0	0	=	0	0	0	0	0	0	0	#
% Trucks	0	0	0	0	2.9	0	0	0	0	0	0	0	1.5

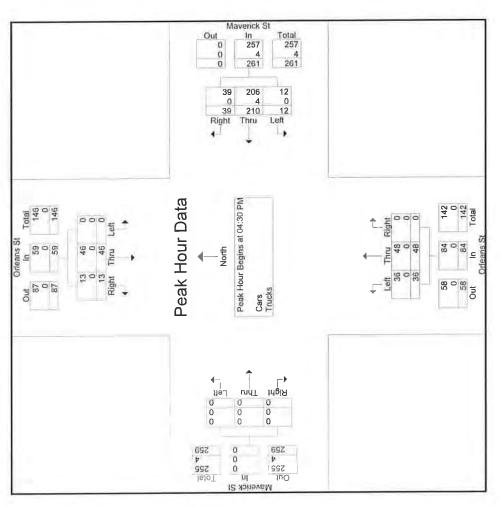
File Name : 75860009 Site Code : 75860009 Start Date : 1/25/2018 Page No : 2

N/S Street: Orleans Street E/W Street: Maverick Street City/State: Boston, MA Weather: Clear

Eat Hour Analysis From Carin Mark Signation			Orleans St	ns St			Maverick St	ick St			Orleans St	ns St			Maverick St	ick St		
App. Total Left Thru Right App. Total Left Thru Right App. Total Int. Thru App. Total Int. Thru Right App. Total Int. Thru App. Total Int. Thru Right App. Total Int. Thru App. Total Int. Thru App. Total Int. Thru App. Total Int. Thru In			From	UDION			FIOII	East			HOLL	Illinos				vvest		
46 58 13 77 6 10 0 16 0 </th <th>Start Time</th> <th></th> <th>Thru</th> <th>Right</th> <th>App. Total</th> <th>Left</th> <th>Thru</th> <th>Right A</th> <th>\pp. Total</th> <th>Left</th> <th>Thru</th> <th>Right</th> <th>App. Total</th> <th>Left</th> <th>Thru</th> <th>Right</th> <th>App. Total</th> <th>Int. Total</th>	Start Time		Thru	Right	App. Total	Left	Thru	Right A	\pp. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Intersection Begins at 04:30 PM 14	eak Hour Analysis Fr	om 04:00 F	M to 05:4	15 PM - Pe	ak 1 of 1													
0 14 4 18 6 13 77 6 10 10 16 0 16 0 16 0 16 0 17 0 <t< td=""><td>Peak Hour for Entire Ir</td><td>ntersection</td><td>Begins at</td><td>04:30 PM</td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td></t<>	Peak Hour for Entire Ir	ntersection	Begins at	04:30 PM														
0 7 4 11 0 52 7 59 9 10 16 0 19 0 16 0 16 0	04:30 PM	0	14	4	18	9	58	13	77	9	10	0	16	0	0	0	0	111
0 12 3 15 6 12 51 10 16 0 26 0 26 0 26 0 26 0 26 0 <t< td=""><td>04:45 PM</td><td>0</td><td>7</td><td>4</td><td>11</td><td>0</td><td>52</td><td>7</td><td>29</td><td>6</td><td>10</td><td>0</td><td>19</td><td>0</td><td>0</td><td>0</td><td>0</td><td>89</td></t<>	04:45 PM	0	7	4	11	0	52	7	29	6	10	0	19	0	0	0	0	89
0 13 2 15 62 7 74 11 12 0 23 0 23 0	05:00 PM	0	12	က	15	-	38	12	51	10	16	0	26	0	0	0	0	95
0 46 13 59 261 36 48 0 84 0 84 0 84 0 84 0 84 0 84 0	05:15 PM	0	13	2	15	2	62	7	74	1	12	0	23	0	0	0	0	112
0 78 22 4.6 80.5 14.9 42.9 57.1 0	Total Volume	0	46	13	59	12	210	39	261	36	48	0	84	0	0	0	0	404
.000 .821 .813 .816 .750 .847 .750 .847 .750 .847 .750 .000 <th< td=""><td>% App. Total</td><td>0</td><td>78</td><td>22</td><td></td><td>4.6</td><td>80.5</td><td>14.9</td><td></td><td>42.9</td><td>57.1</td><td>0</td><td></td><td>0</td><td>0</td><td>0</td><td></td><td></td></th<>	% App. Total	0	78	22		4.6	80.5	14.9		42.9	57.1	0		0	0	0		
0 46 13 59 12 206 39 257 36 48 0 84 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	PHF	000	.821	.813	.819	.500	.847	.750	.847	.818	.750	000.	.808	000	000	000.	000.	905
0 100 100 100 100 98.1 100 98.5 100 100 0 100 0 0 0 0 0 0 0 0 0 0 0 0	Cars	0	46	13	69	12	206	39	257	36	48	0	84	0	0	0	0	400
0 0 0 0 0 4 0 4 0 0 0 0 0 0 0 0 0 0 0 0	% Cars	0	100	100	100	100	98.1	100	98.5	100	100	0	100	0	0	0	0	99.0
$0 \qquad 0 \qquad 0 \qquad 0 \qquad 1.9 \qquad 0 \qquad 1.5 \qquad 0 \qquad $	Trucks	0	0	0	0	0	4	0	4	0	0	0	0	0	0	0	0	4
	% Trucks	0	0	0	0	0	1.9	0	1.5	0	0	0	0	0	0	0	0	1.0

File Name 75860009 Site Code 75860009 Start Date 1/25/2018 Page No 3

N/S Street: Orleans Street E/W Street: Maverick Street City/State: Boston, MA Weather: Clear



Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1

Peak Hour for Each Approach Begins at:

	04:30 PM			J	04:30 PM			J	15:00 PM			ŏ	04:00 PM			
+0 mins.	0	14	4	18	9	58	13	77	10	16	0	26	0	0	0	0
+15 mins.	0	7	4	1	0	52	7	29	=	12	0	23	0	0	0	0
+30 mins.	0	12	က	15	-	38	12	51	1	15	0	56	0	0	0	0
+45 mins.	0	13	2	15	Ŋ	62	7	74	16	ø	0	24	0	0	0	0
otal Volume	0	46	13	59	12	210	39	261	48	51	0	66	0	0	0	0
% App. Total	0	78	22		4.6	80.5	14.9		48.5	51.5	0		0	0	0	

File Name 75860009 Site Code 75860009 Start Date 1/25/2018 Page No 5

					Grou	Groups Printed- Cars	ars						
		Orleans St From North		ΣΨ	Maverick St From East			Orleans St From South		X Œ	Maverick St From West		
Start Time	Left		Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Int. Total
04:00 PM	0	6	9	4	40	7	7	=	0	0	0	0	84
04:15 PM	0	7	4	က	37	9	9	15	0	0	0	0	78
04:30 PM	0	14	4	9	57	13	9	10	0	0	0	0	110
04:45 PM	0	7	4	0	51	7	6	10	0	0	0	0	88
Total	0	37	18	13	185	33	28	46	0	0	0	0	360
05:00 PM	0	12	က	~	37	12	10	16	0	0	0	0	91
05:15 PM	0	13	2	5	61	7	7	12	0	0	0	0	111
05:30 PM	0	10	т	ო	40	4	11	15	0	0	0	0	98
05:45 PM	0	10	4	4	47	7	16	∞	0	0	0	0	96
Total	0	45	12	13	185	30	48	51	0	0	0	0	384
Grand Total	0	82	30	26	370	63	92	26	0	0	0	0	744
Apprch %	0	73.2	26.8	2.7	9.08	13.7	43.9	56.1	0	0	0	0	
Total %	0	11	4	3.5	49.7	8.5	10.2	13	0	0	0	0	

File Name 75860009 Site Code 75860009 Start Date 1/25/2018 Page No 9

	- H	Orleans St From North		Ma	Maverick St From East	St		Orleans St From South		M	Maverick St From West		
Start Time	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Leff	Thru	Right	Int. Total
04:00 PM	0	0	0	0	2	0	0	0	0	0	0	0	2
04:15 PM	0	0	0	0	7	0	0	0	0	0	0	0	2
04:30 PM	0	0	0	0	-	0	0	0	0	0	0	0	₹
04:45 PM	0	0	0	0	-	0	0	0	0	0	0	0	_
Total	0	0	0	0	9	0	0	0	0	0	0	0	9
05:00 PM	0	0	0	0	+	0	0	0	0	0	0	0	-
05:15 PM	0	0	0	0	~	0	0	0	0	0	0	0	_
05:30 PM	0	0	0	0	۲	0	0	0	0	0	0	0	_
05:45 PM	0	0	0	0	2	0	0	0	0	0	0	0	2
Total	0	0	0	0	ις	0	0	0	0	0	0	0	5
Grand Total	0	0	0	0	1	0	0	0	0	0	0	0	1
Apprch %	0	0	0	0	100	0	0	0	0	0	0	0	
Total %	0	0	0	0	100	0	0	0	0	0	0	0	

File Name : 75860009 Site Code : 75860009 Start Date : 1/25/2018 Page No : 13

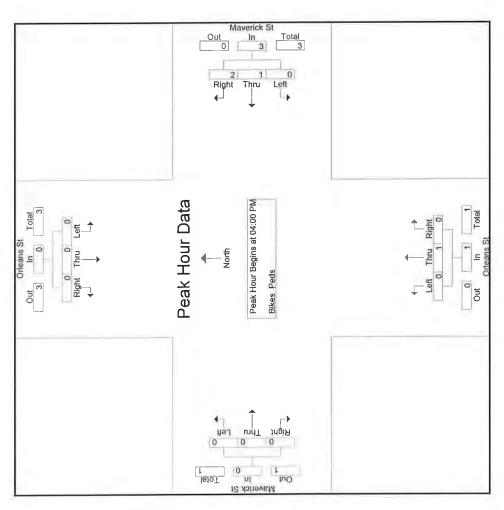
		Orleans St From North	s St orth			Maverick St From East	Maverick St From East			Orleans St From South	s St outh			Maverick St From West	k St fest				
Start Time	Left	Thru	Right	Peds	Left	Thru	Right	Peds	Left	Thru	Right	Peds	Left	Thru	Right	Peds	Exclu. Total Inclu. Total	าclu. Total	Int. Total
04:00 PM	0	0	0	18	0	0		က	0	0	0	4	0	0	0	ιΩ	40	***	41
04 15 PM	0	0	0	13	0	0	+	4	0	0	0	25	0	0	0	5	47		48
04:30 PM	0	0	0	18	0	-	0	7	0	0	0	23	0	0	0	6	57	۳	58
04:45 PM	0	0	0	15	0	0	0	4	0	_	0	22	0	0	0	цэ	46	+	47
Total	0	0	0	64	0	4-	2	18	0	-	0	84	0	0	0	24	190	4	194
05.00 PM	0	0	0	18	0	0	0	φ	0	0	0	31	0	0	0	6	58	0	58
05:15 PM	0	0	0	16	0	0	0	11	0	0	0	34	0	0	0	11	72	0	72
05 30 PM	0	-	0	20	0	0	0	2	+	-	0	56	0	0	0	10	61	ന	64
05:45 PM	0	0	0	20	0	0	0	7	0	0	0	24	0	0	0	00	54	0	54
Total	0	-	0	74	0	0	0	24		T	0	115	0	0	0	32	245	ന	248
Grand Total	0	_	0	138	0	~	2	42	7-	2	0	199	0	0	0	99	435	7	442
Apprch %	0	100	0		0	33.3	66.7		33.3	66.7	0		0	0	0				
Total %	0	14.3	0		0	14.3	28.6		14.3	28.6	0		0	0	0		98.4	1.6	

File Name : 75860009 Site Code : 75860009 Start Date : 1/25/2018 Page No : 14

App. Total Left Thru Right App. Total Left Thru Right App. Total ak 1 of 1 at 1 a			Orlea	Orleans St			Maver	verick St			Orleans St	ns St			Mave	Maverick St		
Left Thru Right App. Total Left Thru Right App. Total Left Thru Right App. Total Ap			From	North			From	East			From (South			From	From West		
0 0 0 0 1 1 1 0 0 0 0 0 0 0 0 0 0 0 0 0	Start Time	Left	Thru	Right	App. Total	Left	Thru	Right /	App. Total	Left	Thu	Right	App. Total	Left	Thru	Right	Right App. Total	Int. Total
0 0 0 0 1 1 1 0 0 0 0 0 0 0 0 0 0 0 0 0	k Hour Analysis From	om 04:00	PM to 05:4	45 PM - P	eak 1 of 1													
0 0 0 0 0 0 0 0 0 1 1 1 0 0 0 0 0 0 0 0	k Hour for Entire In	itersection	ו Begins at	t 04:00 PN	>													
0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	04:00 PM	0	0	0	0	0	0	τ-	-	0	0	0	0	0	0	0	0	
0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	04:15 PM	0	0	0	0	0	0	~	-	0	0	0	0	0	0	0	0	
0 0 0 0 0 0 0 0 0 0 0 0 1 0 1 0 1 0 0 1 0 0 1 0 0 1 0 0 1 0 0 1 0 0 1 0 0 1 0 0 1 0 0 1 0 0 1 0 0 1 0 0 1 0	04:30 PM	0	0	0	0	0	-	0	_	0	0	0	0	0	0	0	0	
0 0 0 0 0 1 2 3 0 1 0 1 0 0 0 0 33.3 66.7 0 100 0 .000 .000 .000 .000 .250 .500 .750 .000 .250 .000 .250	04:45 PM	0	0	0	0	0	. 0	0	0	0	*	0		0	0	0	0	
0 00 0 0 33.3 66.7 0 100 0 0 250 .000 .000 .250 .000 .250 .000 .250 .000 .250	Total Volume	0	0	0	0	0	-	2	က	0	-	0	1	0	0	0	0	
.000 .000 .000 .000 .000 .250 .500 .750 .000 .250 .000 .250	% App. Total	0	0	0		0	33.3	2.99		0	100	0		0	0	0		
	PHF	000	000	000	000.	000.	.250	.500	.750	000	.250	000	.250	000	000.	000	000.	1.00

File Name 75860009 Site Code 75860009 Start Date 1/25/2018 Page No 15

N/S Street: Orleans Street E/W Street: Maverick Street City/State: Boston, MA Weather: Clear



Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1

Peak Hour for Each Approach Begins at:

04	04:45 PM			04	04:00 PM			0	04:45 PM			3	04:00 PM			
+0 mins.	0	0	0	0	0	0	-	-	0	-	0	-	0	0	0	0
+15 mins.	0	0	0	0	0	0	_	_	0	0	0	0	0	0	0	0
+30 mins.	0	0	0	0	0		0	_	0	0	0	0	0	0	0	0
+45 mins.	0	_	0	7-	0	0	0	0	_	-	0	7	0	0	0	0
Total Volume	0	_	0	1	0	-	2	m	-	2	0	က	0	0	0	0
% App. Total	0	100	0		0	33.3	2.99		33.3	2.99	0		0	0	0	

File Name : 75860010 Site Code : 75860010 Start Date : 1/25/2018 Page No : 1

N/S Street: Frankfort Street E/W Street: Maverick Street City/State: Boston, MA Weather: Clear

624 97.7 108 325 639 Int. Total 98 80 314 107 57 53 77 0 0 0 0 0 0 Right 0 0 0 0 Thru 0 0 0 0 0 0 0 0 0 0 0 Maverick St From West 0 Right 0 0 0 0 0 έ. 100 Frankfort St From South 0 2 20 ~ Thru Ŋ 0 100 0 0 Left 4 0 0 2 3 50 Groups Printed- Cars - Trucks Maverick St 10.5 0 0 Right 67 100 9 0 4 1 4 2 40 27 78.6 488 Thru 248 82 85 48 39 254 87 99 54 29 61 From East 4 4.1 5.1 100 Left 4 6.9 43 Right 20 ∞ 9 ∞ $^{\circ}$ 24 8.3 9.0 4 001 0 0 Frankfort St From North Thru 0 0 α 4 0 0 0 0 0 0 0 0 0 0 Left 0 Total % Cars Trucks Grand Total Total Apprch % % Cars % Trucks 07:45 AM Total 08:30 AM 08:45 AM Start Time 07:00 AM 07:15 AM 07:30 AM 08:00 AM 08:15 AM

File Name : 75860010 Site Code : 75860010 Start Date : 1/25/2018 Page No : 2

N/S Street: Frankfort Street E/W Street: Maverick Street City/State: Boston, MA Weather: Clear

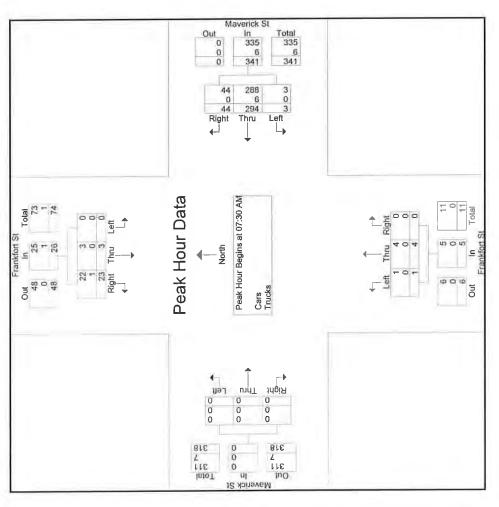
Peak Hou

Peak Hou

		Frank	Frankfort St			Maverick St	ck St			Frankfort St	ort St			Maverick St	ick St		
		From	From North			From East	East			From South	outh			From West	West		
Start Time	Left	Thru	Right App. Total	pp. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1	, 00:70 mc	AM to 08:4	45 AM - Peak	c 1 of 1													
Hour for Entire Intersection Begins at 07:30 AM	tersection	Begins at	1 07:30 AM														
07:30 AM	0	~	00	o	-	61	4	99	0	8	0	7	0	0	0	0	77
07:45 AM	0	_	က	4	-	99	0	92	0	0	0	0	0	0	0	0	80
08:00 AM	0	0	œ	00	-	82	4	26	-	_	0	2	0	0	0	0	107
08:15 AM	0	~	4	2	0	85	17	102	0	-	0	-	0	0	0	0	108
Total Volume	0	က	23	56	က	294	44	341	-	4	0	5	0	0	0	0	372
% App. Total	0	11.5	88.5		6.0	86.2	12.9		20	80	0		0	0	0		
PHF	000	.750	.719	.722	.750	.865	.647	.836	.250	.500	000.	.625	000	000.	000.	000	.861
Cars	0	က	22	25	က	288	44	335	-	4	0	5	0	0	0	0	365
% Cars	0	100	95.7	96.2	100	98.0	100	98.2	100	100	0	100	0	0	0	0	98.1
Trucks	0	0	~	_	0	9	0	9	0	0	0	0	0	0	0	0	7
% Trucks	0	0	4.3	3.8	0	2.0	0	1.8	0	0	0	0	0	0	0	0	1.9

File Name: 75860010 Site Code: 75860010 Start Date: 1/25/2018 Page No: 3

N/S Street: Frankfort Street E/W Street: Maverick Street City/State: Boston, MA Weather: Clear



Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1

Peak Hour for Each Approach Begins at:

	07:15 AM			0	7:30 AM				07:00 AM			.0	07:00 AM			
+0 mins.	0	0	9	9	-	61	4	99	_	7	0	က	0	0	0	0
+15 mins.	0	7	œ	o	-	99	6	92	ო	-	0	4	0	0	0	0
+30 mins.	0	-	က	4	+	82	14	26	0	2	0	2	0	0	0	0
+45 mins.	0	0	œ	00	0	82	17	102	0	0	0	0	0	0	0	0
Total Volume	0	2	25	27	ന	294	44	341	4	22	0	o	0	0	0	0
% App. Total	0	7.4	97.6		0.9	86.2	12.9		44.4	55.6	0		0	0	0	

File Name : 75860010 Site Code : 75860010 Start Date : 1/25/2018 Page No : 5

					Grou	Groups Printed- Cars	ars						
	ŭū	Frankfort St From North		Σπ	Maverick St From East			Frankfort St From South		ΣH	Maverick St From West		
Start Time	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Int. Total
07:00 AM	0	0	7	-	52	9	-	2	0	0	0	0	69
07:15 AM	0	0	9	,	65	80	ဗ	-	0	0	0	0	84
07:30 AM	0	~	00	-	59	4	0	2	0	0	0	0	75
07:45 AM	0	←	က	÷	63	6	0	0	0	0	0	0	77
Total	0	2	24	4	239	27	4	ro	0	0	0	0	305
08:00 AM	0	0	80		60	14	~	~	0	0	0	0	106
08:15 AM	0	-	က	0	85	17	0	-	0	0	0	0	107
08:30 AM	0	0	4	_	46	4	0	0	0	0	0	0	55
08:45 AM	0	-	4	2	37	Ω	2	0	0	0	0	0	51
Total	0	2	19	4	249	40	က	2	0	0	0	0	319
Grand Total	0	4	43	∞	488	29	7	7	0	0	0	0	624
Apprch %	0	8.5	91.5	1.4	86.7	11.9	20	20	0	0	0	0	
Total %	0	9.0	6.9	1.3	78.2	10.7	1.1	1.1	0	0	0	0	

File Name : 75860010 Site Code : 75860010 Start Date : 1/25/2018 Page No : 9

	L					Control and the Control of the Contr							
	Fra	Frankfort St		Ä	Maverick St			Frankfort St		Ma	Maverick St		
	i	From North		Ţ	From East		Fro	From South		Frc	From West		
Start Time	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Int. Total
07:00 AM	0	0	0	0	2	0	0	0	0	0	0	0	2
07:15 AM	0	0	0	0	2	0	0	0	0	0	0	0	2
07:30 AM	0	0	0	0	2	0	0	0	0	0	0	0	2
07:45 AM	0	0	0	0	ဇ	0	0	0	0	0	0	0	(,)
Total	0	0	0	0	6	0	0	0	0	0	0	0	C)
08:00 AM	0	0	0	0	-	0	0	0	0	0	0	0	
08:15 AM	0	0	_	0	0	0	0	0	0	0	0	0	•
08:30 AM	0	0	0	0	2	0	0	0	0	0	0	0	14
08:45 AM	0	0	0	0	2	0	0	0	0	0	0	0	8
Total	0	0	~	0	ις	0	0	0	0	0	0	0	9
Grand Total	0	0	~	0	14	0	0	0	0	0	0	0	15
Apprch %	0	0	100	0	100	0	0	0	0	0	0	0	
Total %	0	0	6.7	0	93,3	0	0	0	0	0	0	0	

File Name : 75860010 Site Code : 75860010 Start Date : 1/25/2018 Page No : 13

	Frankfort St From North	tr.																
-:		1			Mavenck St	k St			Frankfort St	t St			Maverick St	k St				
		סונט			From II	ast			From South	uth			From West	est				
	Thru	Thru Right	Peds	Left	Thru Righ	Right	Peds	Left	Thru	Right	Peds	Left	Thru	Right	Peds	Exclu. Total Inclu. Total	nclu. Total	Int. Total
	0	0	σ	0	0	0	-	0	0	0	90	0	0	0	4	22	0	22
	0	0	10	0	0	0	0	0	0	0	12	0	0	0	9	28	0	28
07:30 AIM 0	0	0	22	0	~	0	2	0	0	0	80	0	0	0	7	39	-	40
07:45 AM 0	0	0	35	0	7	0	ო	0	0	0	6	0	0	0	27	78	2	80
Total : 0	0	0	92	0	m	0	9	0	0	0	14	0	0	0	44	167	ю	170
08:00 AM 0	0	0	37	0	က	0	7	0	0	0	20	0	0	0	25	89	ო	92
08:15 AM 0	0	0	41	0	4	0	Ø	0	0	0	21	0	0	0	12	82	4	86
08:30 AM 0	0	0	15	0	0	0	**	0	0	0	13	0	0	0	9	35	0	35
08:45 AM 0	0	0	7	0	-	0	+	0	0	0	12	0	0	0	-	25	*	26
Total 0	0	0	104	0	60	0	17	0	0	0	99	0	0	0	44	231	60	239
Grand Total 0	0	0	180	0	7	0	23	0	0	0	107	0	0	0	80 80	398	11	409
Apprch % 0	0	0		0	100	0		0	0	0		0	0	0				
Total % 0	0	0		0	100	0		0	0	0		0	0	0		97.3	2.7	

File Name : 75860010 Site Code : 75860010 Start Date : 1/25/2018 Page No : 14

N/S Street: Frankfort Street E/W Street: Maverick Street City/State: Boston, MA Weather: Clear

	Frankfort St	fort St			Mayo	Mayorick St			Frankfort St	+ C + C			Mayorick Ct	77.		
	From I	From North			From	From East			From South	South			From West	West		
Left:	Thru	Right	Thru Right App. Total	Left	Thru	Right	Right App. Total	Left	Thru	Right	Thru Right App. Total	Left	Thru	Right	Thru Right App. Total Int. Total	Int. Total
00	Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1	15 AM - P	eak 1 of 1													
fion	Peak Hour for Entire Intersection Begins at 07:30 AM	07:30 AN	7													
0	0	0	0	0	_	0	~	0	0	0	0	0	0	0	0	_
0	0	0	0	0	2	0	2	0	0	0	0	0	0	0	0	2
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08:15 AM Total Volume % App. Total

0 0

100

4 0

File Name 75860010 Site Code 75860010 Start Date 1/25/2018 Page No 15

N/S Street: Frankfort Street E/W Street: Maverick Street City/State: Boston, MA Weather: Clear

Maverick SI In 10 Out 0 Total 10 10 Thru Right Left Peak Hour Data Peak Hour Begins at 07:30 AM Total 0 Frankfort St Out In Out In Frankfort St North Right o Bikes Peds O Jugist no ot Total Maverick St In 0 0

Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1

Peak Hour for Each Approach Begins at:

.0	07:00 AM			07	07:30 AM			07	07:00 AM			07	07:00 AM			
+0 mins.	0	0	0	.0	0	_	0	_	0	0	0	0	0	0	0	0
+15 mins.	0	0	0	0	0	2	0	2	0	0	0	0	0	0	0	0
+30 mins.	0	0	0	0	0	ო	0		0	0	0	0	0	0	0	0
+45 mins.	0	0	0	0	0	4	0	4	0	0	0	0	0	0	0	0
Total Volume	0	0	0	0	0	10	0		0	0	0	0	0	0	0	0
% App. Total	0	0	0		0	100	0		0	0	0		0	0	0	

File Name: 75860010 Site Code: 75860010 Start Date: 1/25/2018 Page No: 1

N/S Street: Frankfort Street E/W Street: Maverick Street City/State: Boston, MA Weather: Clear

Groups Printed- Cars - Trucks

					7	s Timed-Cals - Hucks							
		Frankfort St From North		Σ IL	Maverick St From Fast		F F	Frankfort St From South		Ma	Maverick St From West		
Start Time	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Int. Total
04:00 PM	0	2	က	-	45	2	2	-	0	0	0	0	56
04:15 PM	0	0	က	2	90	9	0	0	0	0	0	0	61
04:30 PM	0	-	S	က	20	cy.	2	2	0	0	0	0	80
04:45 PM	0	-	80	2	47	ю	0	-	0	0	0	0	62
Total	0	4	19	∞	212	16	4	4	0	0	0	0	267
05:00 PM	0	-	9	2	48	4	0	0	0	0	0	0	61
05:15 PM	0	က	9	-	09	7	0	က	0	0	0	0	80
05:30 PM	0	+	S	~	47	4	0	က	0	0	0	0	61
05:45 PM	0	0	7	-	20	2	0	2	0	0	0	0	62
Total	0	ις	24	Ω	205	17	0	∞	0	0	0	0	264
Grand Total	0	ത	43	13	417	er er	4	12	C	C	c	O	531
Apprch %	0	17.3	82.7	2.8	90.1	7.1	25	75	0	0	0	0	}
Total %	0	1.7	8.1	2.4	78.5	6.2	0.8	2.3	0	0	0	0	
Cars	0	O	43	13	408	33	4	12	0	0	0	0	522
% Cars	0	100	100	100	8.76	100	100	100	0	0	0	0	98.3
Trucks	0	0	0	0	O	0	0	0	0	0	0	0	o
% Trucks	0	0	0	0	2.2	0	0	0	0	0	0	0	1.7

File Name : 75860010 Site Code : 75860010 Start Date : 1/25/2018 Page No : 2

N/S Street: Frankfort Street E/W Street: Maverick Street City/State: Boston, MA Weather: Clear

Mayorick St Frankfort St Mayorick St Frankfort St Peak Hour

Start Time Left Thru Right App. Total Left Thru Right Thru Right Thru Right Thru Right Thru Thru Right Thru Thru			Franktort St	tort St			Maverick St	ick St			Franktort St	ort St			Maverick St	ick St		
App. Total Left Thru Right App. Total Int. Thru 84 1 0f 3 70 5 78 2 2 0 4 0 </th <th></th> <th></th> <th>From</th> <th>North</th> <th></th> <th></th> <th>From</th> <th>East</th> <th></th> <th></th> <th>From 5</th> <th>South</th> <th></th> <th></th> <th>From</th> <th>West</th> <th></th> <th></th>			From	North			From	East			From 5	South			From	West		
9 3 70 5 78 2 2 0 4 0 0 0 0 0 9 2 47 3 52 0 1 0 1 0 0 0 0 0 9 1 60 7 68 0 3 0 3 0 </th <th>Start Time</th> <th>Left</th> <th>Thru</th> <th>Right</th> <th>App. Total</th> <th>Left</th> <th>Thru</th> <th></th> <th>App. Total</th> <th>Left</th> <th>Thru</th> <th></th> <th>App. Total</th> <th>Left</th> <th>Thru</th> <th>Right</th> <th>App. Total</th> <th>Int. Total</th>	Start Time	Left	Thru	Right	App. Total	Left	Thru		App. Total	Left	Thru		App. Total	Left	Thru	Right	App. Total	Int. Total
6 3 70 5 78 2 2 0 4 0	ak Hour Analysis Fr	rom 04:00	PM to 05:4	15 PM - P	eak 1 of 1													
0 1 5 6 3 70 5 78 2 2 2 6 4 0 4 0 4 0 4 0 4 0 4 6 6 6 0	ak Hour for Entire Ir	ntersection	Begins at	04:30 PN	V													
0 1 8 9 2 47 3 52 6 1 0 1 0 1 0 1 0	04:30 PM	0	~	2	9	ო	70	5	78	8	8	0	4	0	0	0	0	õ
0 1 6 4 4 54 6 0	04:45 PM	0	-	8	o	2	47	60	25	0	τ-	0	-	0	0	0	0	9
0 3 6 9 7 68 0 3 0 3 0 3 0 3 0 3 0 3 0 3 0 3 0	05:00 PM	0	-	9	7	2	48	4	54	0	0	0	0	0	0	0	0	61
0 6 25 31 8 25 19 252 6 6 0 8 0 </td <td>05:15 PM</td> <td>0</td> <td>ო</td> <td>9</td> <td>6</td> <td>~</td> <td>09</td> <td>7</td> <td>89</td> <td>0</td> <td>ო</td> <td>0</td> <td>ന</td> <td>0</td> <td>0</td> <td>0</td> <td>0</td> <td>80</td>	05:15 PM	0	ო	9	6	~	09	7	89	0	ო	0	ന	0	0	0	0	80
0 19.4 80.6 3.2 89.3 7.5 3.5 7.5 0	Total Volume	0	9	25	31	00	225	19	252	2	9	0	8	0	0	0	0	291
000 .500 .781 .861 .667 .804 .679 .808 .250 .500 .000	% App. Total	0	19.4	80.6		3.2	89.3	7.5		25	75	0		0	0	0		
0 6 25 31 8 221 19 248 2 6 0 8 0<	PHF	000	.500	.781	.861	799.	.804	629.	808	.250	.500	000.	.500	000	000.	000.	000.	.827
0 100 100 100 100 98.2 100 98.4 100 100 0 100 0 0 0 0 0 0 0 0 0 0 0 0	Cars	0	9	25	31	00	221	19	248	2	9	0	00	0	0	0	0	287
0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	% Cars	0	100	100	100	100	98.2	100	98.4	100	100	0	100	0	0	0	0	98.6
0 0 0 0 0 0 1.8 0 1.6 0 0 0 0 0 0 0 0 0	Trucks	0	0	0	0	0	4	0	4	0	0	0	0	0	0	0	0	
	% Trucks	0	0	0	0	0	1.8	0	1.6	0	0	0	0	0	0	0	0	1.4

File Name: 75860010 Site Code: 75860010 Start Date: 1/25/2018 Page No: 3

N/S Street: Frankfort Street E/W Street: Maverick Street City/State: Boston, MA Weather: Clear

8 0 8 Left 19 0 19 Right 221 4 225 Thru Total 56 0 56 Peak Hour Data Peak Hour Begins at 04:30 PM 000 Frankfort St In 31 0 North 2 25 0 25 Right 41 Out 25 0 25 Left Cars 0 0 0 reu 0 0 0 o o idgiЯi 16/07 845 4 252

Maverick SI In 248 4 252

Out 0 0

Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1 Peak Hour for Each Approach Begins at:

	04:30 PM			J	04:30 PM			J	04:00 PM			Ŏ	04:00 PM		
+0 mins.	0	-	ß	9	ო	70	5	78	2	-	0	က	0	0	0
+15 mins.	0	-	œ	o	2	47	က	52	0	0	0	0	0	0	0
+30 mins.	0	**	9	7	2	48	4	54	2	7	0	4	0	0	0
+45 mins.	0	63	9	6	_	9	7	99	0	-	0	_	0	0	0
otal Volume	0	ဖ	25	31	00	225	19	252	4	4	0	80	0	0	0
% App. Total	0	19.4	80.6		3.2	89.3	7.5		20	20	0		0	0	0

0 0

0 0

File Name : 75860010 Site Code : 75860010 Start Date : 1/25/2018 Page No : 5

					Grou	Groups Printed- Cars	Cars						
	E i	Frankfort St		2	Maverick St			Frankfort St		Μ̈́ι	Maverick St		
į		From North		-	From East			om South		1	From West		
Start Time	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thr	Right	Int. Total
04:00 PM	0	2	ന	-	44	2	2	-	0	0	0	0	55
04:15 PM	0	0	ю	2	48	9	0	0	0	0	0	0	59
04:30 PM	0	-	S	က	69	5	2	7	0	0	0	0	87
04:45 PM	0	~	80	8	46	က	0	-	0	0	0	0	61
Total	0	4	19	∞	207	16	4	4	0	0	0	0	262
05:00 PM	0	+	9	2	47	4	0	0	0	0	0	0	09
05:15 PM	0	8	9	-	59	7	0	က	0	0	0	0	79
05:30 PM	0	-	5	+	46	4	0	3	0	0	0	0	09
05:45 PM	0	0	7	T	49	2	0	2	0	0	0	0	61
Total	0	S	24	ß	201	17	0	ω	0	0	0	0	260
Grand Total	0	6	43	13	408	33	4	12	0	0	0	0	522
Apprch %	0	17.3	82.7	2.9	89.9	7.3	25	75	0	0	0	0	
Total %	0	1.7	8.2	2.5	78.2	6.3	0.8	2.3	0	0	0	0	

File Name : 75860010 Site Code : 75860010 Start Date : 1/25/2018 Page No : 9

	цц	Frankfort St From North		_	Maverick St From East			Frankfort St From South		≥ 11.	Maverick St From West		
Start Time	Left	Thru	Right	reft	Thru	Right	Left	Thru	Right	Left	Thru	Right	Int. Total
04:00 PM	0	0	0	0	-	0	0	0	0	0	0	0	-
04:15 PM	0	0	0	0	2	0	0	0	0	0	0	0	2
04:30 PM	0	0	0	0	1524	0	0	0	0	0	0	0	<u>←</u>
04:45 PM	0	0	0	0	-	0	0	0	0	0	0	0	-
Total	0	0	0	0	2	0	0	0	0	0	0	0	S.
05:00 PM	0	0	0	0	=	0	0	0	0	0	0	0	<i>s</i> =
05:15 PM	0	0	0	0	Ţ	0	0	0	0	0	0	0	5
05:30 PM	0	0	0	0	~	0	0	0	0	0	0	0	7
05:45 PM	o	0	0	0	•	0	0	0	0	0	0	0	*
Total	0	0	0	0	4	0	0	0	0	0	0	0	4
Grand Total	0	0	0	0	6	0	0	0	0	0	0	0	6
Apprch %	0	0	0	0	100	0	0	0	0	0	0	0	
Total %	0	0	0	0	100	0	0	0	0	0	0	0	

eet	eet		
nkfort Str	rerick Str	Boston, MA	ar
N/S Street: Frankfort Street	E/W Street: Maverick Street	٠.	r : Clear
N/S Str	E/W Str	City/State	Weather

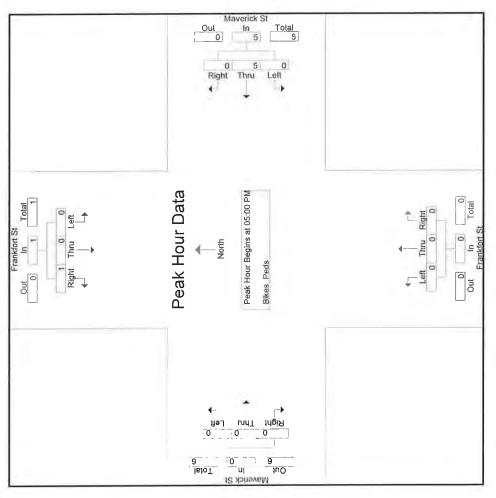
		Frankfort St	T St			Maverick St	k St		Frankfort S	Frankfort St	rt St			Maverick St	k St				
Start Time	Left	Thru	Right	Peds	Left	Thru Rig	Right	Peds	Left	Thru	Right	Peds	Left	Thru	Right	Peds	Exclu. Total	Exclu. Total Inclu. Total	Int. Total
04:00 PM	0	0	0	17	0	0	0	0	0	0	0	∞0	0	0	0	00	33	0	33
04:15 PM	0	0	0	G	0	•	0	2	0	0	0	18	0	0	0	ю	32	7	33
04:30 PM	0	0	0	16	0	0	0	0	0	0	0	17	0	0	0	7	40	0	40
04:45 PM	0	0	0	00	0	0	0	ო	0	0	0	12	0	0	0	00	31	0	31
Total	0	0	0	90	0	+	0	w	0	0	0	55	0	0	0	26	136	₩.	137
05:00 PM	0	0	0	F	0		0	7	0	0	0	18	0	0	0	O	40		41
05:15 PM	0	0	0	18	0	0	0	က	0	0	0	91	0	0	0	10	47	0	47
05:30 PM	0	0	0	21	0	+	0	Ţ	0	0	0	13	0	0	0	22	40	क्त	4
05:45 PM	0	0	-	4	0	က	0	0	0	0	0	00	0	0	0	9	28	4	32
Total	0	0	, -	64	0	rv	0	ဖ	0	0	0	22	0	0	0	30	155	SO.	161
Grand Total	0	0	-	411	0	9	0	1	0	0	0	110	0	0	0	56	291	7	298
Apprch %	0	0	100		0	100	0		0	0	0		0	0	0				
Total %	0	0	14.3		0	85.7	0		0	0	0		0	0	0		97.7	2.3	

File Name : 75860010 Site Code : 75860010 Start Date : 1/25/2018 Page No : 14

		From North	From North			Maverick St From East	ick st East			Franktort St From South	ort St South			Mave	Maverick St From West		
Start Time Left	Left	Thru	Right ,	Thru Right App. Total	Left	Thru	Right A	Right App. Total	Left	Thru	Right	Right App. Total	Left	Thru	Right	Right App. Total	Int. Total
Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1	04:00 P	M to 05:4	5 PM - Pe	ak 1 of 1													
Peak Hour for Entire Intersection Begins at 05:00 PM	ection E	3egins at (05:00 PM														
05:00 PM	0	0	0	0	0	5	0	-	0	0	0	0	0	0	0	0	
05:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
05:30 PM	0	0	0	0	0	-	0	-	0	0	0	0	0	0	0	0	
05:45 PM	0	0	٠	-	0	ო	0	က	0	0	0	0	0	0	0	0	
Total Volume	0	0	•	1	0	5	0	ιΩ	0	0	0	0	0	0	0	0	
% App. Total	0	0	100		0	100	0		0	0	0		0	0	0		
PHF.	000	000	.250	.250	000	.417	000	.417	000	000	000	000	000	000	OUU	000	375

File Name 75860010 Site Code 75860010 Start Date 1/25/2018 Page No 15

N/S Street: Frankfort Street E/W Street: Maverick Street City/State: Boston, MA Weather: Clear



Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1

0	05:00 PM			J	05:00 PM			0	04:00 PM			Ò	04:00 PM			
+0 mins.	0	0	0	0	0	-	0	_	0	0	0	0	0	0	0	0
+15 mins.	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
+30 mins.	0	0	0	0	0	~-	0	_	0	0	0	0	0	0	0	0
+45 mins.	0	0	-	-	0	ო	0	ო	0	0		0	0	0	0	0
otal Volume	0	0	**		0	2	0	ເດ	0	0	0	0	0	0	0	0
App. Total	0	0	100		0	100	0		0	0	0		0	0	0	

File Name : 75860011 Site Code : 75860011 Start Date : 1/25/2018 Page No : 1

Start Time 07:00 AM 07:15 AM 07:30 AM 07:45 AM	Cot Fror Left	Cottage St From North		Ma	Mayerick St		J. C.	Cottage St		Ž	Maverick St		
Start Time 07:00 AM 07:15 AM 07:30 AM 07:45 AM	Left	-		Ē	From East		Froi	From South		Ē.	From West		
07:00 AM , 07:15 AM , 07:30 AM 07:45 AM		Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Int. Total
07:15 AM 07:30 AM 07:45 AM	0	22	18	8	45	0	0	0	0	0	0	0	88
07:30 AM 07:45 AM	0	31	16	4	69	0	0	0	0	0	0	0	120
07:45 AM	0	17	38	5	90	0	0	0	0	0	0	0	110
	0	15	52	е	51	0	0	0	0	0	0	0	121
Total	0	85	124	15	215	0	0	0	0	0	0	0	439
08:00 AM	0	12	70	0	53	0	0	0	0	0	0	0	135
08:15 AM	0	15	80	က	39	0	0	0	0	0	0	0	137
08:30 AM	0	-	15	က	38	0	0	0	0	0	÷	0	68
08:45 AM	0	∞	11	က	38	0	7-	0	0	0	0	0	61
Total	0	46	176	ற	168	0	,-	0	0	0	-	0	401
Grand Total	0	131	300	24	383	0	_	0	0	0	←	0	840
Apprch %	0	30.4	9.69	5.9	94.1	0	100	0	0	0	100	0	
Total %	0	15.6	35.7	2.9	45.6	0	0.1	0	0	0	0.1	0	
Cars	0	128	292	24	370	0	0	0	0	0	-	0	815
% Cars	0	7.79	97.3	100	9.96	0	0	0	0	0	100	0	26
Trucks	0	က	∞	0	13	0		0	0	0	0	0	25
% Trucks	0	2.3	2.7	0	3.4	0	100	0	0	0	0	0	3

File Name : 75860011 Site Code : 75860011 Start Date : 1/25/2018 Page No : 2

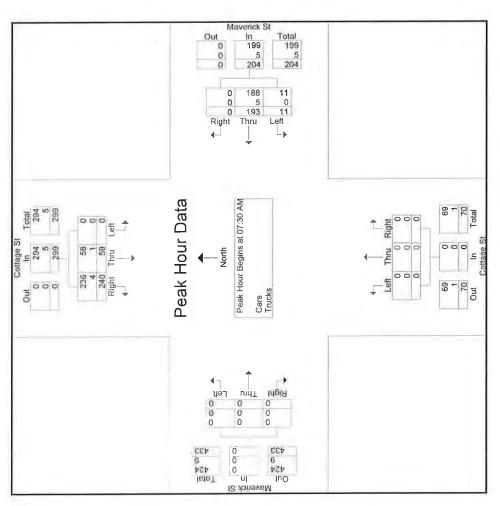
N/S Street : Cottage Street E/W Street : Maverick Street City/State : Boston, MA Weather : Clear

Peak Ho

Peak Ho

Cottage St	+		Maverick St	ick St			Cottage St	e St			Maver	Maverick St		
From North			From East	East			From South	south			From	From West		
Left Thru Right App. Total Left	Left		Thru	Right /	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1														
 Hour for Entire Intersection Begins at 07:30 AM 														
38 55		ю	90	0	55	0	0	0	0	0	0	0	0	110
52 67		m	51	0	54	0	0	0	0	0	0	0	0	121
70 82 (53	0	53	0	0	0	0	0	0	0	0	135
80 95 3	3		39	0	45	0	0	0	0	0	0	0	0	137
240 299 11	1		193	0	204	0	0	0	0	0	0	0	0	503
80.3 5.4	5.4		94.6	0		0	0	0		0	0	0		
.787550	.550		.910	000.	.927	000	000.	000.	0000	000.	000.	000	000	.918
236 294 1	_	-	188	0	199	0	0	0	0	0	0	0	0	493
98.3 98.3 100		0	97.4	0	97.5	0	0	0	0	0	0	0	0	98.0
4 5 0			5	0	5	0	0	0	0	0	0	0	0	10
1.7 1.7 0	0		2.6	0	2.5	0	0	0	0	0	0	0	0	2.0

File Name 75860011 Site Code 75860011 Start Date 1/25/2018 Page No 3



Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1 Peak Hour for Each Approach Begins at:

	0	0	0	-	÷	
	0	0	0	0	0	0
	0	0	0	-	-	100
07:45 AM	0	0	0	0	0	0
20	0	0	0	-	-	
	0	0	0	0	0	0
	0	0	0	0	0	0
08:00 AM	0	0	0	-	-	100
õ	73	22	54	53	235	
	0	0	0	0	0	0
	69	20	51	53	223	94.9
07:15 AM	4	Ŋ	က	0	12	5.1
0	22	29	82	98	299	
	38	52	20	80	240	80.3
	17	15	12	15	59	19.7
07:30 AM	0	0	0	0	0	0
.0	+0 mins.	+15 mins.	+30 mins.	+45 mins.	Fotal Volume	% App. Total

File Name : 75860011 Site Code : 75860011 Start Date : 1/25/2018 Page No : 5

					Grou	Groups Printed- Cars	ars						
		Cottage St From North		Σπ	Maverick St From East			Cottage St From South		May	Maverick St From West		
Start Time	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Int. Total
07:00 AM	0	21	15	9	44	0	0	0	0	0	0	0	83
07:15 AM	0	31	15	4	29	0	0	0	0	0	0	0	117
07:30 AM	0	17	37	Ŋ	49	0	0	0	0	0	0	0	108
07:45 AM	0	4	51	r)	48	0	0	0	0	0	0	0	116
Total	0	83	118	15	208	0	0	0	0	0	0	0	424
08:00 AM	0	12	69	0	52	0	0	0	0	0	0	0	133
08:15 AM	0	15	79	ю	39	0	0	0	0	0	0	0	136
08:30 AM	0	11	15	က	35	0	0	0	0	0	_	0	65
08:45 AM	0	7	1	ო	36	0	0	0	0	0	0	0	22
Total	0	45	174	6	162	0	0	0	0	0	77	0	391
Grand Total	0	128	292	24	370	0	0	0	0	0	~	0	815
Apprch %	0	30.5	69.5	6.1	93.9	0	0	0	0	0	100	0	
Total %	0	15.7	35.8	2.9	45.4	0	0	0	0	0	0.1	0	

File Name 75860011 Site Code 75860011 Start Date 1/25/2018 Page No 9

	OI	Cottage St From North		Σű	Maverick St From East	St		Cottage St From South		Ma	Maverick St From West		
Start Time	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Int. Total
07:00 AM	0	-	က	0	~	0	0	0	0	0	0	0	C)
07:15 AM	0	0	,-	0	2	0	0	0	0	0	0	0	8
07:30 AM	0	0	-	0	7-	0	0	0	0	0	0	0	2
07:45 AM	0	÷	-	0	က	0	0	0	0	0	0	0	2
Total	0	2	Ø	0	7	0	0	0	0	0	0	0	15
08:00 AM	0	0	-	0		0	0	0	0	0	0	0	2
08:15 AM	0	0	_	0	0	0	0	0	0	0	0	0	_
08:30 AM	0	0	0	0	က	0	0	0	0	0	0	0	e
08:45 AM	0	-	0	0	2	0	-	0	0	0	0	0	4
Total	0	-	2	0	9	0	<u>*</u>	0	0	0	0	0	10
Grand Total	0	က	Ø	0	13	0	-	0	0	0	0	0	25
Apprch %	0	27.3	72.7	0	100	0	100	0	0	0	0	0	
Total %	0	12	32	0	52	0	4	0	0	0	0	0	

File Name : 75860011 Site Code : 75860011 Start Date : 1/25/2018 Page No : 13

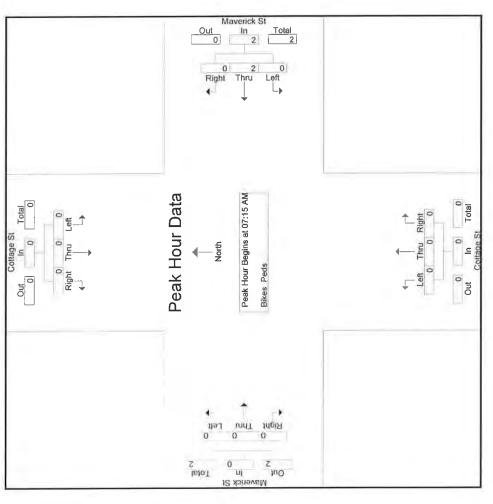
								Groups	Groups Printed- Bikes Peds	3ikes Ped	S								
		Cottage St From North	ge St North			Maverick St From East	k St east			Cottage St From South	ま			Maverick St From West	k St est				
Start Time	Left	Thru	Right	Peds	Left	Thru	Right	Peds	Leff	Thru	Ħ	Peds	Leff	Thru	Right	Peds	Exclu. Total Inclu. Total	Inclu, Total	Int. Total
07:00 AM	0	0	0	ω	0	0	0	eo	0	0	0	60	0	0	0	2	19	0	19
07:15 AM	0	0	0	Ŋ	0	0	0	0	0	0	0	5	0	0	0	9	16	0	16
07 30 AM	0	0	0	7	0	0	0	T	0	0	0	2	0	0	0	4	4	0	14
07:45 AM	0	0	0	20	0	←	0	00	0	0	0	00	0	0	0	14	20	r	51
Total	0	0	0	38	0	-	0	12	0	0	0	23	0	0	0	56	66	STEE	100
08 00 AM	0	0	9)	24	0	Ψ.	0	9	0	0	0	15	0	0	0	17	62	τ.	63
08:15 AM	0	0	0	25	0	0	0	co	0	0	0	17	0	0	0	34	œ 1	0	81
08 30 AM	0	0	0	1	0	0	0	2	0	0	0	4	0	0	0	15	32	0	32
08:45 AM	0	0	0	4	0	0	0	Ŋ	0	0	0	7	0	0	0	00	24	0	24
Total	0	0	0	64	0	. 	0	18	0	0	0	43	0	0	0	74	199	æ	200
F	(C	(Ć	(Ó	C	Ć	C	ć	9	(¢	9	6	•	6
Grand Lotal	0	0	0	701	0	7	O	30	0	>	0	8	ò	0	0	2	298	7	300
Apprch %	0	0	0		0	100	0		0	0	0		0	0	0				
Total %	0	0	0		0	100	0		0	0	0		0	0	0		99.3	0.7	

File Name : 75860011 Site Code : 75860011 Start Date : 1/25/2018 Page No : 14

		From	From North			Maverick St From East	ick St East			Cottage St From South	ge St South			Mave From	Maverick St From West		
Start Time	Left		Thru Right App. Total	vpp. Total	Left	Thru	Right A	Right App. Total	Left	Thru	Right	Right App. Total	Left	Thru	Right	Right App. Total	Int. Total
Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1	om 07:00	AM to 08:	45 AM - Peal	k 1 of 1													
Peak Hour for Entire Intersection Begins at 07:15 AM	ntersection	า Begins ลเ	t 07:15 AM														
07:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
07:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
07:45 AM	0	0	0	0	0	-	0	-	0	0	0	0	0	0	0	0	
08:00 AM	0	0	0	0	0	-	0	-	0	0	0	0	0	0	0	0	
Total Volume	0	0	0	0	0	2	0	2	0	0	0	0	0	0	0	0	2
% App. Total	0	0	0		0	100	0		0	0	0		0	0	0		
PHF	000	000	000	000	000	.500	000.	.500	000	000	000	000	000.	000	000	000.	.500

File Name : 75860011 Site Code : 75860011 Start Date : 1/25/2018 Page No : 15

N/S Street : Cottage Street E/W Street : Maverick Street City/State : Boston, MA Weather : Clear



Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1

	07:00 AM			10	:15 AM			07	:00 AM			07	:00 AM			
+0 mins.	0	0	0	0	0	0	0	0	0	0	0	0	0		0	0
+15 mins.	0	0	0	0	0	0	0	0	0	0	0	0	0		0	0
+30 mins.	0	0	0	0	0	-	0	_	0	0	0	0	0		0	0
+45 mins.	0	0	0	0	0	_	0	-	0	0	0	0	0		0	0
Total Volume	0	0	0	0	0	2	0	2	0	0	0	0	0		0	0
% App. Total	0	0	0		0 10	00	0		0	0	0 0		0	0	0	

File Name : 75860011 Site Code : 75860011 Start Date : 1/25/2018 Page No : 1

Start Time Left :: 04:00 PM 0 0 04:15 PM 0 0 04:45 PM 0 0 Total 0 05:00 PM 0 05:15 PM 0 05:30 PM 0 0	From North Thru 17 26 24 29	Right	ŭ	From East					in the second	From Man		
Let		Right				Fr	From South			TOTA VVEST		
	17 26 24 29		Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Int. Total
	26 24 29	6	က	44	0	0	0	0	0	0	0	73
	24	41	7	42	0	0	0	0	0	0	0	89
	58	33	ო	58	0	0	0	0	0	0	0	118
		12	9	41	0	0	0	0	0	0	0	88
	96	89	19	185	0	0	0	0	0	0	0	368
	18	41	က	42	0	0	0	0	0	0	0	77
	20	36	12	38	0	0	0	0	0	0	0	106
	27	o	2	48	0	0	0	0	0	0	0	98
05:45 PM 0	26	12	Ω	50	0	0	0	0	0	0	0	93
Total 0	91	71	22	178	0	0	0	0	0	0	0	362
Grand Total 0	187	139	41	363	0	0	0	0	0	0	0	730
Apprch % 0	57.4	42.6	10.1	89.9	0	0	0	0	0	0	0	
Total % 0	25.6	19	5.6	49.7	0	0	0	0	0	0	0	
Cars 0	187	137	41	353	0	0	0	0	0	0	0	718
% Cars 0	100	98.6	100	97.2	0	0	0	0	0	0	0	98.4
Trucks 0	0	2	0	10	0	0	0	0	0	0	0	12
% Trucks 0	0	4.1	0	2.8	0	0	0	0	0	0	0	1.6

File Name : 75860011 Site Code : 75860011 Start Date : 1/25/2018 Page No : 2

Int. Total

118 88 77 106

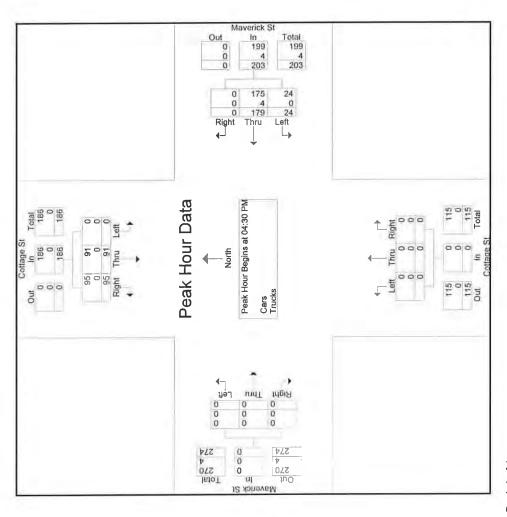
389

.824 99.0

		Cottage St	je St			Maverick St	ick St			Cotta	Cottage St			Maverick St	ick St	
		From North	North			From East	East			From South	South			From West	West	
Start Time	Left	Thru	Right	Right App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total
Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1	n 04:00 F	M to 05:4.	5 PM - P	eak 1 of 1	3											
Peak Hour for Entire Intersection Begins at 04:30 PM	rsection	Begins at	04:30 PN	_												
04:30 PM	0	24	33	22	က	28	0	61	0	0	0	0	0	0	0	0
04:45 PM	0	29	12	41	9	41	0	47	0	0	0	0	0	0	0	0
05:00 PM	0	18	4	32	က	42	0	45	0	0	0	0	0	0	0	0
05:15 PM	0	20	36	99	12	38	0	20	0	0	0	0	0	0	0	0
Total Volume	0	91	92	186	24	179	0	203	0	0	0	0	0	0	0	0
% App. Total	0	48.9	51.1		11.8	88.2	0		0	0	0		0	0	0	
PHF	000.	.784	099.	.816	.500	.772	000.	.832	000.	000	000.	000.	000.	000.	000	000.
Cars	0	91	95	186	24	175	0	199	0	0	0	0	0	0	0	0
% Cars	0	100	100	100	100	97.8	0	98.0	0	0	0	0	0	0	0	0
Trucks	0	0	0	0	0	4	0	4	0	0	0	0	0	0	0	0
% Trucks	0	0	0	0	0	2.2	0	2.0	0	0	0	0	0	0	0	0

File Name 75860011 Site Code 75860011 Start Date 1/25/2018 Page No 3

N/S Street: Cottage Street E/W Street: Maverick Street City/State: Boston, MA Weather: Clear



Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1

	04:30 PM				04:00 PM			0	04:00 PM			0	04:00 PM			
+0 mins.	0	24	33	22	က	44	0	47	0	0	0	0	0	0	0	0
+15 mins.	0	29	12	4	7	42	0	49	0	0	0	0	0	0	0	0
+30 mins.	0	92	4	32	က	28	0	61	0	0	0	0	0	0	0	0
+45 mins.	0	20	36	99	9	41	0	47	0	0	0	0	0	0	0	0
Total Volume	0	91	95	186	19	185	0	204	0	0	0	0	0	0	0	0
% App. Total	0	48.9	51.1		9.3	2.06	0		0	0	0		0	0	0	

File Name 75860011 Site Code 75860011 Start Date 1/25/2018 Page No 5

		Cottage St From North		Σπ	Maverick St From Fast	# .		Cottage St From South		M.	Maverick St From West		
Start Time	Left		Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Int. Total
04:00 PM	0	17	6	8	42	0	0	0	0	0	0	0	71
04:15 PM	0	26	13	7	41	0	0	0	0	0	0	0	87
04:30 PM	0	24	33	က	57	0	0	0	0	0	0	0	117
04:45 PM	0	59	12	9	40	0	0	0	0	0	0	0	87
Total	0	96	29	19	180	0	0	0	0	0	0	0	362
05:00 PM	0	18	4	က	41	0	0	0	0	0	0	0	92
05:15 PM	0	20	36	12	37	0	0	0	0	0	0	0	105
05:30 PM	0	27	00	2	46	0	0	0	0	0	0	0	83
05:45 PM	0	26	12	5	49	0	0	0	0	0	0	0	92
Total	0	91	70	22	173	0	0	0	0	0	0	0	356
Grand Total	0	187	137	14	353	0	0	0	0	0	0	0	718
Apprch %	0	57.7	42.3	10.4	89.6	0	0	0	0	0	0	0	
Total %	0	26	19.1	5.7	49.2	0	0	0	0	0	0	0	

File Name : 75860011 Site Code : 75860011 Start Date : 1/25/2018 Page No : 9

					Group	s Printed-Tru	icks						
	ΟĒ	Cottage St From North		ğű	Maverick St From Fast			Cottage St From South		Σü	Maverick St From West		
Start Time	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Int. Total
04:00 PM	0	0	0	0	2	0	0	0	0	0	0	0	2
04:15 PM	0	0	~	0	÷	0	0	0	0	0	0	0	2
04:30 PM	0	0	0	0	~	0	0	0	0	0	0	0	_
04:45 PM	0	0	0	0	7	0	0	0	0	0	0	0	_
Total	0	0	-	0	ıc	0	0	0	0	0	0	0	9
05:00 PM	0	0	0	0	←	0	0	0	0	0	0	0	-
05:15 PM	0	0	0	0	-	0	0	0	0	0	0	0	+
05:30 PM	0	0	-	0	2	0	0	0	0	0	0	0	ю
05:45 PM	0	0	0	0	~	0	0	0	0	0	0	0	-
Total	0	0	~	0	D.	0	0	0	0	0	0	0	9
Grand Total	0	0	2	0	10	0	0	0	0	0	0	0	12
Apprch %	0	0	100	0	100	0	0	0	0	0	0	0	
Total %	0	0	16.7	0	83.3	0	0	0	0	0	0	0	

File Name : 75860011 Site Code : 75860011 Start Date : 1/25/2018 Page No : 13

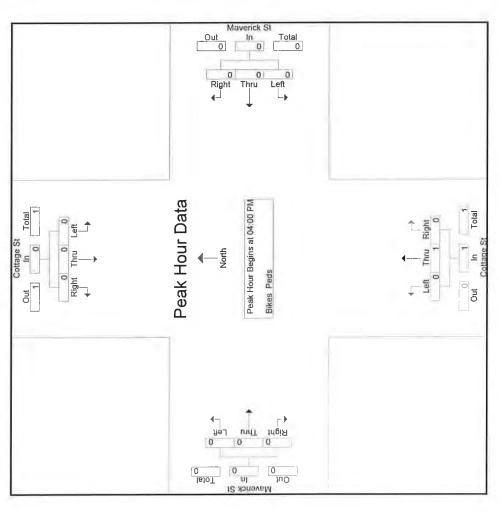
								Group	Groups Printed- Bikes Peds	Bikes Pe	ds								
		Cottage St From North	Cottage St From North			Maverick St From East	sk St ast			Cottage St From South	o St			Maverick St From West	k St est				
Start Time	Left	Thru	Right	Peds	Left	Thru Righ	Right	Peds	Left	Thru	Right	Peds	Left	Thru	Right	Peds	Exclu. Total Inclu. Total		Int. Total
04:00 PM	0	0	0	4	0	0	0	က	0	0	0	80	0	0	0	4	19	0	19
04:15 PM	0	0	0	6	0	0	0	7	0	0	0	65	0	0	0	m	26	0	26
04:30 PM	0	0	0	2	0	0	0	2	0	0	0	7	0	0	0	7	22	0	22
04:45 PM	0	0	0	ø	0	0	0	-	0	_	0	00	0	0	0	2	17	5	18
Total	0	0	0	15	0	0	0	13	0	*	0	36	0	0	0	20	84	-	85
05:00 PM	0	0	0	10	0	0	0	က	0	0	0	12	0	0	0	m	28	0	28
05:15 PM	0	0	0	4	0	0	0	ന	0	0	0	∞	0	0	0	10	35	0	35
05:30 PM	0	0	0	12	0	0	0	7	0	0	0	o	0	0	0	ю	31	0	31
05:45 PM	0	0	0	11	0	0	0	7	0	0	0	17	0	0	0	5	40	0	40
Total	0	0	0	47	0	0	0	20	o	0	0	46	0	0	0	21	134	0	134
Grand Total	0	0	0	62	0	0	0	33	0	7	0	82	0	0	0	4	218	~	219
Apprch %	0	0	0		0	0	0		0	100	0		0	0	0				
Total %	0	0	0		0	0	0		0	100	0		0	0	0		99.5	0,5	

File Name : 75860011 Site Code : 75860011 Start Date : 1/25/2018 Page No : 14

		From North	From North			From East	East			Cottage St From South	ge St South			Maverick St From West	nck St West		
Start Time	Left	Thru	Right	Left Thru Right App. Total	Left	Thru	Right /	Right App. Total	Left	Thru	Right	Right App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1	om 04:00	PM to 05:4	5 PM - Pe	ak 1 of 1													
Peak Hour for Entire Intersection Begins at 04:00 PM	ntersection	Begins at	04:00 PM														
04:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	Ü
04:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	J
04:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	Ü
04:45 PM	0	0	0	0	0	0	0	0	0	-	0	_	0	0	0	0	
Total Volume	0	0	0	0	0	0	0	0	0	-	0	-	0	0	0	0	
% App. Total	0	0	0		0	0	0		0	100	0		0	0	0		
PHF	000.	000	000	000	000	000	000	000	000	.250	000	.250	000	UUU	000	000	250

File Name 75860011 Site Code 75860011 Start Date 1/25/2018 Page No 15

N/S Street: Cottage Street E/W Street: Maverick Street City/State: Boston, MA Weather: Clear



Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1

Ō	04:00 PM			04	04:00 PM			Ö	04:00 PM			70	04:00 PM			
+0 mins.	0	0	0	0	0	0	0	0	0	0	0		0	0	0	0
+15 mins.	0	0	0	0	0	0	0	0	0 0	0	0		0 0	0	0	0
+30 mins.	0	0	0	0	0	0	0	0	0	0	0		0	0	0	0
+45 mins.	0	0	0	0	0	0	0	0	0	-	0		0	0	0	0
otal Volume	0	0	0	0	0	0	0	0	0	-	0		0	0	0	0
% App. Total	0	0	0		0	0	0		0	100	0		0	0	0	

File Name 75860012 Site Code 75860012 Start Date 1/25/2018 Page No 1

	Li.	Bremen St From North		Ma	Maverick St From East	ck St East		Bremen St From South		Σπ	Maverick St From West		
Start Time	Left	Thru	Right	Leff	Thru	Right	Left	Thru	Right	Left	Thru	Right	Int. Total
07:00 AM	0	56	o	2	52	16	00	20	0	0	0	0	133
07:15 AM	0	13	13	ო	63	16	00	27	0	0	0	0	143
07:30 AM	0	19	12	2	46	21	11	16	0	0	0	0	127
07:45 AM	0	23	17	-	54	18	က	14	0	0	0	0	130
Total	0	8	51	ω	215	7.1	30	77	0	0	0	0	533
08:00 AM	0	13	21	9	70	14	4	17	0	0	0	0	145
08:15 AM	0	21	4	4	78	16	9	19	0	0	0	0	148
08:30 AM	0	σ	11	Ŋ	52	7	10	16	0	0	0	0	110
08:45 AM	0	15	18	0	47	13	2	22	0	0	0	0	117
Total	0	28	54	15	247	20	22	74	0	0	0	0	520
Grand Total	0	139	105	23	462	121	52	151	0	0	0	0	1053
Apprch %	0	22	43	3.8	76.2	20	25.6	74.4	0	0	0	0	
Total %	0	13.2	10	2.2	43.9	11.5	4.9	14.3	0	0	0	0	
Cars	0	132	103	23	445	120	52	147	0	0	0	0	1022
% Cars	0	95	98.1	100	96.3	99.2	100	97.4	0	0	0	0	97.1
Trucks	0	2	2	0	17	-	0	4	0	0	0	0	31
% Trucks	0	2	1.9	0	3.7	0.8	0	2.6	0	0	0	0	2.9

File Name : 75860012 Site Code : 75860012 Start Date : 1/25/2018 Page No : 2

N/S Street: Bremen Street E/W Street: Maverick Street City/State: Boston, MA Weather: Clear

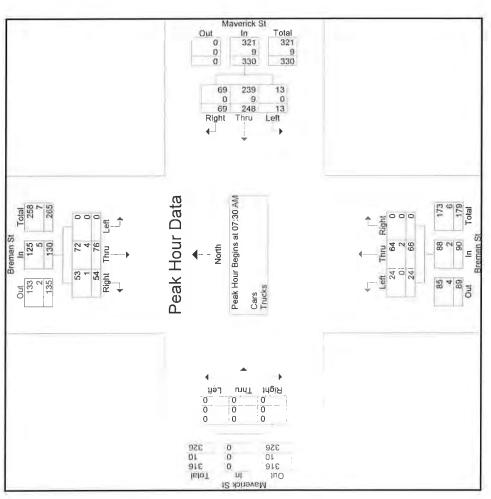
		Bremen St	Bremen St From Morth			Maverick St	Naverick St From East			Bremen St	remen St			Maver	Maverick St		
Start Time Left	Left	Thru	Right	Thru Right App. Total	Left	Thru	Right	Right App. Total	Left	Thru	Right	Thru Right App. Total	Left	Thru	Right	Right App. Total	Int. To

Peak F Peak I

	Int. Total			127	130	145	148	550		.929	534	97.1	16	2.9
	App. Total			0	0	0	0	0		000.	0	0	0	0
ck St Vect	喜			0	0	0	0	0	0	000	0	0	0	0
Maverick St From West	Thru			0	0	0	0	0	0	000.	0	0	0	0
	Left			0	0	0	0	0	0	000.	0	0	0	0
	App. Total			27	17	21	25	06		.833	88	87.8	2	2.2
in St	Ħ			0	0	0	0	0	0	000.	0	0	0	0
Bremen St From South	Thru			16	14	17	19	99	73.3	898.	64	97.0	2	3.0
	Left			11	က	4	9	24	26.7	.545	24	100	0	0
	App. Total			69	73	06	86	330		.842	321	97.3	o	2.7
ck St	Ħ			21	18	14	16	69	20.9	.821	69	100	0	0
Maverick St From Fast	Thru			46	54	20	78	248	75.2	795	239	96.4	6	3.6
	Left			2	~	9	4	13	3.9	.542	13	100	0	0
	Right App. Total	ak 1 of 1		31	40	34	25	130		.813	125	96.2	5	3.8
in St Jorth	Right	5 AM - Pe	07:30 AM	12	17	21	4	54	41.5	.643	53	98.1	_	1.9
Bremen St From North	Thru	M to 08:4.	Begins at	19	23	13	21	9/	58.5	.826	72	94.7	4	5.3
	Left	m 07:00 A	ersection l	0	0	0	0	0	0	000.	0	0	0	0
	Start Time	ak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1	ak Hour for Entire Intersection Begins at 07:30 AM	07:30 AM	07:45 AM	08:00 AM	08:15 AM	Total Volume	% App. Total	PHF	Cars	% Cars	Trucks	% Trucks

File Name 75860012 Site Code 75860012 Start Date 1/25/2018 Page No 3

N/S Street: Bremen Street E/W Street: Maverick Street City/State: Boston, MA Weather: Clear



Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1

07	07:00 AM			.0	07:30 AM				07:00 AM			0	07:00 AM			
+0 mins.	0	26	6	35	7	46	21	69		20	0	28	0	0	0	0
+15 mins.	0	13	13	56	-	54	18	73		27	0	35	0	0	0	0
+30 mins.	0	19	12	31	9	70	14	06		16	0	27	0	0	0	0
+45 mins.	0	23	17	40	4	78	16	86	. m	14	0	17	0	0	0	0
otal Volume	0	81	51	132	13	248	69	330		77	0	107	0	0	0	0
% App. Total	0	61.4	38.6		3.9	75.2	20.9		28	72	0		0	0	0	

File Name: 75860012 Site Code: 75860012 Start Date: 1/25/2018 Page No: 5

					Grou	Groups Printed- Cars	ars						
		Bremen St From North		Σπ	Maverick St From East			Bremen St From South		_	Maverick St From West		
Start Time	r Tett	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Int. Total
07:00 AM	0	24	0	2	20	16	00	19	0	0	0	0	128
07:15 AM	0	13	13	က	09	15	∞	27	0	0	0	0	139
07:30 AM	0	18	=======================================	2	45	21		16	0	0	0	0	124
07:45 AM	0	23	17	-	50	18	က	14	0	0	0	0	126
Total	0	78	90	∞	205	70	30	92	0	0	0	0	517
08:00 AM	0	11	21	9	69	4	4	16	0	0	0	0	141
08:15 AM	0	20	4	4	75	16	9	18	0	0	0	0	143
08:30 AM	0	O	=	5	51	7	10	15	0	0	0	0	108
08:45 AM	0	14	17	0	45	13	2	22	0	0	0	0	113
Total	0	54	53	15	240	50	22	7.1	0	0	0	0	505
Grand Total	0	132	103	23	445	120	52	147	0	0	0	0	1022
Apprch %	0	56.2	43.8	3.9	75.7	20.4	26.1	73.9	0	0	0	0	
Total %	0	12.9	10.1	2.3	43.5	11.7	5.1	14.4	0	0	0	0	

File Name 75860012 Site Code 75860012 Start Date 1/25/2018 Page No 9

					Group	s Printed-Tru	cks						
	P.T.	Bremen St From North		~ =	Maverick St From East			Bremen St From South		Ž Ľ	Maverick St From West		
Start Time	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Int. Total
07:00 AM	0	2	0	0	2	0	0	_	0	0	0	0	ιΩ
07:15 AM	0	0	0	0	r	-	0	0	0	0	0	0	4
07:30 AM	0		-	0	-	0	0	0	0	0	0	0	က
07:45 AM	0	0	0	0	4	0	0	0	0	0	0	0	4
Total	0	က	٠	0	10	~	0	~	0	0	0	0	16
08:00 AM	0	2	0	0	~	0	0	-	0	0	0	0	4
08:15 AM	0	—	0	0	က	0	0	÷	0	0	0	0	72
08:30 AM	0	0	0	0	7-	0	0		0	0	0	0	2
08:45 AM	0	-	_	0	2	0	0	0	0	0	0	0	4
Total	0	4	+	0	7	0	0	က	0	0	0	0	15
Grand Total	0	7	2	0	17	*	0	4	0	0	0	0	31
Apprch %	0	77.8	22.2	0	94.4	5.6	0	100	0	0	0	0	
Total %	0	22.6	6.5	0	54.8	3.2	0	12.9	0	0	0	0	

File Name : 75860012 Site Code : 75860012 Start Date : 1/25/2018 Page No : 13

								Groups	Groups Printed- Bikes Peds	Bikes Per	qs								
		Bremen St From North	orth			Maverick St From East	sk St Fast			Bremen St From South	outh			Maverick St From West	k St fest				
Start Time	Left	Thru	Right	Peds	Left	Thru	Right	Peds	Left	Thru	Right	Peds	Left	Thru	Right	Peds	Exclu. Total Inclu. Total		Int. Total
07:00 AM	0	0	0	16	0	0	0	ო	0	0	0	36	0	0	0	80	63	0	63
07:15 AM	0	0	0	23	0	0	0	0	0	0	0	26	0	0	0	12	61	0	61
07:30 AM	0	0	0	20	0	0	0	-	0	0	0	40	0	0	0	7	99	0	68
07:45 AM	0	0	0	28	0	0	0	e	0	0	0	42	0	0	0	17	06	0	06
Total	0	0	0	87	0	0	0	7	0	0	0	144	0	0	0	44	282	0	282
08:00 AM	0	0	0	38	0	0	0	9	0	0	0	25	0	0	0	16	115	0	115
08:15 AM	0	0	0	32	0	0	0	7	0	0	0	55	0	0	0	14	103	0	103
08:30 AM	0	0	0	30	0	0	0	0	0	0	0	37	0	0	0	15	82	0	82
08:45 AM	0	0	0	17	0	0	0	т	0	0	0	31	0	0	0	თ	09	0	09
Total	0	0	0	117	0	0	0	-	0	0	0	178	0	0	0	54	360	0	360
Grand Total	0	0	0	204	0	0	0	18	0	0	0	322	0	0	0	86	642	0	642
Apprch % Total %	0	0	0		0	0	0		0	0	0		0	0	0		100	0	

File Name : 75860012 Site Code : 75860012 Start Date : 1/25/2018 Page No : 14

N/S Street: Bremen Street E/W Street: Maverick Street City/State: Boston, MA Weather: Clear

		From	From North				From East	∃ast			From	From South			From West	West		
Start Time		Left Thru Right App. Total	Right	App. To		Left	Thru	Right	Right App. Total	Left	Thru	Right	Right App. Total	Left	Thru	Right	Thru Right App. Total Int. Total	Int. Tota
Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1	* From 07:00	AM to 08.	45 AM - P	Post 1 of	7													

		Bren	Bremen St			Maver	Naverick St			Bremen St	an St			Maverick St	rick St		
		From	From North			From East	East			From South	South			From West	West		
Start Time Left Thru Right App. Total	Left	Thru	Right A	op. Total	Left	Thru	Right	Right App. Total	Left	Thru	Right	Right App. Total	Left	Thru	Right	Right App. Total	ī.
eak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1	om 07:00,	AM to 08:4	45 AM - Peak	1 of 1													
eak Hour for Entire Intersection Begins at 07:00 AM	tersection	Begins at	: 07:00 AM														
07:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
07:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
07:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
07:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
Total Volume	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
% App. Total	0	0	0		0	0	0		0	0	0		0	0	0		
PHF	000	000	000	0000	000.	000	000.	000.	000.	000.	000	000	000.	000.	000	000	

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File Name: 75860012 Site Code: 75860012 Start Date: 1/25/2018 Page No: 15

N/S Street: Bremen Street E/W Street: Maverick Street City/State: Boston, MA Weather: Clear

Maverick St Out Total 0 Right Thru Left Total Peak Hour Data Peak Hour Begins at 07:00 AM Total Bremen St In 0 North Out Out **Bikes Peds** Right 0 O Right Total Mavenck St Out 0

Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1

0	07:00 AM			07	07:00 AM			0	07:00 AM			0	07:00 AM			
+0 mins.	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
+15 mins.	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
+30 mins.	0	0	0	0	0	0	0	0	0	0	0	0	0 0	0	0	J
+45 mins.	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	J
otal Volume	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
App. Total	0	0	0		0	0	0		0	0	0		0	0	0	

N/S Street: Bremen Street E/W Street: Maverick Street City/State: Boston, MA Weather: Clear

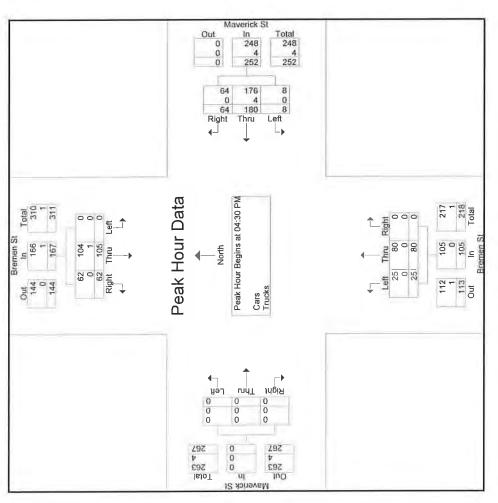
File Name | 75860012 Site Code | 75860012 Start Date | 1/25/2018 Page No | 1

		č			Groups Printed- Cars - Trucks	rinted- Cars -		ò			:		
-	- Ц	Bremen St From North		≥	Naverick St From East			Bremen St From South		Ĭ,	Maverick St From West		
Start Time	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Int. Total
04:00 PM	0	23	12	2	39	Ξ	2	15	0	0	0	0	107
04:15 PM	0	26	18	က	27	4	4	16	0	0	0	0	108
04:30 PM	0	23	13	~	51	19	S	26	0	0	0	0	138
04:45 PM	0	20	14	7	52	1	7	18	0	0	0	0	124
Total	0	92	22	Ξ	169	55	8	75	0	0	0	0	477
05:00 PM	0	33	19	7	29	16	5	18	0	0	0	0	122
05:15 PM	0	29	16	ო	48	18	80	18	0	0	0	0	140
05:30 PM	0	31	15	2	40	15	7	17	0	0	0	0	127
05:45 PM	0	28	10	4	49	00	80	16	0	0	0	0	123
Total	0	121	09	-	166	25	28	69	0	0	0	0	512
Grand Total	0	213	117	22	335	112	46	144	O	o	C	O	686
Apprch %	0	64.5	35.5	4.7	71.4	23.9	24.2	75.8	0	0	0	0	}
Total %	0	21.5	11.8	2.2	33.9	11.3	4.7	14.6	0	0	0	0	
Cars	0	212	116	22	325	112	46	144	0	0	0	0	226
% Cars	0	99.5	99.1	100	97	100	100	100	0	0	0	0	98.8
Trucks	0	-	-	0	10	0	0	0	0	0	0	0	12
% Trucks	0	0.5	6.0	0	က	0	0	0	0	0	0	0	1.2

File Name : 75860012 Site Code : 75860012 Start Date : 1/25/2018 Page No : 2

		Brem	Bremen St			Maver	rerick St			Bremen St	an St			Mave	Maverick St		
		From	From North			From	ım East			From South	South			From	From West		
Start Time	Left	Thru	Right	Thru Right App. Total	Left	Thru	Right	Right App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1	om 04:00	PM to 05:4	45 PM - Pe	eak 1 of 1													
Peak Hour for Entire Intersection Begins at 04:30 PM	ntersection	Begins at	04:30 PN	_													
04:30 PM	0	23	13	36	-	51	19	77	5	26	0	31	0	0	0	0	138
04:45 PM	0	20	4	34	2	52	7	65	7	18	0	25	0	0	0	0	124
05:00 PM	0	33	19	52	2	59	16	47	5	18	0	23	0	0	0	0	122
05:15 PM	0	29	16	45	က	48	18	69	œ	18	0	26	0	0	0	0	140
Total Volume	0	105	62	167	80	180	64	252	25	80	0	105	0	0	0	0	524
% App. Total	0	62.9	37.1		3.2	71.4	25.4		23.8	76.2	0		0	0	0		
PHF	000	795	.816	.803	799.	3865	.842	788.	.781	.769	000	.847	000.	000	000	000.	.936
Cars	0	104	62	166	00	176	64	248	25	80	0	105	0	0	0	0	519
% Cars	0	0.66	100	99.4	100	8.78	100	98.4	100	100	0	100	0	0	0	0	99.0
Trucks	0	_	0	-	0	4	0	4	0	0	0	0	0	0	0	0	2
% Trucks	0	1.0	0	9.0	0	2.2	0	1.6	0	0	0	0	0	0	0	0	1.0

File Name 75860012 Site Code 75860012 Start Date 1/25/2018 Page No 3



Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1

at:
Begins
Approach
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공
Peak

05:00 PM	Mc		0	04:30 PM			0	04:30 PM			0	04:00 PM			
+0 mins.	0 33	19	52	~	51	19	7.1	5	26	0	31	0	0	0	0
+15 mins.	0 29	16	45	2	52	-	65	7	18	0	25	0	0	0	0
+30 mins.	0 31	15	46	2	53	16	47	5	18	0	23	0	0	0	0
+45 mins.	0 28	10	38	ო	48	18	69	00	18	0	56	0	0	0	0
Fotal Volume	0 121	09	181	•••	180	64	252	25	80	0	105	0	0	0	0
% App. Total	0 66.9	33.1		3.2	71.4	25.4		23.8	76.2	0		0	0	0	

File Name : 75860012 Site Code : 75860012 Start Date : 1/25/2018 Page No : 5

					Grou	Groups Printed- Cars	Cars						
		Bremen St From North		Σu	Maverick St From East			Bremen St From South		Ma	Maverick St From West		
Start Time	Left	Thru	Right	Leff	Thru	Right	Left	Thru	Right	Left	Thru	Right	Int. Total
04:00 PM	0	23	12	2	37	7	2	15	0	0	0	0	105
04:15 PM	0	26	18	က	25	14	4	16	0	0	0	0	106
04:30 PM	0	23	13	-	90	19	ις	26	0	0	0	0	137
04:45 PM	0	20	4	2	51	7	7	18	0	0	0	0	123
Total	0	92	25	7	163	55	18	75	0	0	0	0	471
05:00 PM	0	33	19	2	28	16	ιO	18	0	0	0	0	121
05:15 PM	0	28	16	က	47	9	80	18	0	0	0	0	138
05:30 PM	0	31	4	2	39	15	7	17	0	0	0	0	125
05:45 PM	0	28	10	4	48	80	80	16	0	0	0	0	122
Total	0	120	59	11	162	57	28	69	0	0	0	0	506
Grand Total	0	212	116	22	325	112	46	144	0	0	0	0	977
Apprch %	0	64.6	35.4	4.8	70.8	24.4	24.2	75.8	0	0	0	0	
Total %	0	21.7	11.9	2.3	33.3	11.5	4.7	14.7	0	0	0	0	

File Name : 75860012 Site Code : 75860012 Start Date : 1/25/2018 Page No : 9

	يل بد	Bremen St From North		Σu	Maverick St From East	Stooppe rinited rincks St		Bremen St From South		Σπ	Maverick St From West		
Start Time	Leff	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Int. Total
04:00 PM	0	0	0	0	7	0	0	0	0	0	0	0	2
04:15 PM	0	0	0	0	2	0	0	0	0	0	0	0	2
04:30 PM	0	0	0	0	-	0	0	0	0	0	0	0	7
04:45 PM	0	0	0	0	~~	0	0	0	0	0	0	0	_
Total	0	0	0	0	O	0	0	0	0	0	0	0	9
05:00 PM	0	0	0	0	-	0	0	0	0	0	0	0	-
05:15 PM	0	-	0	0	-	0	0	0	0	0	0	0	2
05:30 PM	0	0	7-	0	F	0	0	0	0	0	0	0	2
05:45 PM	0	0	0	0	+	0	0	0	0	0	0	0	+
Total	0	+		0	4	0	0	0	0	0	0	0	9
Grand Total	0	_	-	0	10	0	0	0	0	0	0	0	12
Apprch %	0	20	20	0	100	0	0	0	0	0	0	0	
Total %	0	8.3	8.3	0	83.3	0	0	0	0	0	0	0	

File Name : 75860012 Site Code : 75860012 Start Date : 1/25/2018 Page No : 13

		4	i					Groups	Groups Printed- Bikes Peds	Bikes Pec	Sp	Ì							
		Bremen St From North	ist ell			Maverick St From East	k St ast			Bremen St From South	ı St outh			Maverick St From West	est est				
Start Time	Left	Thru	Thru Right	Peds	Left	Thru Right	Right	Peds	Left	Thru	Right	Peds	Left	Thru	Right	Peds	Exclu. Total Inclu. Total	Inclu. Total	Int. Total
04:00 PM	0	0	0	19	0	0	0	ന	0	0	0	56	0	0	0	တ	57	0	57
04:15 PM	0	0	0	56	0	0	0	Ŋ	0	acri	0	42	0	0	0	00	81	<i>T</i> -	82
04:30 PM	0	0	-	20	0	0	0	4	0	0	0	36	0	0	0	Ø	69	X-	70
04:45 PM	0	0	0	17	0	0	0	+	0	0	0	34	0	0	0	D	61	0	61
Total	0	0	-	82	0	0	0	<u>£</u>	0	Tro sec ci	0	138	0	0	0	35	268	8	270
05:00 PM	0	3 6 2	0	38	0	FT .	0	9	0	0	0	45	0	0	0	F	100	2	102
05:15 PM	0	0	0	56	0	0	0	က	0	0	0	47	0	0	0	7	83	0	83
05:30 PM	0	0	0	27	0	0	0	7	0	0	0	38	0	0	0	12	79	0	79
05:45 PM	0	0	4	24	0	o se s	0	4	0	0	0	39	0	0	0	o	9/	2	78
Total	0		*	115	0	7	0	5	0	0	0	169	0	0	0	39	338	4	342
Grand Total	0	~	2	197	0	2	0	28	0	-	0	307	0	0	0	74	909	9	612
Apprch %	0	33.3	2.99		0	100	0	_	0	100	0		0	0	0				
Total %	0	16.7	33.3		0	33.3	0		0	16.7	0		0	0	0		66	-	

File Name : 75860012 Site Code : 75860012 Start Date : 1/25/2018 Page No : 14

	App. Total Int. Total			0	0	0 0	0 2	0 4		
West	Right			0	0	0	0	0	0	
From West	Thru			0	0	0	0	0	0	
	Left			0	0	0	0	0	0	
	Right App. Total			_	0	0	0	-		
South	Right			0	0	0	0	0	0	
From South	Thru			-	0	0	0	-	100	
	Left			0	0	0	0	0	0	
	App. Total			0	0	0	-	-		
East	Right /			0	0	0	0	0	0	
From East	Thru			0	0	0	-	۳	100	
	Left			0	0	0	0	0	0	
	. Total	of 1		0	_	0	-	2		
Vorth	Thru Right, App. Total	5 PM - Peak 1	04:15 PM	0	-	0	0	-	20	
From North	Thru	M to 05:4:	Begins at t	0	0	0	_	-	20	
	Left	m 04:00 P	ersection t	0	0	0	0	0	0	
	Start Time	Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1	Peak Hour for Entire Intersection Begins at 04:15 PM	04:15 PM	04:30 PM	04:45 PM	05:00 PM	Total Volume	% App. Total	1 2

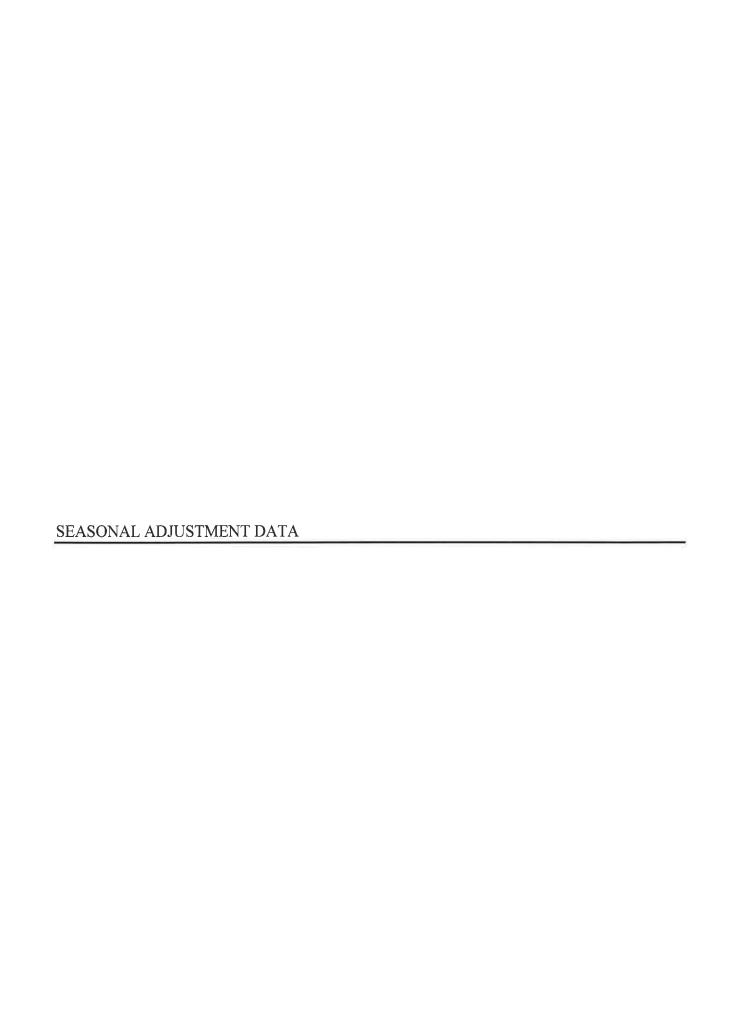
File Name 75860012 Site Code 75860012 Start Date 1/25/2018 Page No 15

N/S Street: Bremen Street E/W Street: Maverick Street City/State: Boston, MA Weather: Clear

Maverick St Out_0 Total 1 Thru Right Left Peak Hour Data Peak Hour Begins at 04:15 PM Total 3 Bremen St Out In 1 2 North Out In Right Bikes Peds υηΤ Τ Right JuO S Total Mavenck St In 0

Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1

04:15 PM +0 mins. 0 +15 mins. 0	14														
+0 mins. 0 +15 mins. 0	2			05:00 PM			Ö	04:00 PM			0	04:00 PM			
+15 mins. 0	0 (0	0	0	-	0	_	0	0	0	0	0	0	0	0
	0 (-	~	0	0	0	0	0	-	0	+	0	0	0	0
+30 mins. 0	0 (0	0	0	0	0	0	0	0	0	0	0	0	0	0
+45 mins. 0	_	0	-	0	-	0	~	0	0	0	0	0	0	0	0
Total Volume 0	1	•	2	0	2	0	2	0	-	0	-	0	0	0	0
% App. Total 0) 20	20		0	100	0		0	100	0		0	0	0	



Massachusetts Highway Department 8087: Monthly Hourly Volume for January 2016

U3		U3	
Seasonal Factor Group:	Daily Factor Group:	Axle Factor Group:	Growth Factor Group:
8087	SUFFOLK	3	LEE BURBANK HIGHWAY
Location ID:	County:	Funcationl Class	Location:

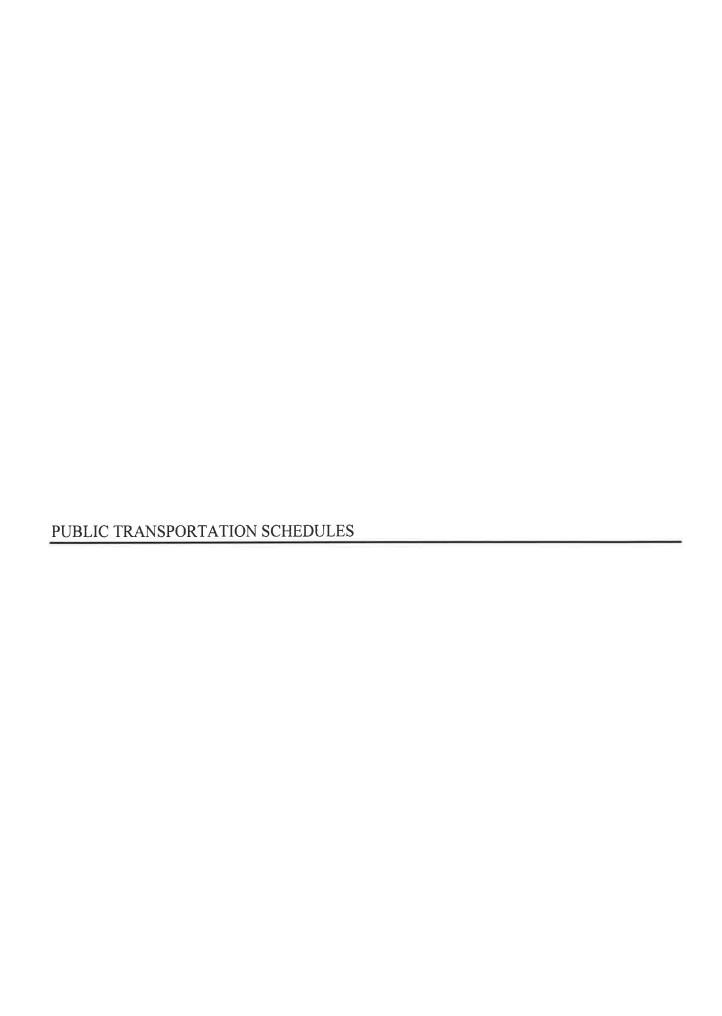
	92	66	46	03	57	00	31	30	32	48		29	63	74	93	08	55	63	98	42	63	05	73	99	41	66	689	20	68	59	122	57
TOTAL	45192	51899	48446	58103	61157	60200	58531	59830	53332	45448		58159	58163	61274	61593	51608	48955	52763	61986	59342	61463	64505	45173	41666	56641	55799	56989	58850	60489	54859	50022	55414.57
23:00	1304	1562	1078	1377	1197	1164	1295	1590	1479	1127		1220	1424	1595	1585	1594	1208	1264	1341	1196	1400	1639	1196	1006	1074	1115	1219	1179	1520	1673	1066	II
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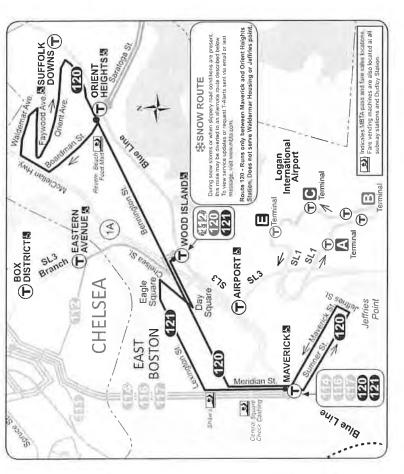
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59367

Yearly Average =



Route 120 Orient Heights - Maverick Station Route 121 Wood Island - Maverick Station



Effective September 2, 2018

120 Orient Heights-Maverick Station via Bennington Street

121 Wood Island-Maverick Station via Lexington Street

Serving

 Waldemar Loop Day Square

Eagle Square

Central Square, East Boston
Jeffries Point

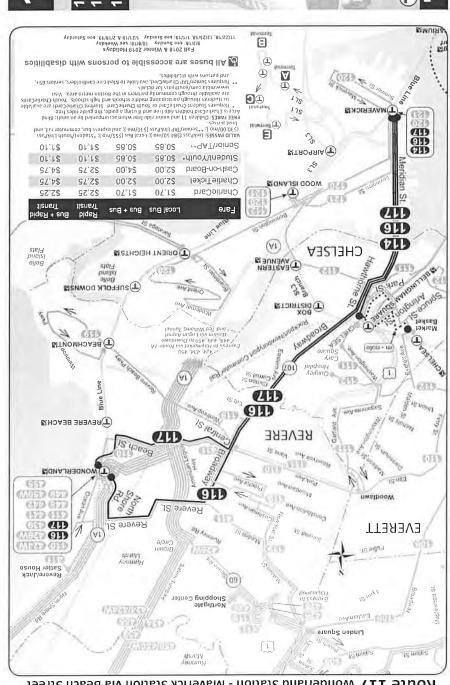
• Blue Line

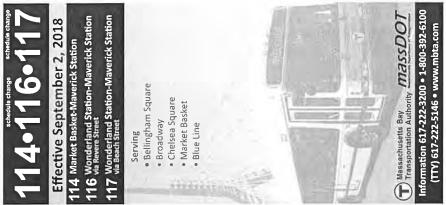
Transportation Authority MASSDOT

Information 617-222-3200 • 1-800-392-6100 (TTY) 617-222-5146 • www.mbta.com

		Arrive Onent Heights	6:48A	8:38	9:36	10:36	2	12:39P	7:39	3:30	4:39	5:39	7:27	8:22	10:17	10:59	12:35A	1:15				abilities		bus + Kapid Transit	\$2.25	\$4.75	\$4.75	\$1.10	\$1.10	ik Pass	plie lie	ind	Charlingards	n.655.		day
	Outbound	Arrive Orient Hts To: Waldemar	6:38A	8:28	9:25	11:28	2	12:28P	2:28	3:28	4:28	5:28	7:17	8:12	10:01	10:49	17:38 12:26A	1:06	ooks Street.		ck Station.	s with disa		Rapid Bus + Transit Tra	\$2.25	\$2.75 \$4		\$1.10 \$1	\$1.10 \$1	udent/Youth Lin	us, communers	d by an adult.	chook years	muliters, senic	ys	8/19: see Salur
lav		Leave Maverick Station	6:25A	8:15	9:10	11:10	-	12:10P	01:1	3:10	4:10	5:10	7:00	555	9:45	10:35	12:13A	w 12:56	Street at Bro	nool variation	ve al Maveri	to persons	ľ		L				\$0.85 \$1.	1525/ma_11*54	Sud stylett	an accompanie s guide, The gu	hools and high	to Medicare can	r 2019 Holida 0/8/18: sec Woo	y 1/21/19 & 2/1
Sunday	-	Arrive Mavenck Station	6:20A	8:10	90:6	10:08	12:08P		2.08	3:07	4:07	5:07	6:54	7:49	9:39	10:30	12:11 12:10A	12:54	b - Leaves from Bennington Street at Brooks Street	6 - Via Waldemar Loop 8 - Does NOT nin during school vacation	w - Wails for last train to arrive at Maverick Station.	A All buses are accessible to persons with disabilities	1	Local Bus + Bus	70 \$1.70	00 \$2.00	00 \$4.00	85 \$0.85		VALID FASSES LawPass (584 50/mo.); Local Bus (555/mo.). "Student/Youth LinkPass	Sautte/Mag; "Jemet/IAP LinkPais (1950/mb.), and system bus, committer (9), a bost printer.	REE FARES. Children 13 and under ride free when accompanied by an adult, filind scars Chaffe and holders ride free and if Luting analog. The guide ride lifee.	to students through participating middle schools and high sthools. Your	week which tem/ four basis for details. The details Served (Ref. Charift Card, see light to Medicare Card and the card for an order 153-, and derivers all the details of the card for the	Fall 2018 & Winter 2019 Holidays 9/2/16: see Sunday 10/8/18: see Weekday	1/19: see Sunda
	punoqui	Arrivo Maverick To: Jeffries Point	6:14A	8:04	8:59	10:01	12:01P		7:0:1	3:00	4:00	2:00	6:48	7:43	9:33	10:24	12:04A	12:48	eaves from	s - Does NOT an during	Waits for las	ses are a		Local	rd \$1.70	:ket \$2.00	oard \$2.00	outh \$0.85	P \$0.85	LinkPass (Sa4 St	HI AV Jahun	ard holders ride	through partici	Hedunes Senso/TAP Churing and persons with disabilities	Fal 9/1/18	718, 12/25/18, 17
120		Leave Onent Heights	6:00A			10.45			,				6:35				13:1		P-P		- M	All bu		Fare	CharlieCard	CharlieTicket	Cash-on-Board	Student/Youth*	Senior/TAP	VALID PASSES	Southerner.	Access Charles	to students	** Fequives S		11/22
D.		Arrive Wood Island	6:24A 6:54			8:53		:		1	:	:					Arriva	П	6:08A	7.46	Until	11:25	Z:01F	12:34P	1:44	2:19	3:29	4:05	4:40	5:50	6:25	7:00	8:40	10:37	12:30A	
	Outbound	Arnve Eagle Square	6:20A	7:20	7:50	8:50	000	3:28P	4:19	4:43	5:08	3.55	6:22		Outbound	Arrive	Onent Hts To:	Valdemar	5:58A	7:36	o Mins.	11:14	06.11	12:23P	1:33	2:08	3:18	3:54	5-04	5:39	6:14	6:49 7:30	8:30	10:28	12:21A	5
day	_	Leave Mavenck Square	6:15A 6:45	7:15	7:45	8:45	000	3:20F	4:10	4:35	5:00	5.50	6:14	20.0			Leave (- 1	5:47A					12:04P	1:14	1:49	2:59	3:35	4:10	5:20	5:55	6:30 7:15	8:15	10:15	12:10A w 12:56	
Weekday	-		6:10A 6:40	7:12	7:40	8:40	0.44	3:41P	4:31	4:56	5:21	6.10	6:34		Saturday	_	Arrive	_	5:44A	7.24	Until	11:31	75U.5	12:38P	1:48	2:20	3:30	4:05	5:15	5:50	6:25	7:12 8:09	9:09	11:09		7
	punoqu	Lv:/Arrive Eagle Square	6:02A 6:32					3.58 8.58	4:23	4:48	5:12	6.02	6:26		hhound		Mavenck To: Jeffries	-	5:37A	7:17	O Mins.	11:24	00:-	12:31P	1:41	2:13	3:23	3:58	4:33 5:08	5:43	6:18	7:05 8:02	9:02	11:02	12:48A	
121		Leave Wood Island	6:00A 6:30	7:00	7:30	8:30			11			1			120		Leave Orient T	-	5:25A		.,	11:05				1:57	3:07	3:42	4:17	5:27	6:02	6:49 7:49				
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Route 114 Market Basket - Maverick Station via Revere Street Route 115 Wonderland Station - Maverick Station via Beach Street Route 117 Wonderland Station - Maverick Station via Beach Street





		Arnve Wonderland Station	2000 2000 2000 2000 2000 2000 2000 200
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Colorer Colo	punoqui	Arnve Bellingham Square	28
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Charles Colores Colo		Arrive Wonderland Station	m 3.04 P
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PRICE PER TRIP Local Bus	Local Bus	Bus + Bus	Rapid Transit	Bus - Rapid Transit
CharlieCard	\$1.70	\$1.50	\$2,25	\$2.25
CharlieTicket	\$2,00	\$2.00	\$2.75	\$4.75***
Cash-on-Board	\$2.00	\$4.00	\$2.75	\$4.75***
Student*	\$0.85	\$0.85	\$1.10	\$1.10
Seniar/TAP**	\$0.85	\$0.85	\$1.10	\$1.10
UNI IMITED TRIP PASSES	PASSES			
1-Day	\$12.00	\$12,00	\$12,00	\$12.00
7-Day	\$21.25	\$21.25	\$21.25	\$21.25
Monthly	\$55.00	\$55.00	\$84.50	\$84.50
Senior/TAP Monthly\$30.00/month for unlimited travel on	:hly\$30.00,	month for u	nlimited 1	travel on
	Local Bus	Local Bus and Papid Transit	ansit	

VALID PASSES: LinkPass (\$84,50/mo.); Student LinkPass" (\$30/mo.); Senior/TAP LinkPass" (\$30/mo.); and express bus, commuter rail, and boat

Blind Access CharlieCard holders ride free: if using a guide, the guide rides free FREE FARES: Children 11 and under ride free when accompanied by an adult;

Available to students through participating middle schools and

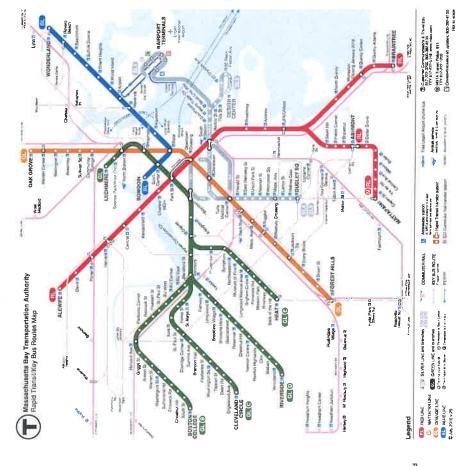
- ** Available to Medicare cardholders, seniors 65+, and persons high schools.
 - with disabilities. *** For Silver Line SL4 or SL5 pay \$2.75. Also see "transfers."

TRANSFERS

If paying with a CharlieTicket or CharlieCard, discounted transfers that are available as automatic. Light such be some before to read throughout your trip. If paying with cash onboard a vehicle, free transfers are only allowed between rapid transit lines and inside paid platform areas at gated stations.

SCHEDULES

Schedules are available at the following stations: Park Street, Airport, Malden, Harvard, Haymarket (Green Line Level), Back Bay, Downtown Crossing (Orange Line Level), and Quincy Center, or ask a Customer Sorvice Agent. Schedules are also available at the State Transportation Building (10 Park Plaza), 45 High St, and online at miba com.



Rapid Transit

Winter December 31, 2017 - March 31, 2018







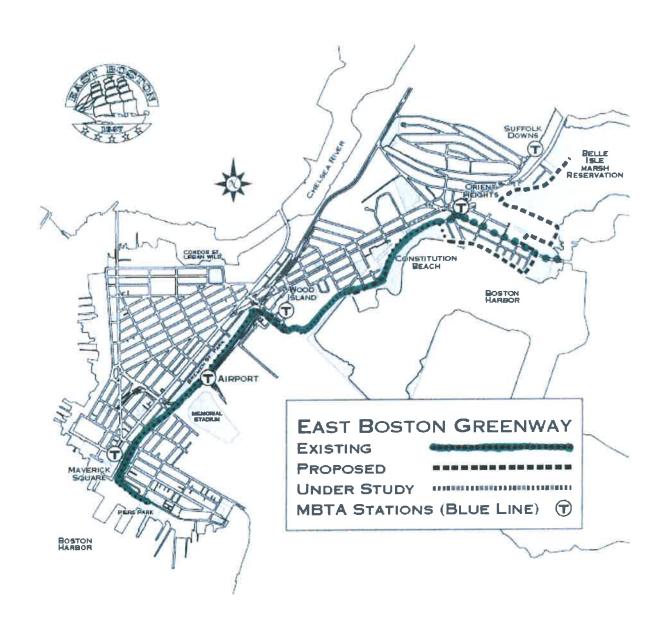


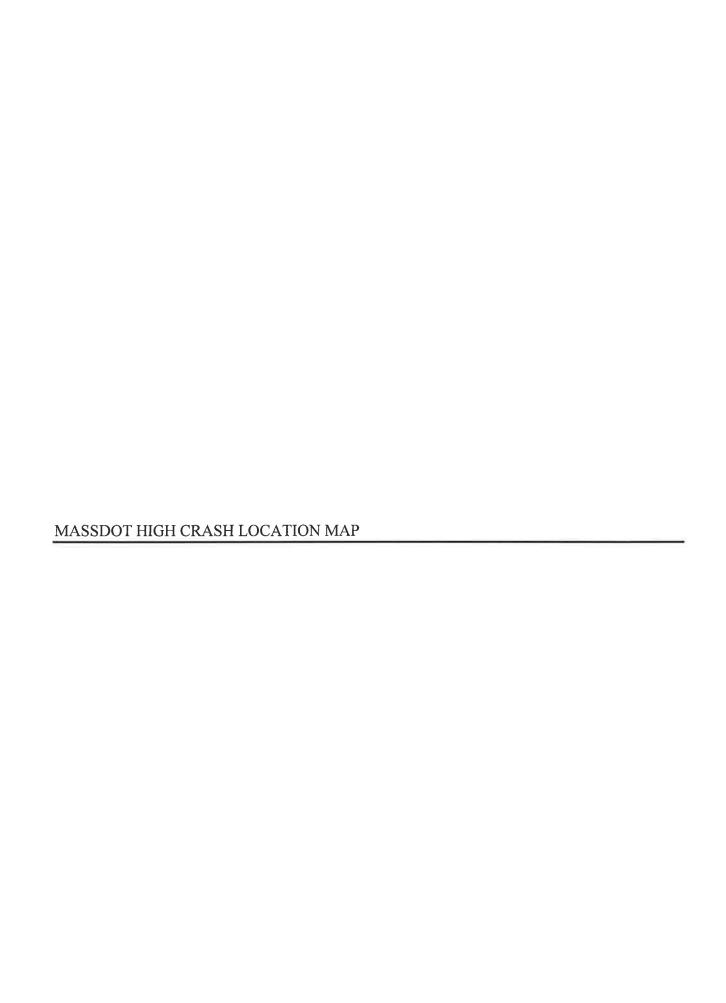


Information 617-222-3200 • 1-800-392-6100 Transportation Authority (TTY) 617-222-5146 • www.mbta.com

First Trip 5:24AM 5:15AM 5:15AM 5:15AM 5:15AM 5:15AM 5:15AM 6:05AM 6:05A	PAM	Midday P	#		Late Last Night Trip 12 min 12:15AM 12 min 12:15AM 12 min w 12:30AM 12 min w 12:30AM 12 min 12:28AM 9 min 12:28AM 9 min 12:28AM 9 min 12:30AM 10 min w 12:30AM 10 min w 12:30AM 11 min w 12:30AM 11 min w 12:30AM 9 min 12:30AM 12:30AM 11 min w 12:30AM 12:30AM 12:30AM 12:30AM 11 min w 12:30AM 12:30AM 11 min w 12:30AM 12:30AM 11 min w 12:30AM	Last Trip Trip Trip Trip Trip Trip Trip Trip	A9 44 44 6 6 6 6 6 6 7 7 7 7 7 7 7 7 7 7 7					 First AM Trip Peak 6:08AM 15 min 6:00AM 15 min 6:00AM 15 min 6:03AM 26 min 5:58AM 13 min 6:12AM 13 min 6:04AM 13 min 6:05AM 13 min 6:05AM 13 min 6:05AM 12 min 6:05AM 12 min 6:05AM 12 min 6:05AM 13 min 6:05AM 12 min	2 5 5 5 5 5 5 7 5 8 8 8 9 9 1 T T 1 8 8 8 9 9 9 9 1 T T T T T T T T T T T T T T T	Sun selection se	00	Late Last Night Trip 16 min 12:15AM 16 min w 12:22AM 16 min w 12:30AM 26 min w 12:33AM 13 min 12:33AM 13 min 12:33AM 11 min w 12:30AM 11 min w 12:30AM 12 min w 12:46AM 12 min 12:46AM 11 min w 12:46AM 12 min 12:46AM 12 min 12:46AM 13 min 12:30AM 14 min 12:30AM 16 min 12:30AM 17 min 12:30AM 17 min 12:30AM 18 min f 12:45AM 18 min f 12:30AM 19 min 12:30AM		Schedule Periods (approximate): AM Rush Hour: 6:30 AM - 9:00 AM Midday: 9:00 AM - 5:30 PM PM Rush Hour: 3:30 PM - 6:30 PM Evening: 6:30 PM - 6:30 PM Evening: 6:30 PM - CLOSE Red Line Note: -Braintree Line: Construction on Wollaston Station will begin in January 2018. Wollaston Station will begin in January 2018. Wollaston Station will begin in January 2018. Wollaston Station will be closed during construction and shuttle buses will operate between Wollaston and North Quincy Stations. Please visit information. Mattapan Note: Saturday and Sunday before 10:00 AM and after 8:00 PM trips depart every 26 minutes and the rest of the day every 12 minutes. Also, see Mattapan Line Schedule Card. Green Line Notes: Saturday and sunday before 10:00 AM and after 8:00 PM trips depart every 26 minutes. Also, see Mattapan Line Schedule Card. Green Line Notes: 3- The first two C Line AM inbound trips run through to Lechmere Station on weekdays. 2- The first B Line and second C Line AM inbound trips run through to Lechmere Station on weekends. 3- The 12:32AM trip from Heath St. runs in service to Lechmere with no guaranteed connections. f - After exiting Ted Williams Tunnel bus will only service World Trade Center and South Station stops.
5:35AM 5:20AM 5:35AM 5:15AM 5:32AM	5 min 12 min 12 min 8 min 8 min		14 min 1.14 min 1.18 min 8 min	12 min 1 12 min 1 7 min 1 7 min 1	12:53AM 19 min 12:20AM 19 min 12:39AM 17 min 12:53AM 17 min w 1:07AM			15 min 15 t 15 min 15 t 10 min 10 t	15 min 15 15 min 15 10 min 11	15 mln 20 15 mln 20 11 min 11 11 min 11	를 발 내		15 min 15 15 min 15 10 min 8 10 min 8	15 min 15 15 min 15 8 min 9 8 min 9	202 66	20 min 12: 20 min 12: 9 min 12	1:01AM P 12:20AM a 12:40AM a 12:25AM w 12:47AM	primarily in the Downtown area, for connecting service. Departure times approximate. Whinter 2018 Holidays January 1: see Sunday January 1: see Sunday Silver Line see Weekday







 $\sigma \, {\mathbb J}_{\lambda}^{*}$

I nan nan



General Background Traffic Growth - Daily Traffic Volumes

Proposed Residential Development, East Boston, MA

Average	Annual	0.92%
	2016	29.367
	2015	60,689 58.092 59.367
	2014	689'09
	2013	53.534
	2012	56,677 5
	2011	56,942
	2010	55.853
	2009	54,448
	2008	52,319
	2002	56.374
	2006	54,548
	LOCATION	North of Boston/Revere City Line Sta. 8087
	ROUTE/STREET	Route 1A
	CITY/TOWN	Revere



W.

Note: Imbalances exist due to numerous curb cuts and side streets that are not shown.

Not To Scale



Figure A-1

135 Bremen Street
Residential Development
Weekday Morning
Peak Hour Traffic Volumes

Note: Imbala

Not To Scale

Note: Imbalances exist due to numerous curb cuts and side streets that are not shown.

Vanasse & Associates, Inc.
Transportation Engineers & Planners

Figure A-2

135 Bremen Street
Residential Development
Weekday Evening
Peak Hour Traffic Volumes

X

Note: Imbalances exist due to numerous curb cuts and side streets that are not shown.

Not To Scale



Figure A-3

31 Orleans Street
Residential Development
Weekday Morning
Peak Hour Traffic Volumes

W.

Note: Imbalances exist due to numerous curb cuts and side streets that are not shown.

Not To Scale



Figure A-4

31 Orleans Street
Residential Development
Weekday Evening
Peak Hour Traffic Volumes

Not To Scale

Vanasse & Associates, Inc.
Transportation Engineers & Planners

Figure A-5

175 Orleans Street Boston Loftel Weekday Morning Peak Hour Traffic Volumes

W.

Note: Imbalances exist due to numerous curb cuts and side streets that are not shown.

Not To Scale



Figure A-6

175 Orleans Street
Boston Loftel
Weekday Evening
Peak Hour Traffic Volumes

Note: Imbalances exist due to numerous curb cuts and side streets that are not shown.

Van

Not To Scale

Vanasse & Associates, Inc.
Transportation Engineers & Planners

Figure A-7

202 Maverick Street
Residential Development
Weekday Morning
Peak Hour Traffic Volumes



Vanasse & Associates, Inc.
Transportation Engineers & Planners

Figure A-8

202 Maverick Street
Residential Development
Weekday Evening
Peak Hour Traffic Volumes



Vanasse & Associates, Inc.
Transportation Engineers & Planners

Figure A-9

70 Bremen Street Mixed-Use Development Weekday Morning Peak Hour Traffic Volumes

Note: Imbalances exist due to numerous curb cuts and side streets that are not shown.

Not To Scale

Figure A-10

70 Bremen Street
Mixed-Use Development
Weekday Evening
Peak Hour Traffic Volumes



Vanasse & Associates, Inc.
Transportation Engineers & Planners

Figure A-11

125 Sumner Street
Residential Development
Weekday Morning
Peak Hour Traffic Volumes



Vanasse & Associates, Inc.
Transportation Engineers & Planners

Figure A-12

125 Sumner Street
Residential Development
Weekday Evening
Peak Hour Traffic Volumes

Note: Imbalances exist due to numerous curb cuts and side streets that are not shown.

Not To Scale

Vanasse & Associates, Inc.
Transportation Engineers & Planners

Figure A-13

114 Orleans Street
Residential Development
Weekday Morning
Peak Hour Traffic Volumes

Note: Imbalances exist due to numerous curb cuts and side streets that are not shown.

Not To Scale

Vanasse & Associates, Inc.
Transportation Engineers & Planners

Figure A-14

114 Orleans Street
Residential Development
Weekday Evening
Peak Hour Traffic Volumes



Vanasse & Associates, Inc.
Transportation Engineers & Planners

Figure A-15

99-111 Sumner Street
Mixed-Use Development
Weekday Morning
Peak Hour Traffic Volumes

Note: Imbalances exist due to numerous curb cuts and side streets that are not shown.

Vanasse & Associates, Inc.
Transportation Engineers & Planners

Figure A-16

99-111 Sumner Street
Mixed-Use Development
Weekday Evening
Peak Hour Traffic Volumes

Not To Scale

Note: Imbalances exist due to numerous curb cuts and side streets that are not shown.

Vanasse & Associates, Inc.
Transportation Engineers & Planners

Figure A-17

10-16 Everett Street
Residential Development
Weekday Morning
Peak Hour Traffic Volumes

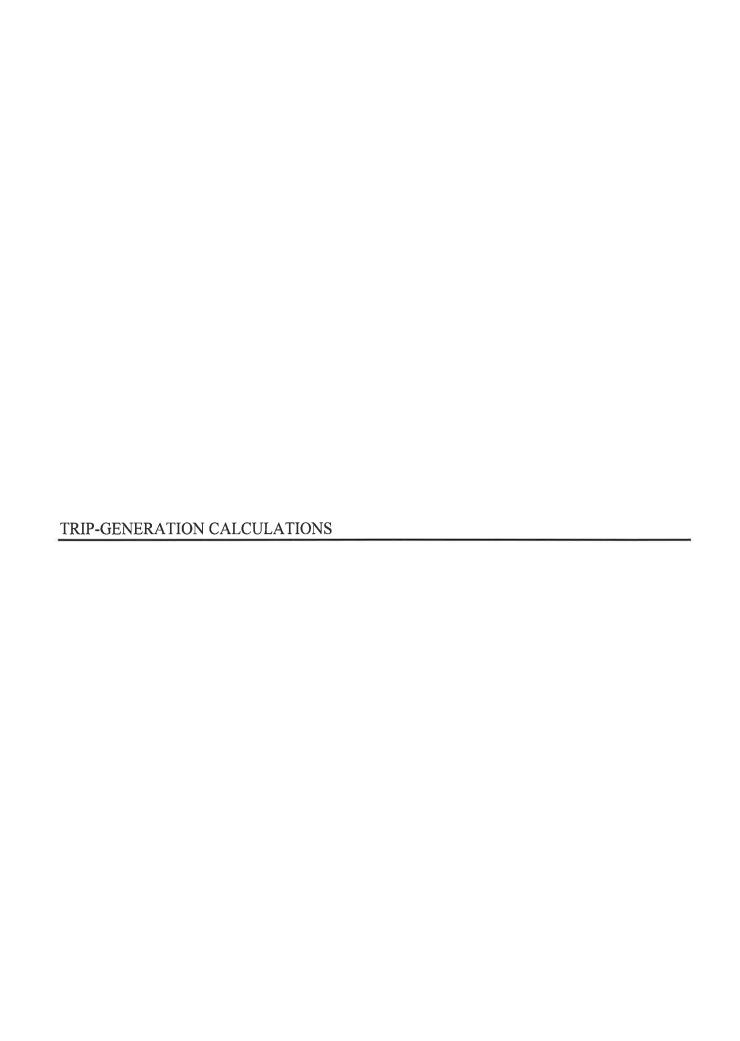
Not To Scale

Vanasse & Associates, Inc.
Transportation Engineers & Planners

Figure A-18

10-16 Everett Street Residential Development Weekday Evening Peak Hour Traffic Volumes

Not To Scale



Multifamily Housing (Mid-Rise)

(221)

Vehicle Trip Ends vs: Dwelling Units

On a: Weekday

Setting/Location: General Urban/Suburban

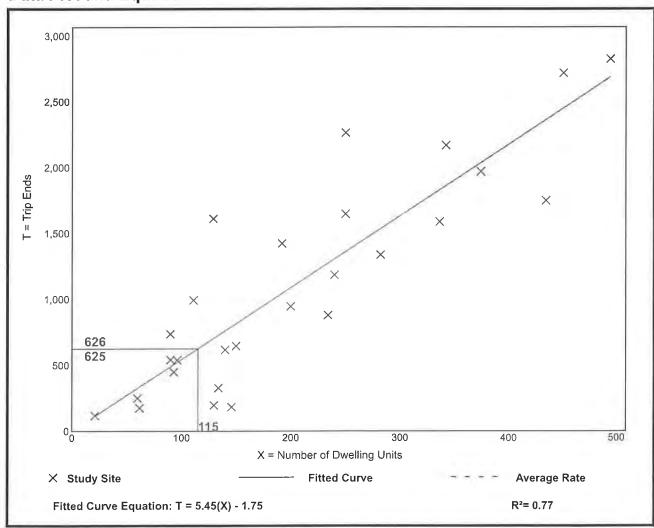
Number of Studies: 27 Avg. Num. of Dwelling Units: 205

Directional Distribution: 50% entering, 50% exiting

Vehicle Trip Generation per Dwelling Unit

Average Rate	Range of Rates	Standard Deviation
5.44	1.27 - 12.50	2.03

Data Plot and Equation



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Multifamily Housing (Mid-Rise)

(221)

Vehicle Trip Ends vs: Dwelling Units

On a: Weekday,

Peak Hour of Adjacent Street Traffic,

One Hour Between 7 and 9 a.m.

Setting/Location: General Urban/Suburban

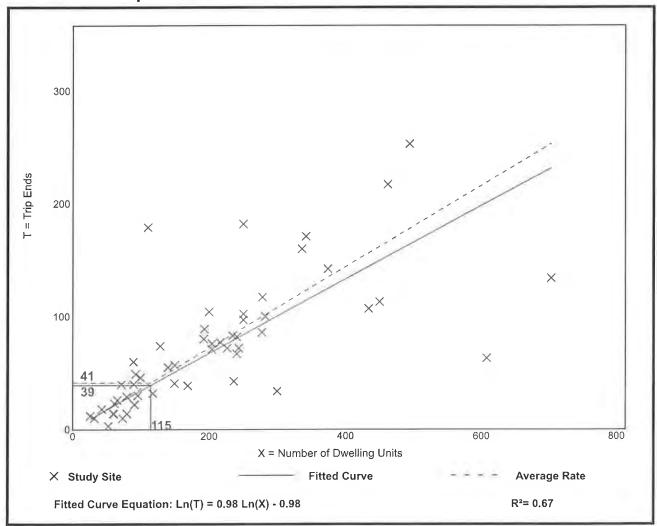
Number of Studies: 53

Avg. Num. of Dwelling Units: 207

Directional Distribution: 26% entering, 74% exiting

Vehicle Trip Generation per Dwelling Unit

Data Plot and Equation



Trip Generation Manual, 10th Edition • Institute of Transportation Engineers

Multifamily Housing (Mid-Rise)

(221)

Vehicle Trip Ends vs: Dwelling Units

On a: Weekday,

Peak Hour of Adjacent Street Traffic,

One Hour Between 4 and 6 p.m.

Setting/Location: General Urban/Suburban

Number of Studies: 60

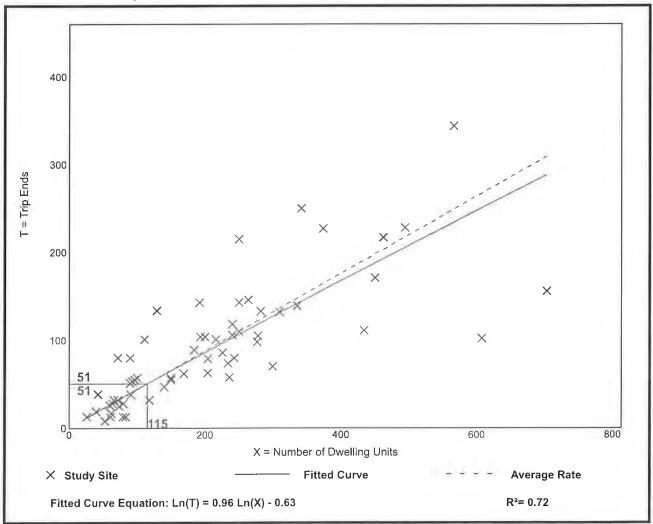
Avg. Num. of Dwelling Units: 208

Directional Distribution: 61% entering, 39% exiting

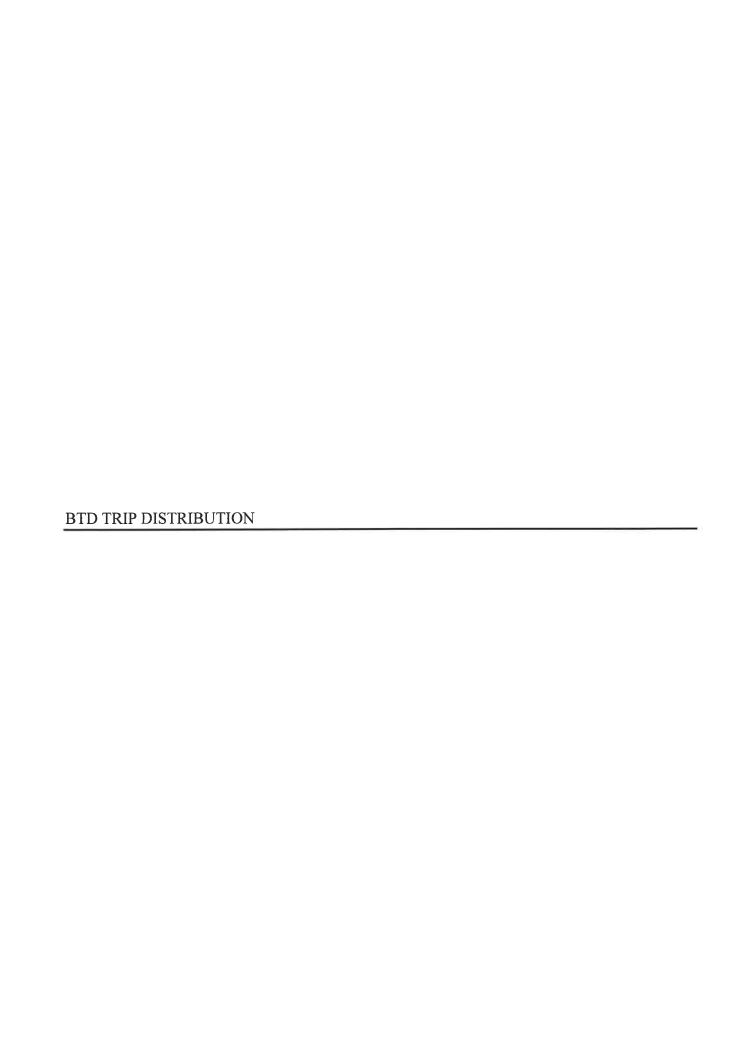
Vehicle Trip Generation per Dwelling Unit

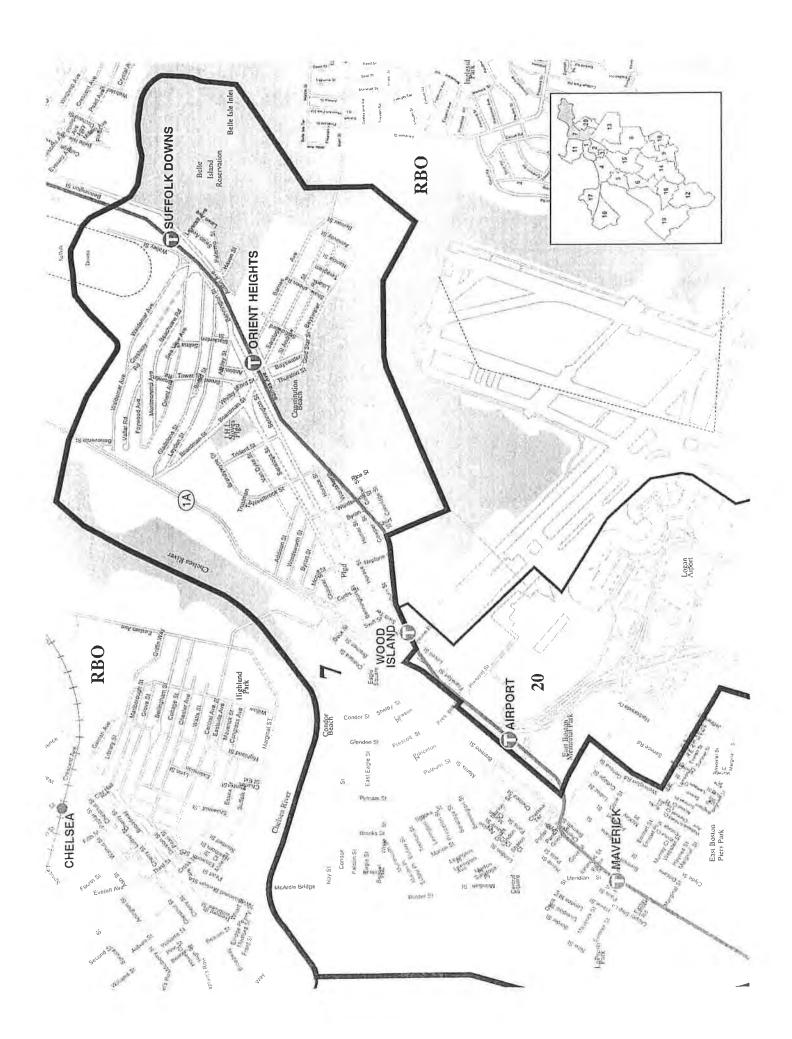
Average Rate	Range of Rates	Standard Deviation
0.44	0.15 - 1.11	0.19

Data Plot and Equation



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Distribution and Mode Share of Daily Trips by Transportation Zone

Daily Trips

Note that the same number of trips are assumed to begin or end in Zone 7

To/From		ode Shares		Geogra	phical Dis	tribution o	f Trips
ZONE		Transit	Walk	Total	Auto	Transit	Walk
1	48.2	51.8	0.0	1.3	1.2	5.0	0.0
2	21.6	78.4	0.0	4.3	1.8	25.8	0.0
3	27.5	72.5	0.0	0.8	0.3	4.7	0.0
4	39.0	61.0	0.0	2.2	1.7	10.2	0.0
5	44.8	55.2	0.0	0.7	0.6	2.8	0.0
6	63.0	37.0	0.0	0.3	0.3	0.7	0.0
7	25.9	4.4	69.7	50.6	25.3	17.1	100.0
8	80.1	19.9	0.0	0.5	0.8	8.0	0.0
9	85.3	14.7	0.0	0.2	0.4	0.3	0.0
10	76.4	23.6	0.0	0.4	0.6	0.7	0.0
11	78.0	22.0	0.0	0.5	0.7	8.0	0.0
12	96.3	3.7	0.0	0.1	0.1	0.0	0.0
. 13	83.2	16.8	0.0	0.5	0.8	0.6	0.0
14	62.8	37.2	0.0	0.3	0.3	0.7	0.0
15	62.5	37.5	0.0	0.9	1.1	2.5	0.0
16	60.5	39.5	0.0	0.1	0.1	0.2	0.0
17	81.4	18.6	0.0	0.2	0.4	0.3	0.0
18	85.0	15.0	0.0	0.1	0.2	0.2	0.0
19	82.1	17.9	0.0	0.1	0.2	0.2	0.0
20	97.9	2.1	0.0	4.2	8.1	0.7	0.0
RBO	88.8	11.2	0.0	17.4	29.9	15.0	0.0
RGR	81.0	19.0	0.0	3.8	6.0	5.6	0.0
RCD	74.4	25.6	0.0	1.1	1.6	2.1	0.0
RMR	90.4	9.6	0.0	0.4	0.8	0.3	0.0
BNE	96.3	3.7	0.0	1.9	3.5	0.5	0.0
BNO	97.9	2.1	0.0	0.9	1.8	0.2	0.0
BNW	89.7	10.3	0.0	0.8	1.3	0.6	0.0
CN	98.8	1.2	0.0	2.6	4.9	0.2	0.0
CW	97.1	2.9	0.0	1.3	2.5	0.3	0.0
CSW	90.6	9.4	0.0	0.8	1.4	0.6	0.0
CSE	93.7	6.3	0.0	0.7	1.3	0.3	0.0
TOTAL	51.6	13.1	35.3	100.0	100.0	100.0	100.0

Mode Share by Purpose* and Time of Day

Trips Beginning in Zone 7

Daily avg. mode shares	All Purposes	Home	Work	Other
Auto	52%	54%	74%	42%
Transit	13%	17%	21%	6%
Walk	35%	29%	5%	52%
AM peak mode shares			A.	
Auto	44%	45%	63%	35%
Transit	24%	25%	32%	9%
Walk	32%	30%	5%	56%
Rest of day mode shares				
Auto	54%	60%	75%	42%
Transit	10%	12%	20%	6%
Walk	36%	28%	5%	52%
PM peak mode shares			•	
Auto	50%	51%	75%	37%
Transit	10%	15%	19%	5%
Walk	40%	34%	6%	58%

Trips Ending in Zone 7

Daily avg. mode shares Auto	All Purposes 52%	Home 54%	Work 74%	Other 42%
Transit	13%	17%	21%	6%
Walk	35%	29%	5%	52%
AM peak mode shares				
Auto	50%	51%	75%	37%
Transit	10%	15%	19%	5%
Walk	40%	34%	6%	58%
Rest of day mode shares				
Auto	52%	54%	74%	43%
Transit	14%	17%	22%	6%
Walk	34%	29%	4%	51%
PM peak mode shares				
Auto	44%	45%	63%	35%
Transit	24%	25%	32%	9%
Walk	32%	30%	5%	56%

^{*}Purpose refers to the activity that occurs in Zone 7.

CAPACITY ANALYSIS WORKSHEETS

Porter Street at Bremen Street

Porter Street at Orleans Street

Porter Street at Frankfort Street

Porter Street at Cottage Street

Gove Street at Berman Street

Gove Street at Orleans Street

Gove Street at Frankfort Street

Gove Street at Cottage Street

Maverick Street at Berman Street

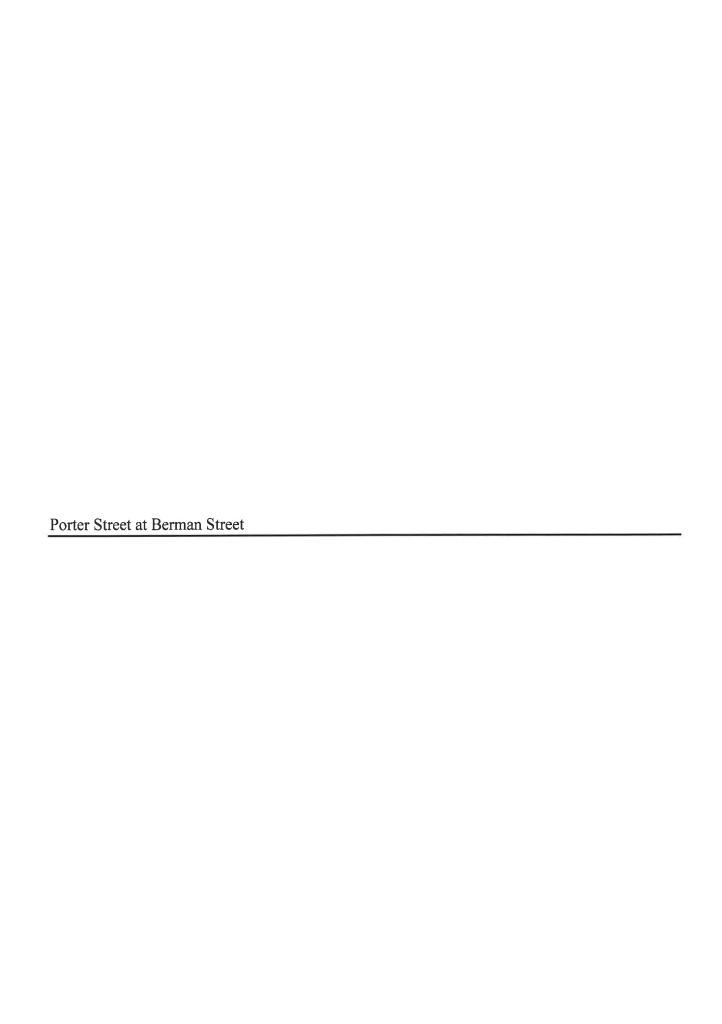
Maverick Street at Orleans Street

Maverick Street at Frankfort Street

Maverick Street at Cottage Street

Frankfort Street at the Project Site Driveway

Lubec Street at the Project Site Driveway



	*	\rightarrow	*	1	4	4	4	†	-	1	+	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Volume (vph)	11	135	35	4	135	131	64	99	32	281	176	, 0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	16	12	12	16	12	12	13	12	12	12	12
Satd. Flow (prot)	0	1854	0	0	1782	0	0	1683	0	0	1626	0
Flt Permitted		0.997			0.999			0.984			0.970	
Satd. Flow (perm)	0	1854	0	0	1782	0	0	1683	0	0	1626	0
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		397			764			716			461	
Travel Time (s)		9.0			17.4			16.3			10.5	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	0%	2%	0%	0%	0%	3%	0%	2%	0%	2%	2%	0%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	201	0	0	300	0	0	217	0	0	508	C
Sign Control		Stop			Stop			Stop			Stop	

Intersection Summary

Area Type:

CBD

Control Type: Unsignalized

Intersection Capacity Utilization 67.6%

ICU Level of Service C

Movement EBU EBL EBT EBR WBU WBL WBT WBR NBU NBL NBT NBU NBL NBT NBU	Intersection												
Value Valu	Intersection Delay, s/veh Intersection LOS												
Vol. veh/h	Movement	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU			NBR
Peak Hour Factor	Vol, veh/h	0	11	135	35	0	4	135					32
Mount Flow 0 12 150 39 0 4 150 146 0 71 110	Peak Hour Factor	0.92	0.90	0.90	0.90	0.92	0.90						0.90
Mymt Flow 0 12 150 39 0 4 150 146 0 71 110 Number of Lanes 0 0 1 0 0 0 1 0 0 0 1 110	Heavy Vehicles, %	2	0	2		2	0						0
Approach	Mvmt Flow	0		150									36
Spherical Approach WB	Number of Lanes	0	0	1	0	0	0	1	0	0	0	1	0
Deposing Approach	Approach		EB				WB		433	1168	NB	The same	
Deposing Lanes			WB				EB						
SB			1										
Conflicting Approach Right NB SB WB Conflicting Approach Right 1 1 1 HCM Control Delay 14.3 16.9 14.3 HCM LOS B C B AND Left, % 33% 6% 1% 61% AND Left, % 51% 75% 50% 39% AND Right, % 16% 19% 49% 0% Sign Control Stop Stop Stop Stop Stop Sign Control Stop Stop Stop Stop Stop Stop Stop Stop	Conflicting Approach Left		SB										
Conflicting Lanes Right	Conflicting Lanes Left												
Acm Control Delay	Conflicting Approach Right		NB				SB						
Acm Acm	Conflicting Lanes Right						•						
Age	HCM Control Delay		14.3										
Vol Left, % 33% 6% 1% 61% Vol Thru, % 51% 75% 50% 39% Vol Right, % 16% 19% 49% 0% Sign Control Stop Stop Stop Stop Traffic Vol by Lane 195 181 270 457 LT Vol 64 11 4 281 Through Vol 99 135 135 176 RT Vol 32 35 131 0 Lane Flow Rate 217 201 300 508 Geometry Grp 1 1 1 Degree of Util (X) 0.404 0.386 0.539 0.88 Departure Headway (Hd) 6.708 6.906 6.464 6.241 Convergence, Y/N Yes Yes Yes Yes Cap 533 517 555 579 Service Time 4.799 4.999 4.544 4.31 HCM Lane V/C Ratio 0.407 0.389 0.541 0.877 HCM Lane LOS B	HCM LOS		В				С				В		
Vol Left, % 33% 6% 1% 61% Vol Thru, % 51% 75% 50% 39% Vol Right, % 16% 19% 49% 0% Sign Control Stop Stop Stop Stop Traffic Vol by Lane 195 181 270 457 LT Vol 64 11 4 281 Through Vol 99 135 135 176 RT Vol 32 35 131 0 Lane Flow Rate 217 201 300 508 Geometry Grp 1 1 1 Degree of Util (X) 0.404 0.386 0.539 0.88 Departure Headway (Hd) 6.708 6.906 6.464 6.241 Convergence, Y/N Yes Yes Yes Yes Cap 533 517 555 579 Service Time 4.799 4.999 4.544 4.31 HCM Lane V/C Ratio 0.407 0.389 0.541 0.877 HCM Lane LOS B													
Vol Thru, % 51% 75% 50% 39% Vol Right, % 16% 19% 49% 0% Sign Control Stop Stop Stop Fraffic Vol by Lane 195 181 270 457 LT Vol 64 11 4 281 Through Vol 99 135 135 176 RT Vol 32 35 131 0 Lane Flow Rate 217 201 300 508 Geometry Grp 1 1 1 1 Degree of Util (X) 0.404 0.386 0.539 0.88 Departure Headway (Hd) 6.708 6.906 6.464 6.241 Convergence, Y/N Yes Yes Yes Yes Cap 533 517 555 579 Service Time 4.799 4.999 4.544 4.31 HCM Lane V/C Ratio 0.407 0.389 0.541 0.877 HCM Lane LOS	Lane												
Vol Right, % 16% 19% 49% 0% Sign Control Stop Stop Stop Stop Traffic Vol by Lane 195 181 270 457 LT Vol 64 11 4 281 Through Vol 99 135 135 176 RT Vol 32 35 131 0 Lane Flow Rate 217 201 300 508 Geometry Grp 1 1 1 1 Degree of Util (X) 0.404 0.386 0.539 0.88 Departure Headway (Hd) 6.708 6.906 6.464 6.241 Convergence, Y/N Yes Yes Yes Yes Cap 533 517 555 579 Service Time 4.799 4.999 4.544 4.31 HCM Lane V/C Ratio 0.407 0.389 0.541 0.877 HCM Control Delay 14.3 14.3 16.9 39.1 HCM Lane LOS B B C E	Vol Left, %												
Sign Control Stop Stop Stop Stop Stop Traffic Vol by Lane 195 181 270 457 LT Vol 64 11 4 281 Through Vol 99 135 135 176 RT Vol 32 35 131 0 Lane Flow Rate 217 201 300 508 Geometry Grp 1 1 1 1 Degree of Util (X) 0.404 0.386 0.539 0.88 Departure Headway (Hd) 6.708 6.906 6.464 6.241 Convergence, Y/N Yes Yes Yes Cap 533 517 555 579 Service Time 4.799 4.999 4.544 4.31 HCM Lane V/C Ratio 0.407 0.389 0.541 0.877 HCM Control Delay 14.3 14.3 16.9 39.1 HCM Lane LOS B B C E	Vol Thru, %												
Traffic Vol by Lane 195 181 270 457 LT Vol 64 11 4 281 Through Vol 99 135 135 176 RT Vol 32 35 131 0 Lane Flow Rate 217 201 300 508 Geometry Grp 1 1 1 1 Degree of Util (X) 0.404 0.386 0.539 0.88 Departure Headway (Hd) 6.708 6.906 6.464 6.241 Convergence, Y/N Yes Yes Yes Yes Cap 533 517 555 579 Service Time 4.799 4.999 4.544 4.31 HCM Lane V/C Ratio 0.407 0.389 0.541 0.877 HCM Control Delay 14.3 14.3 16.9 39.1 HCM Lane LOS B B C E	Vol Right, %												
Trough Vol 99 135 135 176 RT Vol 32 35 131 0 Lane Flow Rate 217 201 300 508 Geometry Grp 1 1 1 1 Degree of Util (X) 0.404 0.386 0.539 0.88 Departure Headway (Hd) 6.708 6.906 6.464 6.241 Convergence, Y/N Yes Yes Yes Yes Cap 533 517 555 579 Service Time 4.799 4.999 4.544 4.31 HCM Lane V/C Ratio 0.407 0.389 0.541 0.877 HCM Control Delay 14.3 14.3 16.9 39.1 HCM Lane LOS B B C E	Sign Control												
Through Vol 99 135 135 176 RT Vol 32 35 131 0 Lane Flow Rate 217 201 300 508 Geometry Grp 1 1 1 1 Degree of Util (X) 0.404 0.386 0.539 0.88 Departure Headway (Hd) 6.708 6.906 6.464 6.241 Convergence, Y/N Yes Yes Yes Cap 533 517 555 579 Service Time 4.799 4.999 4.544 4.31 HCM Lane V/C Ratio 0.407 0.389 0.541 0.877 HCM Control Delay 14.3 14.3 16.9 39.1 HCM Lane LOS B B C E	Traffic Vol by Lane												
RT Vol 32 35 131 0 Lane Flow Rate 217 201 300 508 Geometry Grp 1 1 1 1 Degree of Util (X) 0.404 0.386 0.539 0.88 Departure Headway (Hd) 6.708 6.906 6.464 6.241 Convergence, Y/N Yes Yes Yes Yes Cap 533 517 555 579 Service Time 4.799 4.999 4.544 4.31 HCM Lane V/C Ratio 0.407 0.389 0.541 0.877 HCM Control Delay 14.3 14.3 16.9 39.1 HCM Lane LOS B B C E	LT Vol												
Lane Flow Rate 217 201 300 508 Geometry Grp 1 1 1 1 Degree of Util (X) 0.404 0.386 0.539 0.88 Departure Headway (Hd) 6.708 6.906 6.464 6.241 Convergence, Y/N Yes Yes Yes Cap 533 517 555 579 Service Time 4.799 4.999 4.544 4.31 HCM Lane V/C Ratio 0.407 0.389 0.541 0.877 HCM Control Delay 14.3 14.3 16.9 39.1 HCM Lane LOS B B C E	Through Vol												
Geometry Grp 1 1 1 1 1 Degree of Util (X) 0.404 0.386 0.539 0.88 Departure Headway (Hd) 6.708 6.906 6.464 6.241 Convergence, Y/N Yes Yes Yes Yes Cap 533 517 555 579 Service Time 4.799 4.999 4.544 4.31 HCM Lane V/C Ratio 0.407 0.389 0.541 0.877 HCM Control Delay 14.3 14.3 16.9 39.1 HCM Lane LOS B B C E	RT Vol												
Degree of Util (X) 0.404 0.386 0.539 0.88 Departure Headway (Hd) 6.708 6.906 6.464 6.241 Convergence, Y/N Yes Yes Yes Yes Cap 533 517 555 579 Service Time 4.799 4.999 4.544 4.31 HCM Lane V/C Ratio 0.407 0.389 0.541 0.877 HCM Control Delay 14.3 14.3 16.9 39.1 HCM Lane LOS B B C E	Lane Flow Rate												
Departure Headway (Hd) 6.708 6.906 6.464 6.241 Convergence, Y/N Yes Yes Yes Yes Cap 533 517 555 579 Service Time 4.799 4.999 4.544 4.31 HCM Lane V/C Ratio 0.407 0.389 0.541 0.877 HCM Control Delay 14.3 14.3 16.9 39.1 HCM Lane LOS B B C E			- 1	1		-							
Convergence, Y/N Yes Yes Yes Yes Cap 533 517 555 579 Service Time 4.799 4.999 4.544 4.31 HCM Lane V/C Ratio 0.407 0.389 0.541 0.877 HCM Control Delay 14.3 14.3 16.9 39.1 HCM Lane LOS B B C E	Geometry Grp					0.88							
Cap 533 517 555 579 Service Time 4.799 4.999 4.544 4.31 HCM Lane V/C Ratio 0.407 0.389 0.541 0.877 HCM Control Delay 14.3 14.3 16.9 39.1 HCM Lane LOS B B C E	Geometry Grp Degree of Util (X)		0.404										
Service Time 4.799 4.999 4.544 4.31 HCM Lane V/C Ratio 0.407 0.389 0.541 0.877 HCM Control Delay 14.3 14.3 16.9 39.1 HCM Lane LOS B B C E	Geometry Grp Degree of Util (X) Departure Headway (Hd)		0.404 6.708	6.906	6.464	6.241							
HCM Lane V/C Ratio 0.407 0.389 0.541 0.877 HCM Control Delay 14.3 14.3 16.9 39.1 HCM Lane LOS B B C E	Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N		0.404 6.708 Yes	6.906 Yes	6.464 Yes	6.241 Yes							
HCM Control Delay 14.3 14.3 16.9 39.1 HCM Lane LOS B B C E	Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap		0.404 6.708 Yes 533	6.906 Yes 517	6.464 Yes 555	6.241 Yes 579							
HCM Lane LOS B B C E	Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time		0.404 6.708 Yes 533 4.799	6.906 Yes 517 4.999	6.464 Yes 555 4.544	6.241 Yes 579 4.31							
TOM Edito 200	Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap		0.404 6.708 Yes 533 4.799 0.407	6.906 Yes 517 4.999 0.389	6.464 Yes 555 4.544 0.541	6.241 Yes 579 4.31 0.877							
	Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time HCM Lane V/C Ratio HCM Control Delay		0.404 6.708 Yes 533 4.799 0.407 14.3	6.906 Yes 517 4.999 0.389 14.3	6.464 Yes 555 4.544 0.541 16.9	6.241 Yes 579 4.31 0.877 39.1							

1.9 1.8 3.2 10.1

HCM 95th-tile Q

Intersection Delay, s/veh												
Intersection LOS												
Movement	SBU	SBL	SBT	SBR	W. W.	Mark.		idi s i	101	i lieu	1200	Pri-
Vol, veh/h	0	281	176	0								
Peak Hour Factor	0,92	0.90	0.90	0.90								
Heavy Vehicles, %	2	2	2	0								
Mvmt Flow	0	312	196	0								
Number of Lanes	0	0	1	0								
Approach	100	SB	STEAL!	10 (11)	500		10.1100		778	يرزير	16 LEU **	359
Opposing Approach		NB										
Opposing Lanes		1										
Conflicting Approach Left		WB										
Conflicting Lanes Left		1_										
Conflicting Approach Right		EB										
Conflicting Lanes Right		1										
HCM Control Delay		39.1										
HCM LOS		Е										

	٠	→	*	1	←	*	4	1	1	-	↓	1
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Volume (vph)	76	156	20	4	79	118	46	196	24	180	129	5
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	16	12	12	16	12	12	13	12	12	12	12
Satd. Flow (prot)	0	1888	0	0	1745	0	0	1730	0	0	1652	0
Flt Permitted		0.985			0.999			0.991			0.972	
Satd. Flow (perm)	0	1888	0	0	1745	0	0	1730	0	0	1652	0
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		397			764			716			461	
Travel Time (s)		9.0			17.4			16.3			10.5	
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Heavy Vehicles (%)	0%	0%	0%	0%	1%	3%	0%	0%	0%	0%	1%	0%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	265	0	0	211	0	0	279	0	0	330	0
Sign Control		Stop			Stop			Stop			Stop	

Intersection Summary

Area Type:

CBD

Control Type: Unsignalized

Intersection Capacity Utilization 76.2%

ICU Level of Service D

Intersection												
Intersection Delay, s/veh	14.7											
Intersection LOS	В											
Movement	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBR
Vol, veh/h	0	76	156	20	0	4	79	118	0	46	196	24
Peak Hour Factor	0.92	0.95	0.95	0.95	0.92	0.95	0.95	0.95	0.92	0.95	0.95	0.95
Heavy Vehicles, %	2	0	0	0	2	0	1	3	2	0	0	(
Mvmt Flow	0	80	164	21	0	4	83	124	0	48	206	25
Number of Lanes	0	0	1	0	0	0	1	0	0	0	1	(
Approach		EB				WB				NB		
Opposing Approach		WB				EB				SB		
Opposing Lanes		1				1				1		
Conflicting Approach Left		SB				NB				EB		
Conflicting Lanes Left		1				1				1.		
Conflicting Approach Right		NB				SB				WB		
Conflicting Lanes Right		1				1				1		
HCM Control Delay		14.5				12.4				14.5		
TIOM Common Dolay												
HCM LOS		В				В				В		
HCM [®] LOS		В	EBLn1	WBLn1	SBLn1	В				В		
HCM LOS		B NBLn1	EBLn1	WBLn1	SBLn1	В				В		
HCM LOS Lane Vol Left, %		B NBLn1 17%	30%	2%	57%	В				В		
Lane Vol Left, % Vol Thru, %		NBLn1 17% 74%	30% 62%	2% 39%		В				В		
Lane Vol Left, % Vol Thru, % Vol Right, %		NBLn1 17% 74% 9%	30% 62% 8%	2% 39% 59%	57% 41% 2%	В				В		
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control		NBLn1 17% 74% 9% Stop	30% 62% 8% Stop	2% 39% 59% Stop	57% 41% 2% Stop	В				В		
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane		NBLn1 17% 74% 9% Stop 266	30% 62% 8%	2% 39% 59%	57% 41% 2%	В				В		
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol		NBLn1 17% 74% 9% Stop 266 46	30% 62% 8% Stop 252	2% 39% 59% Stop 201	57% 41% 2% Stop 314	В				В		
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol		NBLn1 17% 74% 9% Stop 266	30% 62% 8% Stop 252 76	2% 39% 59% Stop 201 4	57% 41% 2% Stop 314 180	В				В		
HCM LOS Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol		NBLn1 17% 74% 9% Stop 266 46 196	30% 62% 8% Stop 252 76 156	2% 39% 59% Stop 201 4 79	57% 41% 2% Stop 314 180 129	В				В		
HCM LOS Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate		NBLn1 17% 74% 9% Stop 266 46 196 24	30% 62% 8% Stop 252 76 156 20	2% 39% 59% Stop 201 4 79 118	57% 41% 2% Stop 314 180 129 5	В				В		
HCM LOS Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp		NBLn1 17% 74% 9% Stop 266 46 196 24 280	30% 62% 8% Stop 252 76 156 20 265	2% 39% 59% Stop 201 4 79 118 212	57% 41% 2% Stop 314 180 129 5	В				В		
HCM LOS Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X)		NBLn1 17% 74% 9% Stop 266 46 196 24 280 1	30% 62% 8% Stop 252 76 156 20 265	2% 39% 59% Stop 201 4 79 118 212	57% 41% 2% Stop 314 180 129 5 331	В				В		
HCM LOS Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd)		NBLn1 17% 74% 9% Stop 266 46 196 24 280 1 0.472	30% 62% 8% Stop 252 76 156 20 265 1	2% 39% 59% Stop 201 4 79 118 212 1 0.355	57% 41% 2% Stop 314 180 129 5 331 1 0.558	В				В		
HCM LOS Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N		B NBLn1 17% 74% 9% Stop 266 46 196 24 280 1 0.472 6.068	30% 62% 8% Stop 252 76 156 20 265 1 0.46 6.246	2% 39% 59% Stop 201 4 79 118 212 1 0.355 6.036	57% 41% 2% Stop 314 180 129 5 331 1 0.558 6.081	В				В		
HCM LOS Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd)		NBLn1 17% 74% 9% Stop 266 46 196 24 280 1 0.472 6.068 Yes	30% 62% 8% Stop 252 76 156 20 265 1 0.46 6.246 Yes	2% 39% 59% Stop 201 4 79 118 212 1 0.355 6.036 Yes	57% 41% 2% Stop 314 180 129 5 331 1 0.558 6.081 Yes	В				В		
HCM LOS Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time		NBLn1 17% 74% 9% Stop 266 46 196 24 280 1 0.472 6.068 Yes 594	30% 62% 8% Stop 252 76 156 20 265 1 0.46 6.246 Yes 576	2% 39% 59% Stop 201 4 79 118 212 1 0.355 6.036 Yes 595	57% 41% 2% Stop 314 180 129 5 331 1 0.558 6.081 Yes 592	В				В		
HCM LOS Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap		NBLn1 17% 74% 9% Stop 266 46 196 24 280 1 0.472 6.068 Yes 594 4.115	30% 62% 8% Stop 252 76 156 20 265 1 0.46 6.246 Yes 576 4.295	2% 39% 59% Stop 201 4 79 118 212 1 0.355 6.036 Yes 595 4.086	57% 41% 2% Stop 314 180 129 5 331 1 0.558 6.081 Yes 592 4.127	В				В		
HCM LOS Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time HCM Lane V/C Ratio		NBLn1 17% 74% 9% Stop 266 46 196 24 280 1 0.472 6.068 Yes 594 4.115 0.471	30% 62% 8% Stop 252 76 156 20 265 1 0.46 6.246 Yes 576 4.295 0.46	2% 39% 59% Stop 201 4 79 118 212 1 0.355 6.036 Yes 595 4.086 0.356	57% 41% 2% Stop 314 180 129 5 331 1 0.558 6.081 Yes 592 4.127 0.559	В				В		

Intersection Delay, s/veh											
Intersection LOS											
Movement	SBU	SBL	SBT	SBR	Tak	0.39		1995			
Vol, veh/h	0	180	129	5							
Peak Hour Factor	0.92	0.95	0.95	0.95							
Heavy Vehicles, %	2	0	1	0							
Mvmt Flow	0	189	136	5							
Number of Lanes	0	0	1	0							
Approach	N. S. K.	SB		CVYY VIII			Leou-	v ik		7117L	
Opposing Approach		NB									
Opposing Lanes		. 1									
Conflicting Approach Left		WB									
Conflicting Lanes Left		_1									
Conflicting Approach Right		EB									
Conflicting Lanes Right		1									
HCM Control Delay		16.6									
HCM LOS		С									

	•	→	*	1	-	4	4	†	-	-	↓	1
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			44+			4			4	
Volume (vph)	12	182	44	6	170	142	84	116	39	304	193	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	16	12	12	16	12	12	13	12	12	12	12
Satd. Flow (prot)	0	1857	0	0	1796	0	0	1682	0	0	1626	0
FIt Permitted		0.998			0.999			0.983			0.970	
Satd. Flow (perm)	0	1857	0	0	1796	0	0	1682	0	0	1626	0
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		397			764			716			461	
Travel Time (s)		9.0			17.4			16.3			10.5	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	0%	2%	0%	0%	0%	3%	0%	2%	0%	2%	2%	0%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	264	0	0	354	0	0	265	0	0	552	0
Sign Control		Stop			Stop			Stop			Stop	

Intersection Summary

Area Type:

CBD

Control Type: Unsignalized

Intersection Capacity Utilization 76.4%

ICU Level of Service D

Intersection												
Intersection Delay, s/veh	40.2											
Intersection LOS	E											
Movement	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBF
Vol. veh/h	0	12	182	44	0	6	170	142	0	84	116	39
Peak Hour Factor	0.92	0.90	0.90	0.90	0.92	0.90	0.90	0.90	0.92	0.90	0.90	0.90
Heavy Vehicles, %	2	0	2	0	2	0	0	3	2	0	2	(
Mvmt Flow	0	13	202	49	0	7	189	158	0	93	129	43
Number of Lanes	0	0	1	0	0	0	1	0	0	0	1	(
Approach	- 30	EB			-	WB				NB		
Opposing Approach		WB				EB				SB		
Opposing Lanes		1				1				1		
Conflicting Approach Left		SB				NB				EB		
Conflicting Lanes Left		1				1				1		
Conflicting Approach Right		NB				SB				WB		
Conflicting Lanes Right		1				1				1		
HCM Control Delay		20.6				26.8				20.4		
		C				D				C		
HCM LOS		U										
			EBLn1	WBLn1	SBLn1	25 10						
Lane		NBLn1	EBLn1	WBLn1	SBLn1	20 13						
Lane Vol Left, %		NBLn1 35%	5%	2%	61%	2 1						
Lane Vol Left, % Vol Thru, %		NBLn1 35% 49%	5% 76%	2% 53%	61% 39%	20 10						
Lane Vol Left, % Vol Thru, % Vol Right, %		NBLn1 35% 49% 16%	5% 76% 18%	2% 53% 45%	61% 39% 0%							
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control		NBLn1 35% 49% 16% Stop	5% 76% 18% Stop	2% 53% 45% Stop	61% 39% 0% Stop							
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane		NBLn1 35% 49% 16% Stop 239	5% 76% 18% Stop 238	2% 53% 45% Stop 318	61% 39% 0% Stop 497							
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol		NBLn1 35% 49% 16% Stop 239 84	5% 76% 18% Stop 238 12	2% 53% 45% Stop 318 6	61% 39% 0% Stop 497 304							
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol		NBLn1 35% 49% 16% Stop 239 84 116	5% 76% 18% Stop 238 12 182	2% 53% 45% Stop 318 6 170	61% 39% 0% Stop 497 304 193							
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol		NBLn1 35% 49% 16% Stop 239 84 116 39	5% 76% 18% Stop 238 12 182 44	2% 53% 45% Stop 318 6 170 142	61% 39% 0% Stop 497 304 193							
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate		NBLn1 35% 49% 16% Stop 239 84 116 39 266	5% 76% 18% Stop 238 12 182	2% 53% 45% Stop 318 6 170	61% 39% 0% Stop 497 304 193							
Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp		NBLn1 35% 49% 16% Stop 239 84 116 39 266 1	5% 76% 18% Stop 238 12 182 44 264	2% 53% 45% Stop 318 6 170 142 353	61% 39% 0% Stop 497 304 193 0 552							
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X)		NBLn1 35% 49% 16% Stop 239 84 116 39 266 1 0.567	5% 76% 18% Stop 238 12 182 44 264 1 0.569	2% 53% 45% Stop 318 6 170 142 353	61% 39% 0% Stop 497 304 193 0 552							
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd)		NBLn1 35% 49% 16% Stop 239 84 116 39 266 1 0.567 7.685	5% 76% 18% Stop 238 12 182 44 264 1 0.569 7.74	2% 53% 45% Stop 318 6 170 142 353 1 0.716 7.29	61% 39% 0% Stop 497 304 193 0 552 1 1 7.218							
Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N		NBLn1 35% 49% 16% Stop 239 84 116 39 266 1 0.567 7.685 Yes	5% 76% 18% Stop 238 12 182 44 264 1 0.569	2% 53% 45% Stop 318 6 170 142 353 1 0.716	61% 39% 0% Stop 497 304 193 0 552 1							
Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap		NBLn1 35% 49% 16% Stop 239 84 116 39 266 1 0.567 7.685 Yes 467	5% 76% 18% Stop 238 12 182 44 264 1 0.569 7.74 Yes 465	2% 53% 45% Stop 318 6 170 142 353 1 0.716 7.29 Yes 495	61% 39% 0% Stop 497 304 193 0 552 1 1 7.218 Yes 503							
Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time		NBLn1 35% 49% 16% Stop 239 84 116 39 266 1 0.567 7.685 Yes 467 5.76	5% 76% 18% Stop 238 12 182 44 264 1 0.569 7.74 Yes 465 5.807	2% 53% 45% Stop 318 6 170 142 353 1 0.716 7.29 Yes 495 5.35	61% 39% 0% Stop 497 304 193 0 552 1 1 7.218 Yes 503 5.31							
Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time HCM Lane V/C Ratio		NBLn1 35% 49% 16% Stop 239 84 116 39 266 1 0.567 7.685 Yes 467 5.76 0.57	5% 76% 18% Stop 238 12 182 44 264 1 0.569 7.74 Yes 465 5.807 0.568	2% 53% 45% Stop 318 6 170 142 353 1 0.716 7.29 Yes 495 5.35 0.713	61% 39% 0% Stop 497 304 193 0 552 1 1 7.218 Yes 503 5.31 1.097							
Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time		NBLn1 35% 49% 16% Stop 239 84 116 39 266 1 0.567 7.685 Yes 467 5.76	5% 76% 18% Stop 238 12 182 44 264 1 0.569 7.74 Yes 465 5.807	2% 53% 45% Stop 318 6 170 142 353 1 0.716 7.29 Yes 495 5.35	61% 39% 0% Stop 497 304 193 0 552 1 1 7.218 Yes 503 5.31							

Intersection Delay, s/veh												
Intersection LOS												
Movement	SBU	SBL	SBT	SBR		Mary.	AL S	1184				
Vol, veh/h	0	304	193	0								
Peak Hour Factor	0.92	0.90	0.90	0.90								
Heavy Vehicles, %	2	2	2	0								
Mvmt Flow	0	338	214	0								
Number of Lanes	0	0	1	0								
		CD			201	JJAKSHI C	PST (L	N 2010 N	87.0	IIO EWOS	W 85.45	16.0
Approach	70 di	SB	"Here's									
Opposing Approach		NB										
Opposing Lanes		1										
Conflicting Approach Left		WB										
Conflicting Lanes Left		1										
Conflicting Approach Right		EB										
Conflicting Lanes Right		1										
HCM Control Delay		67.7										
HCM LOS		F - F										

	*	→	*	1	←	*	4	†	-	-		1
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	146 De (466	4	12343		4			4			47+	
Volume (vph)	82	207	34	6	118	128	62	216	31	198	150	5
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	16	12	12	16	12	12	13	12	12	12	12
Satd. Flow (prot)	0	1888	0	0	1767	0	0	1725	0	0	1653	0
Flt Permitted		0.988			0.999			0.990			0.973	
Satd. Flow (perm)	0	1888	0	0	1767	0	0	1725	0	0	1653	0
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		397			764			716			461	
Travel Time (s)		9.0			17.4			16.3			10.5	
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Heavy Vehicles (%)	0%	0%	0%	0%	1%	3%	0%	0%	0%	0%	1%	0%
Shared Lane Traffic (%)		(5.46)										
Lane Group Flow (vph)	0	340	0	0	265	0	0	325	0	0	371	0
Sign Control		Stop			Stop	2		Stop			Stop	

Intersection Summary

Area Type:

CBD

Control Type: Unsignalized

Intersection Capacity Utilization 88.6%

Analysis Period (min) 15

ICU Level of Service E

Intersection												
Intersection Delay, s/veh Intersection LOS	23.7 C											
Movement	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBF
Vol, veh/h	0	82	207	34	0	6	118	128	0	62	216	31
Peak Hour Factor	0.92	0.95	0.95	0.95	0.92	0.95	0.95	0.95	0.92	0.95	0.95	0.95
Heavy Vehicles, %	2	0	0	0	2	0	1	3	2	0	0	(
Mvmt Flow	0	86	218	36	0	6	124	135	0	65	227	33
Number of Lanes	0	0	1	0	0	0	1	0	0	0	1	(
Approach		EB				WB				NB		
Opposing Approach		WB				EB				SB		
Opposing Lanes		1				1				1		
Conflicting Approach Left		SB				NB				EB		
Conflicting Lanes Left		1				1				1		
Conflicting Approach Right		NB				SB				WB		
Conflicting Lanes Right		1				1				1		
						101				20.7		
HCM Control Delay		24.4				18.1				22.7		
		24.4 C				18.1 C				C C		
HCM Control Delay			EBLn1	WBLn1	SBLn1							
HCM Control Delay HCM LOS Lane		С	EBLn1 25%	WBLn1	SBLn1 56%							
HCM Control Delay HCM LOS Lane Vol Left, %		C NBLn1										
HCM Control Delay HCM LOS Lane Vol Left, % Vol Thru, %		C NBLn1 20%	25%	2%	56%							
HCM Control Delay HCM LOS Lane Vol Left, % Vol Thru; % Vol Right, %		C NBLn1 20% 70%	25% 64%	2% 47%	56% 42%							
HCM Control Delay HCM LOS Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control		C NBLn1 20% 70% 10%	25% 64% 11%	2% 47% 51%	56% 42% 1%							
HCM Control Delay HCM LOS Lane Vol Left, % Vol Thru; % Vol Right, %		C NBLn1 20% 70% 10% Stop	25% 64% 11% Stop	2% 47% 51% Stop	56% 42% 1% Stop							
HCM Control Delay HCM LOS Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT. Vol		NBLn1 20% 70% 10% Stop 309	25% 64% 11% Stop 323	2% 47% 51% Stop 252	56% 42% 1% Stop 353							
HCM Control Delay HCM LOS Lane Vol Left, % Vol Thru; % Vol Right, % Sign Control Traffic Vol by Lane	: f :	20% 70% 10% Stop 309 62	25% 64% 11% Stop 323 82	2% 47% 51% Stop 252 6	56% 42% 1% Stop 353 198							
HCM Control Delay HCM LOS Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT: Vol Through Vol		C NBLn1 20% 70% 10% Stop 309 62 216	25% 64% 11% Stop 323 82 207	2% 47% 51% Stop 252 6 118	56% 42% 1% Stop 353 198 150							
HCM Control Delay HCM LOS Lane Vol Left, % Vol Thru; % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol		C NBLn1 20% 70% 10% Stop 309 62 216 31	25% 64% 11% Stop 323 82 207 34	2% 47% 51% Stop 252 6 118 128 265	56% 42% 1% Stop 353 198 150 5 372							
HCM Control Delay HCM LOS Lane Vol Left, % Vol Thru; % Vol Right, % Sign Control Traffic Vol by Lane LT: Vol Through Vol RT Vol Lane Flow Rate		C NBLn1 20% 70% 10% Stop 309 62 216 31 325	25% 64% 11% Stop 323 82 207 34 340	2% 47% 51% Stop 252 6 118 128 265 1	56% 42% 1% Stop 353 198 150 5 372 1 0.738							
HCM Control Delay HCM LOS Lane Vol Left, % Vol Thru; % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp		C NBLn1 20% 70% 10% Stop 309 62 216 31 325 1	25% 64% 11% Stop 323 82 207 34 340 1 0.68 7.198	2% 47% 51% Stop 252 6 118 128 265 1 0.529 7.179	56% 42% 1% Stop 353 198 150 5 372 1 0.738 7.154							
HCM Control Delay HCM LOS Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X)		C NBLn1 20% 70% 10% Stop 309 62 216 31 325 1 0.648	25% 64% 11% Stop 323 82 207 34 340 1 0.68 7.198 Yes	2% 47% 51% Stop 252 6 118 128 265 1 0.529 7.179 Yes	56% 42% 1% Stop 353 198 150 5 372 1 0.738 7.154 Yes							
HCM Control Delay HCM LOS Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd)		C NBLn1 20% 70% 10% Stop 309 62 216 31 325 1 0.648 7.176	25% 64% 11% Stop 323 82 207 34 340 1 0.68 7.198	2% 47% 51% Stop 252 6 118 128 265 1 0.529 7.179 Yes 499	56% 42% 1% Stop 353 198 150 5 372 1 0.738 7.154 Yes 502							
HCM Control Delay HCM LOS Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N		C NBLn1 20% 70% 10% Stop 309 62 216 31 325 1 0.648 7.176 Yes	25% 64% 11% Stop 323 82 207 34 340 1 0.68 7.198 Yes	2% 47% 51% Stop 252 6 118 128 265 1 0.529 7.179 Yes	56% 42% 1% Stop 353 198 150 5 372 1 0.738 7.154 Yes 502 5.233							
HCM Control Delay HCM LOS Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap		NBLn1 20% 70% 10% Stop 309 62 216 31 325 1 0.648 7.176 Yes 501	25% 64% 11% Stop 323 82 207 34 340 1 0.68 7.198 Yes 500	2% 47% 51% Stop 252 6 118 128 265 1 0.529 7.179 Yes 499 5.265 0.531	56% 42% 1% Stop 353 198 150 5 372 1 0.738 7.154 Yes 502 5.233 0.741							
HCM Control Delay HCM LOS Lane Vol Left, % Vol Thru; % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time HCM Lane V/C Ratio		NBLn1 20% 70% 10% Stop 309 62 216 31 325 1 0.648 7.176 Yes 501 5.259	25% 64% 11% Stop 323 82 207 34 340 1 0.68 7.198 Yes 500 5.276 0.68 24.4	2% 47% 51% Stop 252 6 118 128 265 1 0.529 7.179 Yes 499 5.265 0.531 18.1	56% 42% 1% Stop 353 198 150 5 372 1 0.738 7.154 Yes 502 5.233 0.741 27.9							
HCM Control Delay HCM LOS Lane Vol Left, % Vol Thru; % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time		NBLn1 20% 70% 10% Stop 309 62 216 31 325 1 0.648 7.176 Yes 501 5.259 0.649	25% 64% 11% Stop 323 82 207 34 340 1 0.68 7.198 Yes 500 5.276 0.68	2% 47% 51% Stop 252 6 118 128 265 1 0.529 7.179 Yes 499 5.265 0.531	56% 42% 1% Stop 353 198 150 5 372 1 0.738 7.154 Yes 502 5.233 0.741							

Intersection					
Intersection Delay, s/veh Intersection LOS					
Movement	SBU	SBL	SBT	SBR	
Vol, veh/h	0	198	150	5	
Peak Hour Factor	0.92	0.95	0.95	0.95	
Heavy Vehicles, %	2	0	1	0	
Mvmt Flow	0	208	158	5	
Number of Lanes	0	0	1	0	
Approach	MI IS	SB			
Opposing Approach		NB			
Opposing Lanes		1			
Conflicting Approach Left		WB			
Conflicting Lanes Left		1			
Conflicting Approach Right		EB			
Conflicting Lanes Right		1			
HCM Control Delay		27.9			
HCM LOS		D			

	1	-	7	1	←		1	†	-	-	↓	1
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Volume (vph)	12	184	44	6	175	146	84	116	39	306	193	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	16	12	12	16	12	12	13	12	12	12	12
Satd. Flow (prot)	0	1857	0	0	1796	0	0	1682	0	0	1626	0
Flt Permitted		0.998			0.999			0.983			0.970	
Satd. Flow (perm)	0	1857	0	0	1796	0	0	1682	0	0	1626	0
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		397			764			716			461	
Travel Time (s)		9.0			17.4			16.3			10.5	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	0%	2%	0%	0%	0%	3%	0%	2%	0%	2%	2%	0%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	266	0	0	363	0	0	265	0	0	554	0
Sign Control		Stop			Stop			Stop			Stop	

Intersection Summary

Area Type:

CBD

Control Type: Unsignalized

Intersection Capacity Utilization 77.1%

Analysis Period (min) 15

ICU Level of Service D

Intersection	TILL!		4000		Kindle (per d		18.6	6,040	V met libra		
Intersection Delay, s/veh	40.9											
Intersection LOS	E											
Movement	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBF
Vol, veh/h	0	12	184	44	0	6	175	146	0	84	116	3
Peak Hour Factor	0.92	0.90	0.90	0.90	0.92	0.90	0.90	0.90	0.92	0.90	0.90	0.9
Heavy Vehicles, %	2	0	2	0	2	0	0	3	2	0	2	
Mvmt Flow	0	13	204	49	0	7	194	162	0	93	129	4
Number of Lanes	0	0	1	0	0	0	1	0	0	0	1	
Approach		EB		N How		WB		2874 Y.T.	Trans.	NB	Sul Val	188 A.
Opposing Approach		WB				EB				SB		
Opposing Lanes		1				1				1		
Conflicting Approach Left		SB				NB				EB		
Conflicting Lanes Left		1				1				1		
Conflicting Approach Right		NB				SB				WB		
Conflicting Lanes Right		1				1				1		
HCM Control Delay		21.1				28.5				20.8		
HCM LOS		C				D				C		
Lane	126.4	VBLn1	EBLn1	WBLn1	SBLn1	No. 3.4	13 OF	16 × 183				A RAIS
Vol Left, %		35%	5%	2%	61%							
Vol Thru, %		49%	77%	54%	39%							
Vol Right, %		16%	18%	45%	0%							
Sign Control		Stop	Stop	Stop	Stop							
Traffic Vol by Lane		239	240	327	499							
LT Vol		84	12	6	306							
Through Vol		116	184	175	193							
RT Vol		39	44	146	0							
Lane Flow Rate		266	267	363	554							
Geometry Grp		1		_ 1	1							
Degree of Util (X)		0.573	0.578	0.739	1							
Departure Headway (Hd)		7.773	7.804	7.322	7.303							
Convergence, Y/N		Yes	Yes	Yes	Yes							
Сар		462	461	494	493							
Service Time		5.84	5.868	5.38	5.389							
HCM Lane V/C Ratio		0.576	0.579	0.735	1.124							
HCM Control Delay		20.8	21.1	28.5	68.1							
HCM Lane LOS		С	С	D	F							
HCM 95th-tile Q		3.5	3.6	6.1	13.5							

Intersection Delay, s/veh Intersection LOS					
Movement	SBU	SBL	SBT	SBR	
Vol, veh/h	0	306	193	0	
Peak Hour Factor	0.92	0.90	0.90	0.90	
Heavy Vehicles, %	2	2	2	0	
Mvmt Flow	0	340	214	0	
Number of Lanes	0	0	1	0	
A source to		SB		97E 115	
Approach	FILE OF SEC.		Seller		
Opposing Approach		NB			
Opposing Lanes		1			
Conflicting Approach Left		WB			
Conflicting Lanes Left		1			
Conflicting Approach Right		EB			
Conflicting Lanes Right		1			
HCM Control Delay		68.1			
HCM LOS		F			

	1		*	•	-	*	4	†	-	-	↓	1
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			43-			4			4	
Volume (vph)	82	214	34	6	122	132	62	216	31	202	150	5
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	16	12	12	16	12	12	13	12	12	12	12
Satd. Flow (prot)	0	1888	0	0	1767	0	0	1725	0	0	1652	0
Flt Permitted		0.988			0.999			0.990			0.972	
Satd. Flow (perm)	0	1888	0	0	1767	0	0	1725	0	0	1652	0
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		397			764			716			461	
Travel Time (s)		9.0			17.4			16.3			10.5	
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Heavy Vehicles (%)	0%	0%	0%	0%	1%	3%	0%	0%	0%	0%	1%	0%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	. 0	347	0	0	273	0	0	325	0	0	376	0
Sign Control	1. * -	Stop			Stop			Stop			Stop	

Intersection Summary

Area Type:

CBD

Control Type: Unsignalized

Intersection Capacity Utilization 89.7%

ICU Level of Service E

Intersection		3,									
Intersection Delay, s/veh Intersection LOS	25.3 D										
		COT	EDD	MOUL	MOL	WDT	WIDD	NIDIT	NBL	NBT	NDE
Movement	EBU EBL	EBT	EBR	WBU	WBL	WBT 122	WBR 132	NBU 0	62	216	NBF
Vol, veh/h	0 82	214	34	0	6	0.95			0.95	0.95	0.9
Peak Hour Factor	0.92 0.95	0.95	0.95	0.92	0.95		0.95	0.92		0.93	0.5.
Heavy Vehicles, %	2 0	0	0	2	0	1	3	2	0		3
Mvmt Flow	0 86	225	36	0	6	128	139	0	65	227	3.
Number of Lanes	0 0	1	0	0	0	1	0	0	0	1	,
Approach	ЕВ				WB				NB		
Opposing Approach	WB				EB				SB		
Opposing Lanes	1				1				1		
Conflicting Approach Left	SB				NB				EB		
Conflicting Lanes Left	%35,5 3 1 1				1				1		
Conflicting Approach Right	NB				SB				WB		
Conflicting Lanes Right					1				1		
HCM Control Delay	26.2				19.5				23.8		
HCM LOS	D				C				C		
The state of the s											
Lane	NBLn1	EBLn1	WBLn1	SBLn1							
Vol Left, %	20%	25%	2%	57%							
Vol Thru, %	70%	65%	47%	42%							
Vol Right, %	10%	10%	51%	1%							
Sign Control	Stop	Stop	Stop	Stop							
Traffic Vol by Lane	309	330	260	357							
LT Vol	62	82	6	202							
Through Vol	216	214	122	150							
RT Vol	. 31	34	132	5							
		347	274	376							
	325	347	214	010							
Lane Flow Rate	325 1	347	1	1							
Lane Flow Rate Geometry Grp											
Lane Flow Rate Geometry Grp Degree of Util (X)	1	1	1	1							
Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd)	1 0.661	1 0.705	1 0.562	1 0.76							
Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N	0.661 7.43	1 0.705 7.413	0.562 7.391	0.76 7.384							
Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap	1 0.661 7.43 Yes 488	1 0.705 7.413 Yes	1 0.562 7.391 Yes	1 0.76 7.384 Yes							
Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time	1 0.661 7.43 Yes 488 5.43	1 0.705 7.413 Yes 490 5.413	1 0.562 7.391 Yes 490 5.391	1 0.76 7.384 Yes 492							
Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time HCM Lane V/C Ratio	1 0.661 7.43 Yes 488 5.43 0.666	1 0.705 7.413 Yes 490 5.413 0.708	1 0.562 7.391 Yes 490 5.391 0.559	1 0.76 7.384 Yes 492 5.384							
Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time HCM Lane V/C Ratio HCM Control Delay HCM Lane LOS	1 0.661 7.43 Yes 488 5.43	1 0.705 7.413 Yes 490 5.413	1 0.562 7.391 Yes 490 5.391	1 0.76 7.384 Yes 492 5.384 0.764							

5.5

4.8

6.6

3.4

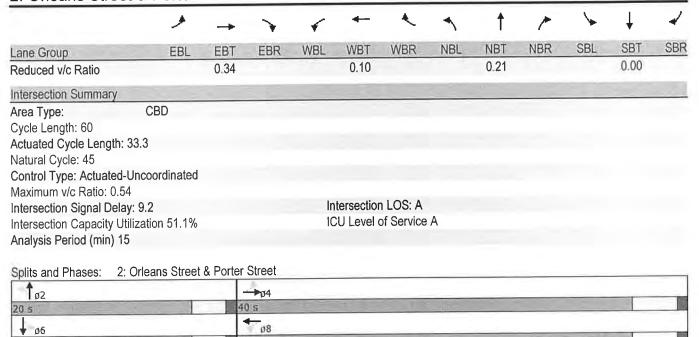
HCM 95th-tile Q

Intersection Delay, s/veh					
Intersection LOS					
Movement	SBU	SBL	SBT	SBR	
Vol, veh/h	0	202	150	5	
Peak Hour Factor	0.92	0.95	0.95	0.95	
Heavy Vehicles, %	2	0	1	0	
Mvmt Flow	0	213	158	5	
Number of Lanes	0	0	1	0	
Approach		SB		88. Jul 20	(2) - 4 (1 (b) "A) & FT & FT (8) (FT (7) (1) (1) (1) (1) (1) (1) (1)
Opposing Approach	NEOGNA PE	NB			
Opposing Lanes		1			
Conflicting Approach Left		WB			
Conflicting Lanes Left		1			
Conflicting Approach Right		EB			
Conflicting Lanes Right		1			
HCM Control Delay		30.1			
HCM LOS		D			



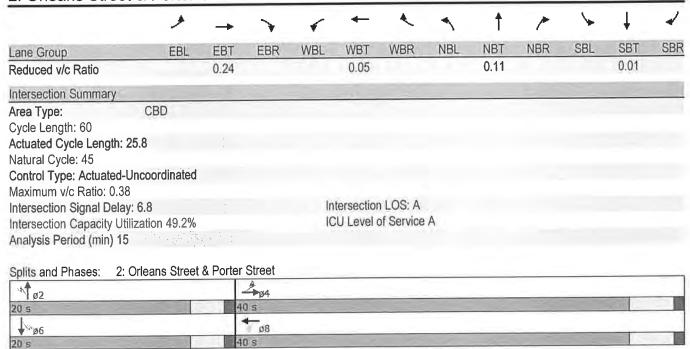
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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			43+			↔			4	
Volume (vph)	3	360	85	0	139	0	129	1	3	1	1	2
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12	12	15	12	12	15	12
Satd. Flow (prot)	0	1643	0	0	1710	0	0	1738	0	0	1732	0
FIt Permitted		0.998						0.729			0.923	
Satd. Flow (perm)	0	1640	0	0	1710	0	0	1328	0	0	1618	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		35						2			2	
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		764			713			700			308	
Travel Time (s)		17.4			16.2			15.9			7.0	
Peak Hour Factor	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88
Heavy Vehicles (%)	0%	1%	3%	0%	0%	0%	3%	0%	0%	0%	0%	0%
Shared Lane Traffic (%)	070	1 70	0.10	•								
Lane Group Flow (vph)	= = 0	509	0	0	158	0	0	151	0	0	4	0
Turn Type	Perm	NA	0	0	NA		Perm	NA		Perm	NA	
Protected Phases	1 01111	4			8			2			6	
Permitted Phases	4	7		8			2			6		
Detector Phase	4	4		8	8		2	2		6	6	
Switch Phase		7					_	_				
Minimum Initial (s)	4.0	4.0		4.0	4.0		4.0	4.0		4.0	4.0	
	21.0	21.0		21.0	21.0		20.0	20.0		20.0	20.0	
Minimum Split (s)	40.0	40.0		40.0	40.0		20.0	20.0		20.0	20.0	
Total Split (s)	66.7%	66.7%		66.7%	66.7%		33.3%	33.3%		33.3%	33.3%	
Total Split (%)	4.0	4.0		4.0	4.0		3.0	3.0		3.0	3.0	
Yellow Time (s)	1.0	1.0		1.0	1.0		1.0	1.0		1.0	1.0	
All-Red Time (s)	1.0	-1.0		1.0	-1.0		1.0	0.0		1.0	0.0	
Lost Time Adjust (s)		4.0			4.0			4.0			4.0	
Total Lost Time (s)		4.0			4.0			7.0			4.0	
Lead/Lag												
Lead-Lag Optimize?	Nana	None		None	None		None	None		None	None	
Recall Mode	None	None		None	17.9		140116	11.2		None	11.0	
Act Effct Green (s)		18.8			0.54			0.34			0.33	
Actuated g/C Ratio		0.56			0.34			0.34			0.01	
v/c Ratio		0.54			5.9			15.0			10.5	
Control Delay		8.6						0.0			0.0	
Queue Delay		0.0			0.0			15.0			10.5	
Total Delay		8.6			5.9			15.0 B			10.5 B	
LOS		A			A						10.5	
Approach Delay		8.6			5.9			15.0			10.3 B	
Approach LOS		A			A			В				
Queue Length 50th (ft)		52			14			22 77			0	
Queue Length 95th (ft)		147			43						6	
Internal Link Dist (ft)		684			633			620			228	
Turn Bay Length (ft)					4=00			705			000	
Base Capacity (vph)		1500			1560			725			883	
Starvation Cap Reductn		0			0			0			0	
Spillback Cap Reductn		0			0			0			0	
Storage Cap Reductn		0			0			0			0	

20 s

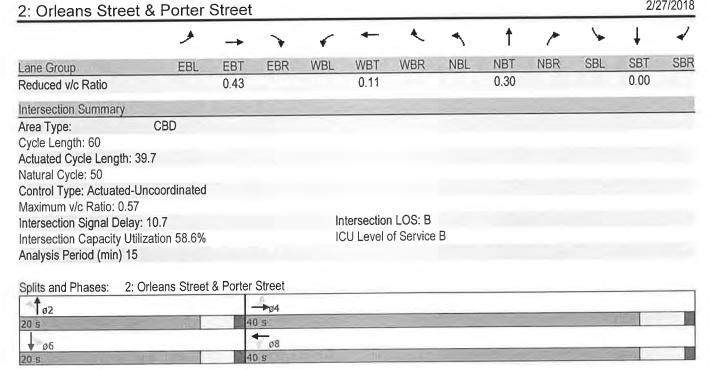


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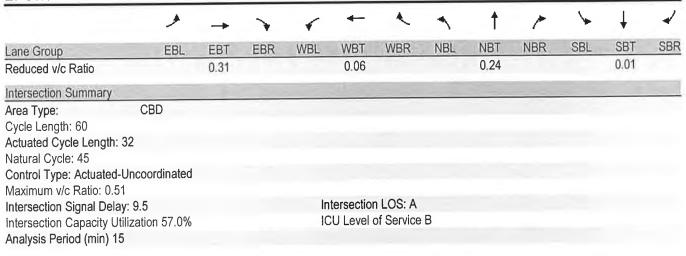
	1	-	*	1	4-	*	1	†	1	1	Ţ	1
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Volume (vph)	12	271	77	1	75	1	120	1	0	0	1	6
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12	12	15	12	12	15	12
Satd. Flow (prot)	0	1657	0	0	1688	0	0	1793	0	0	1663	C
Flt Permitted		0.990			0.994			0.885				
Satd. Flow (perm)	0	1644	0	0	1680	0	0	1665	0	0	1663	(
Right Turn on Red	ŭ		Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		41			1						6	
Link Speed (mph)		30			30			30			30	
Link Opeed (mpn) Link Distance (ft)		764			713			700			308	
Travel Time (s)		17.4			16.2			15.9			7.0	
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
	0.94	0.34	0.54	0%	1%	0%	0%	0%	0%	0%	0%	0%
Heavy Vehicles (%)	070	076	0 /0	0 70	1 70	0 70	070	070	070	070	0 10	
Shared Lane Traffic (%)	0	383	0	0	82	0	0	129	0	0	7	(
Lane Group Flow (vph)	0		U	Perm	NA	U	Perm	NA	U	U	NA.	
Turn Type	Perm	NA		Pellii	8		r em	2			6	
Protected Phases		4		0	0		2	2		6	U	
Permitted Phases	4			8	0		2 2	2		6	6	
Detector Phase	4	4		8	8		2	2		0	0	
Switch Phase				4.0	4.0		4.0	4.0		4.0	4.0	
Minimum Initial (s)	4.0	4.0		4.0	4.0		4.0	4.0		4.0		
Minimum Split (s)	21.0	21.0		21.0	21.0		20.0	20.0		20.0	20.0	
Total Split (s)	40.0	40.0		40.0	40.0		20.0	20.0		20.0	20.0	
Total Split (%)	66.7%	66.7%		66.7%	66.7%		33.3%	33.3%		33.3%	33.3%	
Yellow Time (s)	4.0	4.0		4.0	4.0		3.0	3.0		3.0	3.0	
All-Red Time (s)	1.0	1.0		1.0	1.0		1.0	1.0		1.0	1.0	
Lost Time Adjust (s)		-1.0			-1.0			0.0			0.0	
Total Lost Time (s)		4.0			4.0			4.0			4.0	
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	None	None		None	None		None	None		None	None	
Act Effct Green (s)		15.5			15.0			9.2			8.7	
Actuated g/C Ratio		0.60			0.58			0.36			0.34	
v/c Ratio		0.38			0.08			0.22			0.01	
Control Delay		6.0			4.9			10.1			7.0	
Queue Delay		0.0			0.0			0.0			0.0	
Total Delay		6.0			4.9			10.1			7.0	
LOS		Α			А			В			Α	
Approach Delay		6.0			4.9			10.1			7.0	
Approach LOS		А			Α			В			Α	
Queue Length 50th (ft)		30			6			14			0	
Queue Length 95th (ft)		89			22			52			6	
Internal Link Dist (ft)		684			633			620			228	
Turn Bay Length (ft)		004			000			023				
		1603			1637			1148			1148	
Base Capacity (vph)		0			0			0			0	
Starvation Cap Reducts		0			0			Ő			0	
Spillback Cap Reductn Storage Cap Reductn		0			0			0			0	



	*	-	*	6	←	*	1	†	-	-	ţ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Volume (vph)	3	409	118	0	149	0	167	1	4	1	1	2
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12	12	15	12	12	15	12
Satd. Flow (prot)	0	1635	0	0	1710	0	0	1739	0	0	1732	0
Flt Permitted		0.999						0.730			0.925	
Satd. Flow (perm)	0	1634	0	0	1710	0	0	1330	0	0	1622	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		43						2			2	
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		764			713			700			308	
Travel Time (s)		17.4			16.2			15.9			7.0	
Peak Hour Factor	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88
Heavy Vehicles (%)	0%	1%	3%	0%	0%	0%	3%	0%	0%	0%	0%	0%
Shared Lane Traffic (%)	0 70	1 70	070	0,0	0.70	0,0						
Lane Group Flow (vph)	0	602	0	0	169	0	0	196	0	0	4	0
	Perm	NA	0	0	NA	· ·	Perm	NA		Perm	NA	
Turn Type	r eiiii	4			8		1 01111	2			6	
Protected Phases	1	4		8	0		2	_		6		
Permitted Phases	4	4		8	8		2	2		6	6	
Detector Phase	4	4		0	0		2	4				
Switch Phase	4.0	4.0		4.0	4.0		4.0	4.0		4.0	4.0	
Minimum Initial (s)	4.0	4.0			21.0		20.0	20.0		20.0	20.0	
Minimum Split (s)	21.0	21.0		21.0			20.0	20.0		20.0	20.0	
Total Split (s)	40.0	40.0		40.0	40.0			33.3%		33.3%	33.3%	
Total Split (%)	66.7%	66.7%		66.7%	66.7%		33.3%			3.0	3.0	
Yellow Time (s)	4.0	4.0		4.0	4.0		3.0	3.0			1.0	
All-Red Time (s)	1.0	1.0		1.0	1.0		1.0	1.0		1.0		
Lost Time Adjust (s)		-1.0			-1.0			0.0			0.0	
Total Lost Time (s)		4.0			4.0			4.0			4.0	
Lead/Lag												
Lead-Lag Optimize?											Minne	
Recall Mode	None	None		None	None		None	None		None	None	
Act Effct Green (s)		25.2			25.2			12.1			11.6	
Actuated g/C Ratio		0.63			0.63			0.30			0.29	
v/c Ratio		0.57			0.16			0.48			0.01	
Control Delay		9.1			5.7			19.9			12.8	
Queue Delay		0.0			0.0			0.0			0.0	
Total Delay		9.1			5.7			19.9			12.8	
LOS		Α			Α			В			В	
Approach Delay		9.1			5.7			19.9			12.8	
Approach LOS		Α			Α			В			В	
Queue Length 50th (ft)		78			17			36			0	
Queue Length 95th (ft)		185			45			115			6	
Internal Link Dist (ft)		684			633			620			228	
Turn Bay Length (ft)												
Base Capacity (vph)		1416			1476			657			801	
Starvation Cap Reductn		0			0			0			0	
Spillback Cap Reductn		0			0			0			0	
Storage Cap Reductn		0			0			0			0	



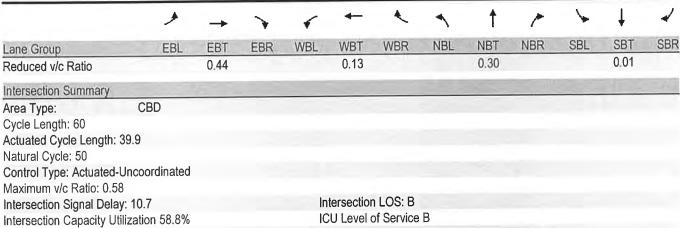
	*	-	*	1	4	*	4	†	1	1	1	1
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			₩	
Volume (vph)	13	313	110	1	80	1	166	1	2	0	1	6
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12	12	15	12	12	15	12
Satd. Flow (prot)	0	1649	0	0	1688	0	0	1789	0	0	1663	0
Flt Permitted		0.991			0.994			0.725				
Satd. Flow (perm)	0	1637	0	0	1680	0	0	1361	0	0	1663	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		51			1			1			6	
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		764			713			700			308	
Travel Time (s)		17.4			16.2			15.9			7.0	
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Heavy Vehicles (%)	0%	0%	0%	0%	1%	0%	0%	0%	0%	0%	0%	0%
Shared Lane Traffic (%)	0 70	0 70	0 70	070	1,0	0,0						
Lane Group Flow (vph)	0	464	0	0	87	0	0	180	0	0	7	0
	Perm	NA	U	Perm	NA	U	Perm	NA		_	NA	
Turn Type Protected Phases	t Cilli	4		Citii	8		1 10 11	2			6	
	1	4		8	U		2	_		6		
Permitted Phases	4	4		8	8		2	2		6	6	
Detector Phase	4	4		U	0		_	_		J		
Switch Phase	4.0	4.0		4.0	4.0		4.0	4.0		4.0	4.0	
Minimum Initial (s)	4.0	4.0		21.0	21.0		20.0	20.0		20.0	20.0	
Minimum Split (s)	21.0	21.0			40.0		20.0	20.0		20.0	20.0	
Total Split (s)	40.0	40.0		40.0			33.3%	33.3%		33.3%	33.3%	
Total Split (%)	66.7%	66.7%		66.7%	66.7%		3.0	3.0		3.0	3.0	
Yellow Time (s)	4.0	4.0		4.0	4.0					1.0	1.0	
All-Red Time (s)	1.0	1.0		1.0	1.0		1.0	1.0		1.0	0.0	
Lost Time Adjust (s)		-1.0			-1.0			0.0			4.0	
Total Lost Time (s)		4.0			4.0			4.0			4.0	
Lead/Lag												
Lead-Lag Optimize?										Mana	Mana	
Recall Mode	None	None		None	None		None	None		None	None	
Act Effct Green (s)		17.2			16.4			11.4			11.2	
Actuated g/C Ratio		0.54			0.51			0.36			0.35	
v/c Ratio		0.51			0.10			0.37			0.01	
Control Delay		8.4			5.9			14.0			8.0	
Queue Delay		0.0			0.0			0.0			0.0	
Total Delay		8.4			5.9			14.0			8.0	
LOS		Α			Α			В			Α	
Approach Delay		8.4			5.9			14.0			8.0	
Approach LOS		Α			Α			В			Α	
Queue Length 50th (ft)		44			7			24			0	
Queue Length 95th (ft)		134			27			85			7	
Internal Link Dist (ft)		684			633			620			228	
Turn Bay Length (ft)												
Base Capacity (vph)		1519			1555			759			931	
Starvation Cap Reductn		0			0			0			0	
Spillback Cap Reductn		0			0			0			0	
Storage Cap Reductn		0			0			0			0	



Splits and Phases: 2: Orleans Street & Porter Street

↑ ø2	→ ø4	
20 s	40 s	
↓ ø6	ø8	
20 s	40 s	

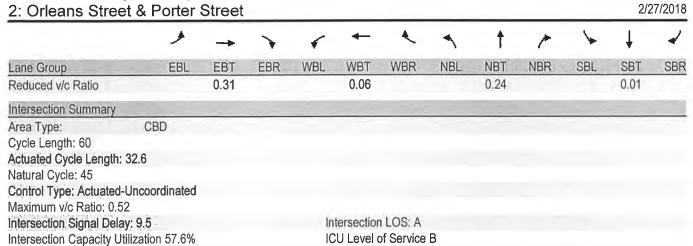
	*	\rightarrow	*	•	←	*	1	†	-	1	↓	1
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL		SBR
Lane Configurations		43+			4			4			4	
Volume (vph)	3	411	120	0	158	0	167	1	4	1	1_	2
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12	12	15	12	12	15	12
Satd. Flow (prot)	0	1635	0	0	1710	0	0	1739	0	0	1732	0
FIt Permitted		0.999						0.730			0.925	
Satd. Flow (perm)	0	1633	0	0	1710	0	0	1330	0	0	1622	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		43						2			2	
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		764			713			700			308	
Travel Time (s)		17.4			16.2			15.9			7.0	
Peak Hour Factor	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.88
Heavy Vehicles (%)	0%	1%	3%	0%	0%	0%	3%	0%	0%	0%	0%	0%
Shared Lane Traffic (%)	070	1 /0	070	0,0	0,0							
Lane Group Flow (vph)	0	606	0	0	180	0	0	196	0	0	4	0
Turn Type	Perm	NA	U	U	NA	Ū	Perm	NA		Perm	NA	
Protected Phases	Felli	4			8		1 01111	2		2 11 1 11	6	
Permitted Phases	4	7		8	U		2	_		6		
Detector Phase	4	4		8	8		2	2		6	6	
Switch Phase	4	4		U				_				
	4.0	4.0		4.0	4.0		4.0	4.0		4.0	4.0	
Minimum Initial (s)	21.0	21.0		21.0	21.0		20.0	20.0		20.0	20.0	
Minimum Split (s)	40.0	40.0		40.0	40.0		20.0	20.0		20.0	20.0	
Total Split (s)	66.7%	66.7%		66.7%	66.7%		33.3%	33.3%		33.3%	33.3%	
Total Split (%)	4.0	4.0		4.0	4.0		3.0	3.0		3.0	3.0	
Yellow Time (s)	1.0	1.0		1.0	1.0		1.0	1.0		1.0	1.0	
All-Red Time (s)	1.0			1.0	-1.0		1.0	0.0		1.0	0.0	
Lost Time Adjust (s)		-1.0			4.0			4.0			4.0	
Total Lost Time (s)		4.0			4.0			4.0			7.0	
Lead/Lag												
Lead-Lag Optimize?	Mana	Mana		Mana	None		None	None		None	None	
Recall Mode	None	None		None	None 25.4		NOHE	12.1		NONE	11.6	
Act Effct Green (s)		25.4			0.64			0.30			0.29	
Actuated g/C Ratio		0.64						0.30			0.23	
v/c Ratio		0.58			0.17			20.0			12.8	
Control Delay		9.1			5.7			0.0			0.0	
Queue Delay		0.0			0.0						12.8	
Total Delay		9.1			5.7			20.0			12.0 B	
LOS		A			A			B			12.8	
Approach Delay		9.1			5.7			20.0			12.0 B	
Approach LOS		Α			A			В			0	
Queue Length 50th (ft)		79			18			36				
Queue Length 95th (ft)		187			47			115			6	
Internal Link Dist (ft) Turn Bay Length (ft)		684			633			620			228	
Base Capacity (vph)		1381			1439			655			799	
Starvation Cap Reductn		0			0			0			0	
Spillback Cap Reductn		0			0			0			0	
Storage Cap Reductn		0			0			0			0	



Splits and Phases: 2: Orleans Street & Porter Street

4 1	- 34	
1 0 2 20 s	40 s	
₩ ø6	4 08	
20 s	40 s	The state of the s

	*	-	*	1	4	*	4	†	-	1	ļ	1
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		44			4			4			4	
Volume (vph)	13	319	115	1	80	1	166	1	2	0	1	6
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12	12	15	12	12	15	12
Satd. Flow (prot)	0	1648	0	0	1688	0	0	1789	0	0	1663	C
Flt Permitted		0.992			0.994			0.725				
Satd. Flow (perm)	0	1637	0	0	1680	0	0	1361	0	0	1663	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		52			1			1			6	
Link Speed (mph)		30			30			30			30	
Link Opeed (mpn) Link Distance (ft)		764			713			700			308	
		17.4			16.2			15.9			7.0	
Travel Time (s) Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
	0.94	0.34	0.54	0.34	1%	0%	0%	0%	0%	0%	0%	0%
Heavy Vehicles (%)	0%	070	U /0	0 70	1 /0	0 70	0 70	070	0,0	0,0		
Shared Lane Traffic (%)		475	0	0	87	0	0	180	0	0	7	0
Lane Group Flow (vph)	0	475	U		NA	U	Perm	NA	U	U	NA	
Turn Type	Perm	NA		Perm			reiiii	2			6	
Protected Phases		4		0	8		2			6	0	
Permitted Phases	4			8	0		2	2		6	6	
Detector Phase	4	4		8	8		2			O	U	
Switch Phase					4.0		4.0	4.0		4.0	4.0	
Minimum Initial (s)	4.0	4.0		4.0	4.0		4.0	4.0		4.0		
Minimum Split (s)	21.0	21.0		21.0	21.0		20.0	20.0		20.0	20.0	
Total Split (s)	40.0	40.0		40.0	40.0		20.0	20.0		20.0	20.0	
Total Split (%)	66.7%	66.7%		66.7%	66.7%		33.3%	33.3%		33.3%	33.3%	
Yellow Time (s)	4.0	4.0		4.0	4.0		3.0	3.0		3.0	3.0	
All-Red Time (s)	1.0	1.0		1.0	1.0		1.0	1.0		1.0	1.0	
Lost Time Adjust (s)		-1.0			-1.0			0.0			0.0	
Total Lost Time (s)		4.0			4.0			4.0			4.0	
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	None	None		None	None		None	None		None	None	
Act Effct Green (s)		17.8			16.9			11.5			11.3	
Actuated g/C Ratio		0.55			0.52			0.35			0.35	
v/c Ratio		0.52			0.10			0.37			0.01	
Control Delay		8.4			5.8			14.4			8.1	
Queue Delay		0.0			0.0			0.0			0.0	
Total Delay		8.4			5.8			14.4			8.1	
LOS		А			Α			В			Α	
Approach Delay		8.4			5.8			14.4			8.1	
Approach LOS		А			Α			В			Α	
Queue Length 50th (ft)		47			7			26			0	
Queue Length 95th (ft)		138			27			87			7	
Internal Link Dist (ft)		684			633			620			228	
		004			000							
Turn Bay Length (ft)		1544			1581			750			919	
Base Capacity (vph)					0			0			0	
Starvation Cap Reductn		0			0			0			Ő	
Spillback Cap Reductn Storage Cap Reductn		0			0			0			0	



Analysis Period (min) 15

Splits and Phases: 2: Orleans Street & Porter Street

Spills and Friases. 2. Offeat	S Street & Forter Street
↑ ø2	
20 s	40 s
₩ø6	4 08
20 s	40 s



	-	*	1	4-	4	1	
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	1>			4	M		
Volume (vph)	332	35	0	46	74	5	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Satd. Flow (prot)	1673	0	0	1710	1618	0	
Flt Permitted					0.955		
Satd. Flow (perm)	1673	0	0	1710	1618	0	
Link Speed (mph)	30			30	30		
Link Distance (ft)	713			898	694		
Travel Time (s)	16.2			20.4	15.8		
Peak Hour Factor	0.86	0.86	0.86	0.86	0.86	0.86	
Heavy Vehicles (%)	1%	0%	0%	0%	0%	0%	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	427	0	0	53	92	0	
Sign Control	Free			Free	Stop		

Intersection Summary

Area Type:

CBD

Control Type: Unsignalized

Intersection Capacity Utilization 33.3%

ICU Level of Service A

Intersection							3.		
nt Delay, s/veh	2								
Movement	EB	T EBF	2	WBL	WBT	1	NBL	NBR	
Vol, veh/h	33			0	46		74	5	
Conflicting Peds, #/hr		0 (0	0		0	0	
Sign Control	Fre			Free	Free	8	Stop	Stop	
RT Channelized	110	- None			None		-	None	
Storage Length				2	-		0	3.5	
Veh in Median Storage, #			- -		0		0		
		4.	-		0		0		
Grade, %		6 8		86	86		86	86	
Peak Hour Factor	C)	0	0		0	0	
Heavy Vehicles, %	20			0	53		86	6	
Mvmt Flow	38	6 4		U	JJ		00	0	
Major/Minor	Major	1		Major2		Mir	nor1		
Conflicting Flow All	00000		0	427	0		459	406	
Stage 1			-	-	-		406	+	
Stage 2				-			53	-	
Critical Hdwy				4.1			6.4	6.2	
				-			5.4		
Critical Hdwy Stg 1			-	-			5.4		
Critical Hdwy Stg 2		-		2.2			3.5	3.3	
Follow-up Hdwy		-	_	1143			564	649	
Pot Cap-1 Maneuver				1145			677	-	
Stage 1			-				975	1.2	
Stage 2		•	•	-	-		313		
Platoon blocked, %		7	•	4440			564	649	
Mov Cap-1 Maneuver		*	•	1143	*			049	
Mov Cap-2 Maneuver			*	-			564	-	
Stage 1			•				677		
Stage 2		-	-	-			975	,	
Angroach		В		WB			NB		
Approach		0		0			12.5		
HCM Control Delay, s		U		U			В		
HCM LOS							D		
Minor Lane/Major Mymt	NBLn1 EB	T EB	R WBL	WBT					
Capacity (veh/h)	569		- 1143	- Annahili					
HCM Lane V/C Ratio	0.161	-	1170						
		3	- 0						
HCM Control Delay (s)	12.5								
HCM Lane LOS	В	-	- A	-					
HCM 95th %tile Q(veh)	0.6	•	- 0	7					

	-	*	•	-	1	1	
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	1>			4	ήγf		
Volume (vph)	233	32	1	24	44	6	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Satd. Flow (prot)	1683	0	0	1628	1612	0	
Flt Permitted				0.998	0.958		
Satd. Flow (perm)	1683	0	0	1628	1612	0	
Link Speed (mph)	30			30	30		
Link Distance (ft)	713			898	694		
Travel Time (s)	16.2			20.4	15.8		
Peak Hour Factor	0.85	0.85	0.85	0.85	0.85	0.85	
Heavy Vehicles (%)	0%	0%	0%	5%	0%	0%	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	312	0	0	29	59	0	
Sign Control	Free			Free	Stop		
Intersection Summary							
Area Type:	CBD						
Control Type: Unsignalized							
Intersection Capacity Utiliza				[0	CU Level	of Service A	
Analysis Period (min) 15							

nt Delay, s/veh 1.	.6						
in Bolay, arvoir							
Movement	EBT	EBR	WBL	WBT	NBL	NBR	
/ol, veh/h	233	32	1	24	44	6	
Conflicting Peds, #/hr	0	0	0	0	0	0	
Sign Control	Free	Free	Free	Free	Stop	Stop	
RT Channelized		41/4	-	None		None	
Storage Length			-	-	0		
Veh in Median Storage, #	0		14	0	0		
Grade, %	0	-		0	0	(4)	
Peak Hour Factor	85		85	85	85	85	
Heavy Vehicles, %	0		0	5	0	0	
Mymt Flow	274		1	28	52	7	
AIAIIIT I IOM	214	00	·				
Major/Minor	Major1		Major2		Minor1		
Conflicting Flow All	0	0	312	0	324	293	
Stage 1				121	293		
Stage 2			-	-	31		
Critical Hdwy			4.1		6.4	6.2	
				-	5.4		
Critical Hdwy Stg 1					5.4	- 12-	
Critical Hdwy Stg 2	-		2.2		3.5	3.3	
Follow-up Hdwy			1260		674	751	
Pot Cap-1 Maneuver		1	1200		762	-	
Stage 1				-	997	15	
Stage 2	-			-	991		
Platoon blocked, %	7		4000		070	751	
Mov Cap-1 Maneuver	-	-	1260		673	/51	
Mov Cap-2 Maneuver			-		673		
Stage 1	-		-	(*)	762		
Stage 2		-			996		
A	EB		WB		NB		
Approach			0.3		10.8		
HCM Control Delay, s HCM LOS	C		0.3		В		
Minor Lane/Major Mvmt	NBLn1 EBT	EBR					
Capacity (veh/h)	681		1260 -				
HCM Lane V/C Ratio	0.086		0.001 -				
HCM Control Delay (s)	10.8	ė ie	7.9 0				
HCM Lane LOS	В		A A				
HCM 95th %tile Q(veh)	0.3		0 -				

	-	7	1	←	1		
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	1→			स	3/4		
Volume (vph)	357	38	0	49	79	5	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Satd. Flow (prot)	1673	0	0	1710	1620	0	
FIt Permitted					0.955		
Satd. Flow (perm)	1673	0	0	1710	1620	0	
Link Speed (mph)	30			30	30		
Link Distance (ft)	713			898	694		
Travel Time (s)	16.2			20.4	15.8		
Peak Hour Factor	0.86	0.86	0.86	0.86	0.86	0.86	
Heavy Vehicles (%)	1%	0%	0%	0%	0%	0%	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	459	0	0	57	98	0	
Sign Control	Free			Free	Stop		
Intersection Cummary	10 THE ST	U/O/ NIV	IIIs	THE M	Alle e et	7	

Intersection Summary

Area Type:

CBD

Control Type: Unsignalized

Intersection Capacity Utilization 35.3%

Analysis Period (min) 15

Intersection							
	2.1						
Movement	EBT	EBR	WBL	WBT	NBL	NBR	
Vol, veh/h	357	38	0	49	79	5	
Conflicting Peds, #/hr	0		0	0	0	0	
Sign Control	Free		Free	Free	Stop	Stop	
RT Channelized	4 1.50	None	-	None	+	None	
Storage Length		108000		-	0		
Veh in Median Storage, #	0		-	0	0	-	
Grade, %	0		-	0	0	6	
Peak Hour Factor	86		86	86	86	86	
Heavy Vehicles, %	1	0	0	0	0	0	
Mymt Flow	415		Ō	57	92	6	
particle (1971) 188 188 (1971)	410	77	U				

Major/Minor	Major1	1 - 1	Major2	and the last	Minor1	35-	
Conflicting Flow All	C	0	459	0	494	437	
Stage 1		-	-	4	437		
Stage 2		-		-	57		
Critical Hdwy		-	4.1	+	6.4	6.2	
Critical Hdwy Stg 1			-		5.4	4	
Critical Hdwy Stg 2	14	24	/-	J - 12-	5.4		
Follow-up Hdwy			2.2		3.5	3.3	
Pot Cap-1 Maneuver			1113	-	538	624	
01 4				141	655		
Stage 1 Stage 2				4	971		
Platoon blocked, %							
Mov Cap-1 Maneuver			1113		538	624	
Mov Cap-2 Maneuver					538		
Stage 1				-	655		
Stage 2					971	-	
Stage Z							
Approach	EB		WB	V=	NB		
HCM Control Delay, s	(0		13.1		
HCM LOS			0		В		
LICIAI FOS							
Minor Lane/Major Mvmt	NBLn1 EB7	EBR	WBL WBT		4		
Capacity (veh/h)	542		1113 -				
HCM Lane V/C Ratio	0.18						
HCM Control Delay (s)	13.1		0 -				
HCM Lane LOS	В		Α -				
HCM 95th %tile Q(veh)	0.7		0 -				

	-	7	1	-	1	1	
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	7>			स	3/4		
Volume (vph)	253	34	1	26	47	6	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Satd. Flow (prot)	1683	0	0	1628	1614	0	
Flt Permitted				0.998	0.958		
Satd. Flow (perm)	1683	0	0	1628	1614	0	
Link Speed (mph)	30			30	30		
Link Distance (ft)	713			898	694		
Travel Time (s)	16.2			20.4	15.8		
Peak Hour Factor	0.85	0.85	0.85	0.85	0.85	0.85	
Heavy Vehicles (%)	0%	0%	0%	5%	0%	0%	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	338	0	0	32	62	0	
Sign Control	Free			Free	Stop		

Intersection Summary

Area Type:

CBD

Control Type: Unsignalized

Intersection Capacity Utilization 27.1%

ICU Level of Service A

ntersection							
nt Delay, s/veh 1.6	6						
Movement	EBT	EBR	WBL	WBT	NBL	NBR	
/ol, veh/h	253	34	1	26	47	6	
Conflicting Peds, #/hr	0	0	0	0	0	0	
Sign Control	Free	Free	Free	Free	Stop	Stop	
RT Channelized	1100	None		None	-	None	
Storage Length		-		2	0		
Veh in Median Storage, #	0		-	0	0		
Grade, %	0	-		0	0		
Peak Hour Factor	85	85	85	85	85	85	
	0	0	0	5	0	0	
Heavy Vehicles, %	298	40	1	31	55	7	
Mvmt Flow	230	40		01			
Major/Minor	Major1		Major2		Minor1	0.7%	
Conflicting Flow All	0	0	338	0	351	318	
Stage 1	-		-		318		
Stage 2		-		-	33	-	
_			4.1		6.4	6.2	
Critical Hdwy	-		77.1		5.4	-	
Critical Hdwy Stg 1					5.4	4	
Critical Hdwy Stg 2	-		2.2		3.5	3.3	
Follow-up Hdwy	•		1232		650	727	
Pot Cap-1 Maneuver			1202		742		
Stage 1	-	-	2		995		
Stage 2				-	555		
Platoon blocked, %			4000		649	727	
Mov Cap-1 Maneuver	*		1232		649	121	
Mov Cap-2 Maneuver				•		-	
Stage 1	-		(2		742	-	
Stage 2				*.	994		
X	ED		WB		NB		- 3 317
Approach Delay a	EB 0	r	0.3		11.1		
HCM Control Delay, s	Ü		0.5		В		
HCM LOS					Б		
Minor Lane/Major Mvmt	NBLn1 EBT	EBR	WBL WBT				
	657 -	LU10	1232				
Capacity (veh/h)			0.001				
HCM Lane V/C Ratio	0.095		7.9				
HCM Control Delay (s)	11.1 -						
HCM Lane LOS	В -		A A				
HCM 95th %tile Q(veh)	0.3		0 -				

3: Frankfort Street & Porter Street

	-	*	1	—	1	-	
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	1→			4	A		
Volume (vph)	357	40	0	50	87	5	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Satd. Flow (prot)	1671	0	0	1710	1620	0	
Flt Permitted					0.955		
Satd. Flow (perm)	1671	0	0	1710	1620	0	
Link Speed (mph)	30			30	30		
Link Distance (ft)	713			898	694		
Travel Time (s)	16.2			20.4	15.8		
Peak Hour Factor	0.86	0.86	0.86	0.86	0.86	0.86	
Heavy Vehicles (%)	1%	0%	0%	0%	0%	0%	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	462	0	0	58	107	0	
Sign Control	Free			Free	Stop		
Intersection Summary		40-2	The P	1000		Selle	

CBD

Area Type:
Control Type: Unsignalized

Intersection Capacity Utilization 35.9%

Analysis Period (min) 15

Intersection	U. T. IV	31 YA	150	SHIT I	SAUTA	we TEW			TO US	11-12-11		771
Int Delay, s/veh 2.	.3											
Movement	distance of P	ВТ	EBR	offer t	WBL	WBT	5 5 0 5 B 1 7	NBL	NBR			
Vol, veh/h		357	40		0	50		87	5			18 Y
Conflicting Peds, #/hr		0	0		0	0		0	0			
Sign Control	F	ree	Free		Free	Free		Stop	Stop			
RT Channelized			None			None			None			
Storage Length		(a)	F40					0				
Veh in Median Storage, #		0	120			0		0				
Grade, %		0				0		0	Plan Be			
Peak Hour Factor		86	86		86	86		86	86			
Heavy Vehicles, %		1	0		0	0		0	0			
Mvmt Flow		415	47		0	58		101	6			
Major/Minor	Ma	jor1	1211	N	//ajor2	Davin		Minor1				(A867)
Major/Minor	IVIC	0	0		462	0		496	438			Agr
Conflicting Flow All		U			102	-		438	-			
Stage 1		=027	0.25		100	111, 211		58	,			
Stage 2		-0	72		4.1			6.4	6.2			
Critical Hdwy		255			7.1			5.4				
Critical Hdwy Stg 1								5.4				
Critical Hdwy Stg 2		250	3.5		2.2			3.5	3.3			
Follow-up Hdwy			100		1110	-		537	623			
Pot Cap-1 Maneuver		1100	112		1110			655				
Stage 1		44			-			970				
Stage 2		-	10			ure, juli						
Platoon blocked, %		-			1110	32		537	623			
Mov Cap-1 Maneuver		-			1110	- 2		537				
Mov Cap-2 Maneuver			- 5		12			655				
Stage 1 Stage 2		*						970	11/9			
		and the '			WID			ND	/8/0 ₂ / 1/4 ₂	erifica (I.C.)		2. 3.
Approach		EB	() T	10 1	WB		CONTRACTOR OF THE PARTY OF THE	NB	- 10 (c)	The state of the s		
HCM Control Delay, s HCM LOS		0			0			13.3 B				
Minor Lane/Major Mvmt	NBLn1	EBT	EBR	WBL	WBT	11-12-17		Hh 312		Les Horas	10	
Capacity (veh/h)	541			1110	•							
HCM Lane V/C Ratio	0.198	-		-								
HCM Control Delay (s)	13.3	:+	*	0	-							
HCM Lane LOS	В	-	*	Α	-							
HCM 95th %tile Q(veh)	0.7	:2	¥	0	*							

	→	*	1	-	4	1	
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	1>			4	sha		
Volume (vph)	254	39	1	27	54	6	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Satd. Flow (prot)	1679	0	0	1628	1615	0	
Flt Permitted				0.998	0.957		
Satd. Flow (perm)	1679	0	0	1628	1615	0	
Link Speed (mph)	30			30	30		
Link Distance (ft)	713			898	694		
Travel Time (s)	16.2			20.4	15.8		
Peak Hour Factor	0.85	0.85	0.85	0.85	0.85	0.85	
Heavy Vehicles (%)	0%	0%	0%	5%	0%	0%	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	345	0	0	33	71	0	
Sign Control	Free			Free	Stop		
Intersection Summary						-	

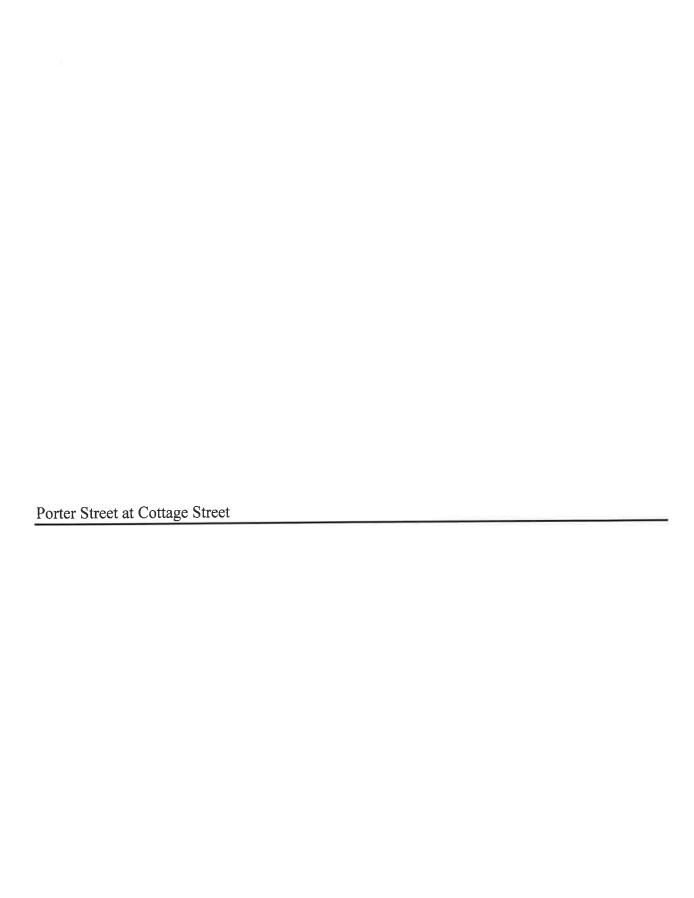
Area Type:

CBD

Control Type: Unsignalized Intersection Capacity Utilization 27.9%

Analysis Period (min) 15

BL NBR 54 6 0 0 op Stop - None 0 - 0 - 0 - 85 85 0 0 64 7
54 6 0 0 0 op Stop - None 0 - 0 - 0 - 85 85 0 0 64 7
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None None None - No
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5.4
3.5 3.3
46 724
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74
45 724
45 -
39
93
NB
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В
000



	→	*	1	4-	4	-
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	7+			4		ř.
Volume (vph)	0	290	0	1	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	15	12	12
Satd. Flow (prot)	1465	0	0	1881	0	1710
Flt Permitted						
Satd. Flow (perm)	1465	0	0	1881	0	1710
Link Speed (mph)	30			30	30	
Link Distance (ft)	898			337	677	
Travel Time (s)	20.4			7.7	15.4	
Peak Hour Factor	0.91	0.91	0.91	0.91	0.91	0.91
Heavy Vehicles (%)	0%	1%	0%	0%	0%	0%
Shared Lane Traffic (%)						
Lane Group Flow (vph)	319	0	0	1	0	0
Sign Control	Free			Free	Stop	
Intersection Summary	3757					
Area Type:	CBD					
Control Type: Unsignalized						
Intersection Capacity Utiliza	ation 23.3%			IC	CU Level	of Service A
Analysis Period (min) 15	÷ 9					

ntersection	7/1-97 9	NI DIE		Mode				"TENEDEZE TENED	
nt Delay, s/veh	0								
Movement		EBT	EBR	78 13	WBL	WBT	NBL	NBR	
Vol, veh/h		0	290		0	1	0	0	
Conflicting Peds, #/hr		0	0		0	0	0	0	
Sign Control		Free	Free		Free	Free	Stop	Stop	
RT Channelized		-	None		-	None	<u>#</u> :	None	
Storage Length								0	
/eh in Median Storage, #		0	-		7.	0	0	-	
Grade, %		0	100		-	0	0		
Peak Hour Factor		91	91		91	91	91	91	
Heavy Vehicles, %		0	1		0	0	0	0	
Mvmt Flow		0	319		0	1	0	0	
Major/Minor	N	lajor1		N	lajor2	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	Minor1	NA ENLEY	PISTY LINE WEST
Conflicting Flow All		0	0		319	0	160	159	
Stage 1		-	-			2	159	-	
Stage 2			-				1	2	
Critical Hdwy		-			4.1	-	6.4	6.2	
Critical Hdwy Stg 1							5.4		
Critical Hdwy Stg 2		-					5.4	-	
Follow-up Hdwy		2			2.2	54	3.5	3.3	
Pot Cap-1 Maneuver			-		1252	2	836	892	
Stage 1			-			-	875		
Stage 2		-	-			-	1028	-	
Platoon blocked, %									
Mov Cap-1 Maneuver		-	:=::		1252		836	892	
Mov Cap-2 Maneuver		-	10 40			-	836		
Stage 1		14	:41				875		
Stage 2			- 4		1.2	-	1028		
Approach	OVER 1	EB	7.10	LVE	WB		NB		allantie in Si
HCM Control Delay, s		0			0		0		
HCM LOS							A		
Minor Lane/Major Mvmt	NBLn1	EBT	EBR	WBL	WBT				
Capacity (veh/h)		36	***	1252	~				
HCM Lane V/C Ratio	-		35	-	-				
HCM Control Delay (s)	0			0	- 2				
HCM Lane LOS	Α	-7		Α	2				
HCM 95th %tile Q(veh)	-	3.5	:*:	0					

4: Cottage Street & Porter Street

	-	*	1	←	1		
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	4			ન		ř	
Volume (vph)	0	208	0	0	0	0	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Lane Width (ft)	12	12	12	15	12	12	
Satd. Flow (prot)	1479	0	0	1881	0	1710	
Flt Permitted							
Satd. Flow (perm)	1479	0	0	1881	0	1710	
Link Speed (mph)	30			30	30		
Link Distance (ft)	898			337	677		
Travel Time (s)	20.4			7.7	15.4		
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93	
Heavy Vehicles (%)	0%	0%	0%	0%	0%	0%	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	224	0	0	0	0	0	
Sign Control	Free			Free	Stop		

Intersection Summary

Area Type:

CBD

Control Type: Unsignalized

Intersection Capacity Utilization 17.6%

Analysis Period (min) 15

Intersection							
nt Delay, s/veh	0						
Movement	EBT	EBR	WBL	WBT	NBL	NBR	EST 33
Vol, veh/h	0	208	0	0	0	0	
Conflicting Peds, #/hr	0	0	0	0	0	0	
Sign Control	Free	Free	Free	Free	Stop	Stop	
RT Channelized		None	1.40	None	-	None	
Storage Length	-	_	-	-		0	
Veh in Median Storage, #	0	-		0	0		
Grade, %	0	-	14	0	0	-	
Peak Hour Factor	93	93	93	93	93	93	
Heavy Vehicles, %	0	0	0	0	0	0	
Mymt Flow	0	224	0	0	0	0	
			7,000,000				
Major/Minor	Major1		Major2		Minor1	-wilking	
Conflicting Flow All	0	0	224	0	112	112	
Stage 1				-	112		
Stage 2			-		0		
Critical Hdwy		-	4.1	-	6.4	6.2	
Critical Hdwy Stg 1			-		5.4		
Critical Hdwy Stg 2			-		5.4	*	
Follow-up Hdwy		2	2.2		3.5	3.3	
Pot Cap-1 Maneuver			1357	-	890	947	
Stage 1			-	-	918		
Stage 2	_				-		
Platoon blocked, %							
Mov Cap-1 Maneuver			1357	-	890	947	
			1007		890		
Mov Cap-2 Maneuver			_	-	918		
Stage 1 Stage 2				4	-		
Stage 2							
Approach	EB	VI	WB		NB		
HCM Control Delay, s	0		0		0		
HCM LOS					A		
Minor Lang (Marior Marret	NBLn1 EBT	EBR	WBL WBT				
Minor Lane/Major Mvmt	INDLIII EDI	LDI					
Capacity (veh/h)		-	1357				
HCM Lane V/C Ratio			-				
HCM Control Delay (s)	0 -		0 -				
HCM Lane LOS	Α -		Α -				
HCM 95th %tile Q(veh)	-		0 -				

4: Cottage Street & Porter Street

	-	7	1	-	1	1	
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR	AND THE STATE OF THE STATE OF THE
Lane Configurations	7+			र्स		ř	
Volume (vph)	0	312	0	1	0	0	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Lane Width (ft)	12	12	12	15	12	12	
Satd. Flow (prot)	1465	0	0	1881	0	1710	
Flt Permitted							
Satd. Flow (perm)	1465	0	0	1881	0	1710	
Link Speed (mph)	30			30	30		
Link Distance (ft)	898			337	677		
Travel Time (s)	20.4			7.7	15.4		
Peak Hour Factor	0.91	0.91	0.91	0.91	0.91	0.91	
Heavy Vehicles (%)	0%	1%	0%	0%	0%	0%	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	343	0	0	1	0	0	
Sign Control	Free			Free	Stop		
Intersection Summary		31 177	W = 119		F		

Area Type:

CBD

Control Type: Unsignalized Intersection Capacity Utilization 24.8%

ICU Level of Service A

ntersection							
nt Delay, s/veh	0						
Movement	EBT	EBR	WBL	WBT	NBL	NBR	
Vol, veh/h	0	312	0		0	0	
Conflicting Peds, #/hr	0	0	0	0	0	0	
Sign Control	Free	Free	Free	Free	Stop	Stop	
RT Channelized		None		None	ů.	None	
Storage Length		-	14			0	
Veh in Median Storage, #	0	-		0	0	-	
Grade, %	0	2		0	0		
Peak Hour Factor	91	91	91	91	91	91	
Heavy Vehicles, %	0	1	0	0	0	0	
Mvmt Flow	0	343	0		0	0	
VI	Malad		Major		Minor1		
Major/Minor	Major1		Major2		172	171	
Conflicting Flow All	0	0	343	0	172	401	
Stage 1			•		1/1		
Stage 2						6.2	
Critical Hdwy		-	4.1	*	6.4	0.2	
Critical Hdwy Stg 1	-	-	-	*	5.4		
Critical Hdwy Stg 2		-	-	*	5.4		
Follow-up Hdwy			2.2		3.5	3.3	
Pot Cap-1 Maneuver		-	1227		823	878	
Stage 1			18		864	-	
Stage 2		-	-		1028	1.6	
Platoon blocked, %					444		
Mov Cap-1 Maneuver		-	1227		823	878	
Mov Cap-2 Maneuver	-	-	19		823	*	
Stage 1	+	-	-	191	864		
Stage 2				-	1028	4	
Approach	EB		WB	(/	NB		
HCM Control Delay, s	0		C		0		
HCM LOS	· ·				А		
Minor Lane/Major Mvmt	NBLn1 EBT	EBR	WBL WBT				
Capacity (veh/h)		12	1227				
HCM Lane V/C Ratio							
HCM Control Delay (s)	0 -		0 -				
HCM Lane LOS	Å -		Α -				
HCM 95th %tile Q(veh)	• •		0 -				

	-	*	1	4	1		
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	7			4		74	
Volume (vph)	0	226	0	0	0	0	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Lane Width (ft)	12	12	12	15	12	12	
Satd. Flow (prot)	1479	0	0	1881	0	1710	
FIt Permitted							
Satd. Flow (perm)	1479	0	0	1881	0	1710	
Link Speed (mph)	30			30	30		
Link Distance (ft)	898			337	677		
Travel Time (s)	20.4			7.7	15.4		
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93	
Heavy Vehicles (%)	0%	0%	0%	0%	0%	0%	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	243	0	0	0	0	0	
Sign Control	Free			Free	Stop		

Area Type: CBD

Control Type: Unsignalized

Intersection Capacity Utilization 18.9%

Analysis Period (min) 15

ntersection				70			
nt Delay, s/veh)						
Movement	EBT	EBR	WBL	WBT	NBL	NBR	
Vol, veh/h	0	226	0	0	0	0	
Conflicting Peds, #/hr	0	0	0	0	0	0	
Sign Control	Free	Free	Free	Free	Stop	Stop	
RT Channelized	1100	None	,,,,,	None	+	None	
		110110	- 1.	-		0	
Storage Length	0			0	0	-	
Veh in Median Storage, #	0			0	0	- 6	
Grade, %	93	93	93	93	93	93	
Peak Hour Factor		0	0	0	0	0	
Heavy Vehicles, %	0		0	0	ő	0	
Mvmt Flow	0	243	U	U		O .	
Major/Minor	Major1		Major2		Minor1		
Conflicting Flow All	0	0	243	0	122	122	
Stage 1	-			-	122	*	
Stage 2					0		
Critical Hdwy			4.1		6.4	6.2	
	12		-		5.4	_	
Critical Hdwy Stg 1	-			-	5.4	-	
Critical Hdwy Stg 2	-		2.2		3.5	3.3	
Follow-up Hdwy	-		1335		878	935	
Pot Cap-1 Maneuver	-		1333	-	908	-	
Stage 1	5			-	300	_	
Stage 2		•	1.7	-	~		
Platoon blocked, %	*		1005		878	935	
Mov Cap-1 Maneuver			1335			930	
Mov Cap-2 Maneuver	-	*		*	878	-	
Stage 1	+				908		
Stage 2					-		
Approach	EB		WB	į.	NB		
HCM Control Delay, s	0		0		0		
HCM LOS	0				A		
NA - 1 - NA - 1 - 1	MDI 44 FDT	EDD	WBL WBT				
Minor Lane/Major Mvmt	NBLn1 EBT	EBR					
Capacity (veh/h)		-	1335 -				
HCM Lane V/C Ratio	-						
HCM Control Delay (s)	0 -		0 -				
HCM Lane LOS	Α -		Α -				
HCM 95th %tile Q(veh)	- 105		0 -				

	-	*	1	←	4	1	
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	1>			4		74	
Volume (vph)	0	312	0	1	0	0	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Lane Width (ft)	12	12	12	15	12	12	
Satd. Flow (prot)	1465	0	0	1881	0	1710	
Flt Permitted							
Satd. Flow (perm)	1465	0	0	1881	0	1710	
Link Speed (mph)	30			30	30		
Link Distance (ft)	898			337	677		
Travel Time (s)	20.4			7.7	15.4		
Peak Hour Factor	0.91	0.91	0.91	0.91	0.91	0.91	
Heavy Vehicles (%)	0%	1%	0%	0%	0%	0%	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	343	0	0	1	0	0	
Sign Control	Free			Free	Stop		
Intersection Summary	Mary Said	MINISTRA	No. of the last	THE PARTY	a The	10/11/200	
	CBD	V 12 2 14	1. 11.10				
Control Type: Unsignalized							
Intersection Capacity Utiliza	tion 24.8%			IC	U Level	of Service A	A
Analysis Period (min) 15		- III. ∰	8	118 12			

ntersection	Kran Jan	(C)			2274	4 48 V F	AND ESTIMATE		
nt Delay, s/veh	0								
Movement		EBT	EBR		WBL	WBT	NBL	NBR	
/ol, veh/h		0	312		0	1	0	0	
Conflicting Peds, #/hr		0	0		0	0	0	0	
Sign Control	F	Free	Free		Free	Free	Stop	Stop	
RT Channelized		-	None		:=:	None		None	
Storage Length		-	-					0	
eh in Median Storage, #		0	247		(#)	0	0	553	
Grade, %		0	74			0	0		
eak Hour Factor		91	91		91	91	91	91	
leavy Vehicles, %		0	1		0	0	0	0	
Nymt Flow		0	343		0	1	0	0	
Major/Minor	Ma	ajor1		N	Najor2	A. 2005.	Minor1		
Conflicting Flow All	1410	0	0		343	0	172	171	
Stage 1		_	-		-	-	171	-	
Stage 2		10				100	1		
Critical Hdwy					4.1	741	6.4	6.2	
Critical Hdwy Stg 1					2		5.4		
Critical Hdwy Stg 2			-				5.4	· ·	
follow-up Hdwy					2.2		3.5	3.3	
Pot Cap-1 Maneuver		-			1227	-	823	878	
Stage 1		2			122		864	quite a	
			A 2				1028		
Stage 2 Platoon blocked, %		2							
Mov Cap-1 Maneuver		- 8	9		1227	2	823	878	
		- 8			1221		823		
Mov Cap-2 Maneuver					-		864	840	
Stage 1		- 5			77.		1028	-	
Stage 2					п		1020		
pproach		EB	CESSE	HYLLY	WB		NB		of the strain of the strain
HCM Control Delay, s HCM LOS		0			0		0 A		
	100	CDT	FDD	WDI	MOT		n 581 201	Marine Her	State of the same
/linor Lane/Major Mvmt	NBLn1	EBT	EBR	WBL	WBT				
Capacity (veh/h)	. 	•	*	1227					
HCM Lane V/C Ratio	-	*		•	*				
HCM Control Delay (s)	0	*	*	0	÷				
HCM Lane LOS	Α		*	Α					
HCM 95th %tile Q(veh)		12	-	0	9				

	→	•	6	←	4	-	
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR	T Wil
Lane Configurations	1>	N. S.		4		۳	
Volume (vph)	0	226	0	0	0	0	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Lane Width (ft)	12	12	12	15	12	12	
Satd. Flow (prot)	1479	0	0	1881	0	1710	
Flt Permitted							
Satd. Flow (perm)	1479	0	0	1881	0	1710	
Link Speed (mph)	30			30	30		
Link Distance (ft)	898			337	677		
Travel Time (s)	20.4			7.7	15.4		
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93	
Heavy Vehicles (%)	0%	0%	0%	0%	0%	0%	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	243	0	0	0	0	0	
Sign Control	Free			Free	Stop		
Intersection Summary	e Tilly	CIVIS U				HURE V	STATE
Area Type:	CBD						
Control Type: Unsignalized							
Intersection Capacity Utiliza	ation 18.9%			IC	CU Level	of Service	e A
Analysis Period (min) 15							

ntersection	0,17 107	1			4/			Year To	Zielije)	TO THE OTHER	4
nt Delay, s/veh	0										
Novement		EBT	EBR	V	NBL	WBT	or Supply/18	NBL	NBR	1808340	
ol, veh/h		0	226		0	0	11/2/15	0	0		
Conflicting Peds, #/hr		0	0		0	0		0	0		
Sign Control	F	ree	Free	F	ree	Free		Stop	Stop		
T Channelized		520	None		*	None		:=	None		
torage Length		G.	-		164						
eh in Median Storage, #		0	240		:80	0		0			
Grade, %		0				0		0	8 L		
eak Hour Factor		93	93		93	93		93	93		
leavy Vehicles, %		0	0		0	0		0	0		
Nymt Flow		0	243		0	0		0	(
Major/Minor	Ma	jor1	level.	Ma	ajor2	WA ES IN	М	inor1			
Conflicting Flow All	100	0	0		243	0	THE VI	122	122		
Stage 1		-				-		122			
Stage 2		-						0	1 ·	ا الرجاء	
Critical Hdwy		000	123		4.1	121		6.4	6.2		
Critical Hdwy Stg 1		722			114			5.4			
		1175	-					5.4	-		
Critical Hdwy Stg 2		120			2.2	1/2		3.5	3.3		
ollow-up Hdwy			1421		1335	:*:		878	935		
Pot Cap-1 Maneuver		7.	1693		-			908			
Stage 1					-			-			
Stage 2											
Platoon blocked, %			180		1335			878	935		
Mov Cap-1 Maneuver		70	9.5		1333			878	300	- V - I	
Nov Cap-2 Maneuver		*	05		15	· · · · ·		908			
Stage 1		-	(6)		3.50	: ::::::::::::::::::::::::::::::::::::		300			
Stage 2		•						0 15.			
Approach	La La Exche	EB	RIFT	6-2-	WB			NB			- !!!
HCM Control Delay, s		0			0			0			
HCM LOS								Α			
Minor Lane/Major Mvmt	NBLn1	EBT	EBR	WBL \	WBT	L TOWN			910		- 2
Capacity (veh/h)	*	8		1335							
HCM Lane V/C Ratio	-	9	*	-	5.						
HCM Control Delay (s)	0	2	¥	0	*						
HCM Lane LOS	A	2		Α	-						
HCM 95th %tile Q(veh)		9	2	0	2						

	•	*	4	†	+	1	
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR	
Lane Configurations	1qf			4	7+		
Volume (vph)	48	42	5	154	156	15	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Lane Width (ft)	13	12	12	12	12	12	
Satd. Flow (prot)	1590	0	0	1690	1659	0	
FIt Permitted	0.974			0.998			
Satd. Flow (perm)	1590	0	0	1690	1659	0	
Link Speed (mph)	30			30	30		
Link Distance (ft)	292			750	716		
Travel Time (s)	6.6			17.0	16.3		
Peak Hour Factor	0.87	0.87	0.87	0.87	0.87	0.87	
Heavy Vehicles (%)	0%	3%	0%	1%	2%	0%	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	103	0	0	183	196	0	
Sign Control	Stop			Stop	Stop		
Intersection Summary							
Area Type:	CBD						
Control Type: Unsignalize	ed						
Intersection Capacity Utili	zation 26.0%			IC	CU Level	of Service A	
Analysis Period (min) 15							

Intersection										
Intersection Delay, s/veh	8.6									
Intersection LOS	Α									
Movement	EBU	EBL		EBR	NBU	NBL	NBT	SBU	SBT	SBR
Vol, veh/h	0	48		42	0	5	154	0	156	15
Peak Hour Factor	0.92	0.87		0.87	0.92	0.87	0.87	0.92	0.87	0.87
Heavy Vehicles, %	2	0		3	2	0	1	2	2	0
Mymt Flow	0	55		48	0	6	177	0	179	17
Number of Lanes	0	1		0	0	0	1	0	1	0
Approach		EB				NB			SB	
Opposing Approach						SB			NB	
Opposing Lanes		0				1			1	
Conflicting Approach Left		SB				EB				
Conflicting Lanes Left		1				1			.0	
Conflicting Approach Right		NB							EB	
Conflicting Lanes Right		1				0			1	
HCM Control Delay		8.3				8.6			8.7	
HCM LOS		A				Α			A	
Lane		NBLn1	EBLn1	SBLn1						
Vol Left, %		3%	53%	0%						
VOI LOIL, /0										
Vol Thru, %		97%	0%	91%						
		97% 0%	0% 47%	91% 9%						
Vol Thru, %		97% 0% Stop	0% 47% Stop	91% 9% Stop						
Vol Thru, % Vol Right, %		97% 0% Stop 159	0% 47% Stop 90	91% 9% Stop 171						
Vol Thru, % Vol Right, % Sign Control		97% 0% Stop 159 5	0% 47% Stop 90 48	91% 9% Stop 171 0						
Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane		97% 0% Stop 159 5 154	0% 47% Stop 90 48	91% 9% Stop 171 0						
Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol		97% 0% Stop 159 5 154 0	0% 47% Stop 90 48 0 42	91% 9% Stop 171 0 156 15						
Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol		97% 0% Stop 159 5 154	0% 47% Stop 90 48 0 42 103	91% 9% Stop 171 0 156 15						
Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol		97% 0% Stop 159 5 154 0 183	0% 47% Stop 90 48 0 42 103	91% 9% Stop 171 0 156 15 197						
Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate		97% 0% Stop 159 5 154 0 183 1 0.221	0% 47% Stop 90 48 0 42 103 1	91% 9% Stop 171 0 156 15 197 1						
Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp		97% 0% Stop 159 5 154 0 183 1 0.221 4.357	0% 47% Stop 90 48 0 42 103 1 0.131 4.561	91% 9% Stop 171 0 156 15 197 1 0.236 4.318						
Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X)		97% 0% Stop 159 5 154 0 183 1 0.221 4.357 Yes	0% 47% Stop 90 48 0 42 103 1 0.131 4.561 Yes	91% 9% Stop 171 0 156 15 197 1 0.236 4.318 Yes						
Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N		97% 0% Stop 159 5 154 0 183 1 0.221 4.357 Yes 826	0% 47% Stop 90 48 0 42 103 1 0.131 4.561 Yes 787	91% 9% Stop 171 0 156 15 197 1 0.236 4.318 Yes 833						
Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd)		97% 0% Stop 159 5 154 0 183 1 0.221 4.357 Yes 826 2.374	0% 47% Stop 90 48 0 42 103 1 0.131 4.561 Yes 787 2.582	91% 9% Stop 171 0 156 15 197 1 0.236 4.318 Yes 833 2.336						
Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap		97% 0% Stop 159 5 154 0 183 1 0.221 4.357 Yes 826	0% 47% Stop 90 48 0 42 103 1 0.131 4.561 Yes 787 2.582 0.131	91% 9% Stop 171 0 156 15 197 1 0.236 4.318 Yes 833 2.336 0.236						
Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time		97% 0% Stop 159 5 154 0 183 1 0.221 4.357 Yes 826 2.374	0% 47% Stop 90 48 0 42 103 1 0.131 4.561 Yes 787 2.582	91% 9% Stop 171 0 156 15 197 1 0.236 4.318 Yes 833 2.336 0.236 8.7						
Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time HCM Lane V/C Ratio		97% 0% Stop 159 5 154 0 183 1 0.221 4.357 Yes 826 2.374 0.222	0% 47% Stop 90 48 0 42 103 1 0.131 4.561 Yes 787 2.582 0.131	91% 9% Stop 171 0 156 15 197 1 0.236 4.318 Yes 833 2.336 0.236						



	٦	*	1	†	ļ	1	
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR	
Lane Configurations	74			4	4		
Volume (vph)	41	39	4	180	139	2	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Lane Width (ft)	13	12	12	12	12	12	
Satd. Flow (prot)	1586	0	0	1708	1690	0	
Fit Permitted	0.975			0.999			
Satd. Flow (perm)	1586	0	0	1708	1690	0	
Link Speed (mph)	30			30	30		
Link Distance (ft)	292			750	716		
Travel Time (s)	6.6			17.0	16.3		
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	
Heavy Vehicles (%)	3%	0%	0%	0%	1%	0%	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	89	0	0	204	156	0	
Sign Control	Stop			Stop	Stop		
Intersection Summary	S 15 1			11 x24 1	THE		
Area Type:	CBD						
Control Type: Unsignalized	d						
Intersection Capacity Utiliz				10	CU Level	of Service A	

Intersection										
Intersection Delay, s/veh	8.4 A									
Movement	EBU	EBL		EBR	NBU	NBL	NBT	SBU	SBT	SBF
Vol, veh/h	0	41		39	0	4	180	0	139	2
Peak Hour Factor	0.92	0.90		0.90	0.92	0.90	0.90	0.92	0.90	0.90
Heavy Vehicles, %	2	3		0	2	0	0	2	1	(
Mvmt Flow	0	46		43	0	4	200	0	154	:
Number of Lanes	0	1		0	0	0	1	0	1	(
Approach	-	EB				NB			SB	
Opposing Approach						SB			NB	
Opposing Lanes		0				1			1	
Conflicting Approach Left		SB				EB				
Conflicting Lanes Left		1				1			0	
Conflicting Approach Right		NB							EB	
Conflicting Lanes Right		1				0			1	
HCM Control Delay		8.1				8.6			8.3	
HCM LOS		Α				Α			Α	
HCM LOS			EBLn1	SBLn1		A			A	
HCM LOS		NBLn1	EBLn1 51%	SBLn1		A			Α	
Lane Vol Left, %		NBLn1	51%	0%		A			A	
Lane Vol Left, % Vol Thru, %		NBLn1 2% 98%	51% 0%	0% 99%		A			A .	
Lane Vol Left, % Vol Thru, % Vol Right, %		NBLn1 2% 98% 0%	51% 0% 49%	0% 99% 1%		A			A	
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control		NBLn1 2% 98% 0% Stop	51% 0% 49% Stop	0% 99% 1% Stop		A			A	
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane		NBLn1 2% 98% 0% Stop 184	51% 0% 49%	0% 99% 1%		A			A	
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol		NBLn1 2% 98% 0% Stop 184 4	51% 0% 49% Stop 80	0% 99% 1% Stop 141		A			A	
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol		NBLn1 2% 98% 0% Stop 184	51% 0% 49% Stop 80 41	0% 99% 1% Stop 141		A			A	
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol		NBLn1 2% 98% 0% Stop 184 4 180 0	51% 0% 49% Stop 80 41 0	0% 99% 1% Stop 141 0		A			A	
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate		NBLn1 2% 98% 0% Stop 184 4 180	51% 0% 49% Stop 80 41 0	0% 99% 1% Stop 141 0 139		A			A	
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp		NBLn1 2% 98% 0% Stop 184 4 180 0 204	51% 0% 49% Stop 80 41 0 39 89	0% 99% 1% Stop 141 0 139 2		A			A	
HCM LOS Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X)		NBLn1 2% 98% 0% Stop 184 4 180 0 204	51% 0% 49% Stop 80 41 0 39	0% 99% 1% Stop 141 0 139 2 157		A			A	
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd)		NBLn1 2% 98% 0% Stop 184 4 180 0 204 1 0.237 4.291	51% 0% 49% Stop 80 41 0 39 89 1	0% 99% 1% Stop 141 0 139 2 157 1		A			A	
HCM LOS Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N		NBLn1 2% 98% 0% Stop 184 4 180 0 204 1 0.237	51% 0% 49% Stop 80 41 0 39 89 1 0.112 4.553	0% 99% 1% Stop 141 0 139 2 157 1 0.188 4.33		A			A	
HCM LOS Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap		NBLn1 2% 98% 0% Stop 184 4 180 0 204 1 0.237 4.291 Yes	51% 0% 49% Stop 80 41 0 39 89 1 0.112 4.553 Yes	0% 99% 1% Stop 141 0 139 2 157 1 0.188 4.33 Yes		A			A	
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time		NBLn1 2% 98% 0% Stop 184 4 180 0 204 1 0.237 4.291 Yes 842	51% 0% 49% Stop 80 41 0 39 89 1 0.112 4.553 Yes 791	0% 99% 1% Stop 141 0 139 2 157 1 0.188 4.33 Yes 832		A			A	
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time HCM Lane V/C Ratio		NBLn1 2% 98% 0% Stop 184 4 180 0 204 1 0.237 4.291 Yes 842 2.291	51% 0% 49% Stop 80 41 0 39 89 1 0.112 4.553 Yes 791 2.56	0% 99% 1% Stop 141 0 139 2 157 1 0.188 4.33 Yes 832 2.338		A			A	
HCM LOS Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time		NBLn1 2% 98% 0% Stop 184 4 180 0 204 1 0.237 4.291 Yes 842 2.291 0.242	51% 0% 49% Stop 80 41 0 39 89 1 0.112 4.553 Yes 791 2.56 0.113	0% 99% 1% Stop 141 0 139 2 157 1 0.188 4.33 Yes 832 2.338 0.189		A			A	

	*	*	1	†	1	1	
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR	
Lane Configurations	N/			4	1>		
Volume (vph)	51	45	5	176	175	16	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Lane Width (ft)	13	12	12	12	12	12	
Satd. Flow (prot)	1590	0	0	1692	1661	0	
Flt Permitted	0.974			0.999			
Satd. Flow (perm)	1590	0	0	1692	1661	0	
Link Speed (mph)	30			30	30		
Link Distance (ft)	292			750	716		
Travel Time (s)	6.6			17.0	16.3		
Peak Hour Factor	0.87	0.87	0.87	0.87	0.87	0.87	
Heavy Vehicles (%)	0%	3%	0%	1%	2%	0%	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	111	0	0	208	219	0	
Sign Control	Stop			Stop	Stop		
Intersection Summary							

Area Type:

CBD

Control Type: Unsignalized

Intersection Capacity Utilization 27.6%

ICU Level of Service A

Intersection										
Intersection Delay, s/veh	8.9									
Intersection LOS	A									
Movement	EBU	EBL		EBR	NBU	NBL	NBT	SBU	SBT	SBR
Vol, veh/h	0	51		45	0	5	176	0	175	16
Peak Hour Factor	0.92	0.87		0.87	0.92	0.87	0.87	0.92	0.87	0.87
Heavy Vehicles, %	2	0		3	2	0	1	2	2	(
Mymt Flow	0	59		52	0	6	202	0	201	18
Number of Lanes	0	1		0	0	0	1	0	1	.(
Approach		EB				NB			SB	
Opposing Approach						SB			NB	
Opposing Lanes		0				1			1	
Conflicting Approach Left		SB				EB				
Conflicting Lanes Left		1				1			0	
Conflicting Approach Right		NB							EB	
Conflicting Lanes Right		1				0			1	
HCM Control Delay		8.5				8.9			9	
						۸			A	
HCM LOS		Α				Α				
			EBLn1	SBLn1		A				
Lane		NBLn1	EBLn1	SBLn1		A				
Lane Vol Left, %		NBLn1	53%	0%		A				
Lane Vol Left, % Vol Thru, %		NBLn1 3% 97%	53% 0%	0% 92%		A				
Lane Vol Left, % Vol Thru, % Vol Right, %		NBLn1 3% 97% 0%	53% 0% 47%	0% 92% 8%		A				
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control		NBLn1 3% 97% 0% Stop	53% 0% 47% Stop	0% 92% 8% Stop		A				
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane		NBLn1 3% 97% 0% Stop 181	53% 0% 47%	0% 92% 8%		A				
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol		NBLn1 3% 97% 0% Stop 181 5	53% 0% 47% Stop 96	0% 92% 8% Stop 191		A				
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol		NBLn1 3% 97% 0% Stop 181	53% 0% 47% Stop 96 51	0% 92% 8% Stop 191		A				
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol		NBLn1 3% 97% 0% Stop 181 5 176 0	53% 0% 47% Stop 96 51 0	0% 92% 8% Stop 191 0		A				
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate		NBLn1 3% 97% 0% Stop 181 5 176	53% 0% 47% Stop 96 51 0 45	0% 92% 8% Stop 191 0 175		A				
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp		NBLn1 3% 97% 0% Stop 181 5 176 0 208	53% 0% 47% Stop 96 51 0 45	0% 92% 8% Stop 191 0 175 16 220		A				
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X)		NBLn1 3% 97% 0% Stop 181 5 176 0 208 1	53% 0% 47% Stop 96 51 0 45 110	0% 92% 8% Stop 191 0 175 16 220		A				
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd)		NBLn1 3% 97% 0% Stop 181 5 176 0 208 1 0.255	53% 0% 47% Stop 96 51 0 45 110 1	0% 92% 8% Stop 191 0 175 16 220 1		A				
Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N		NBLn1 3% 97% 0% Stop 181 5 176 0 208 1 0.255 4.404	53% 0% 47% Stop 96 51 0 45 110 1 0.143 4.668	0% 92% 8% Stop 191 0 175 16 220 1 0.267 4.372		A				
Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap		NBLn1 3% 97% 0% Stop 181 5 176 0 208 1 0.255 4.404 Yes	53% 0% 47% Stop 96 51 0 45 110 1 0.143 4.668 Yes	0% 92% 8% Stop 191 0 175 16 220 1 0.267 4.372 Yes		A				
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time		NBLn1 3% 97% 0% Stop 181 5 176 0 208 1 0.255 4.404 Yes 816	53% 0% 47% Stop 96 51 0 45 110 1 0.143 4.668 Yes 768	0% 92% 8% Stop 191 0 175 16 220 1 0.267 4.372 Yes 823		A				
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time HCM Lane V/C Ratio		NBLn1 3% 97% 0% Stop 181 5 176 0 208 1 0.255 4.404 Yes 816 2.426	53% 0% 47% Stop 96 51 0 45 110 1 0.143 4.668 Yes 768 2.697	0% 92% 8% Stop 191 0 175 16 220 1 0.267 4.372 Yes 823 2.394		A				
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time		NBLn1 3% 97% 0% Stop 181 5 176 0 208 1 0.255 4.404 Yes 816 2.426 0.255	53% 0% 47% Stop 96 51 0 45 110 1 0.143 4.668 Yes 768 2.697 0.143	0% 92% 8% Stop 191 0 175 16 220 1 0.267 4.372 Yes 823 2.394 0.267		A				

	٠	*	4	†		1		
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR	11,111,5 1,-1	
Lane Configurations	A			स	1>			
Volume (vph)	44	42	4	206	157	2		
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900		
Lane Width (ft)	13	12	12	12	12	12		
Satd. Flow (prot)	1585	0	0	1708	1690	0		
FIt Permitted	0.975			0.999				
Satd. Flow (perm)	1585	0	0	1708	1690	0		
Link Speed (mph)	30			30	30			
Link Distance (ft)	292			750	716			
Travel Time (s)	6.6			17.0	16.3			
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90		
Heavy Vehicles (%)	3%	0%	0%	0%	1%	0%		
Shared Lane Traffic (%)								
Lane Group Flow (vph)	96	0	0	233	176	0		
Sign Control	Stop			Stop	Stop			
Intersection Summary	1 8 3 W							
Area Type:	CBD							
Control Type: Unsignalized								
Intersection Capacity Utiliza	ation 27.8%			IC	CU Level	of Service A		
Analysis Period (min) 15								

Intersection										
Intersection Delay, s/veh	8.7									
Intersection LOS	Α									
Movement	EBU	EBL		EBR	NBU	NBL	NBT	SBU	SBT	SBR
Vol, veh/h	0	44		42	0	4	206	0	157	2
Peak Hour Factor	0.92	0.90		0.90	0.92	0.90	0.90	0.92	0.90	0.90
Heavy Vehicles, %	2	3		0	2	0	0	2	1	(
Mvmt Flow	0	49		47	0	4	229	0	174	2
Number of Lanes	0	1		0	0	0	1	0	1	C
Approach	7	EB	5-7-13			NB			SB	. 3
Opposing Approach						SB			NB	
Opposing Lanes		0				1			1	
Conflicting Approach Left		SB				EB				
Conflicting Lanes Left		1				1			0	
Conflicting Approach Right		NB							EB	
Conflicting Lanes Right		1				0			1	
HCM Control Delay		8.3				9			8.6	
HCM LOS		Α				Α			Α	
				OD) 4						
Lane	N	IBLn1	EBLn1	SBLn1						
Vol Left, %		2%	51%	0%						
Vol Thru, %		98%	0%	99%						
			4004	4.07						
Vol Right, %		0%	49%	1%						
Vol Right, % Sign Control		Stop	Stop	Stop						
Vol Right, % Sign Control Traffic Vol by Lane		Stop 210	Stop 86	Stop 159						
Vol Right, % Sign Control Traffic Vol by Lane LT Vol		Stop 210 4	Stop 86 44	Stop 159 0						
Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol		Stop 210 4 206	Stop 86 44 0	Stop 159 0 157						
Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol		Stop 210 4 206 0	86 44 0 42	Stop 159 0 157 2						
Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate		Stop 210 4 206 0 233	86 44 0 42 96	Stop 159 0 157 2 177						
Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp		Stop 210 4 206 0 233 1	86 44 0 42 96	Stop 159 0 157 2 177						
Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X)		Stop 210 4 206 0 233 1 0.28	86 44 0 42 96 1 0.124	Stop 159 0 157 2 177 1 0.215						
Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd)		Stop 210 4 206 0 233 1 0.28 4.321	86 44 0 42 96 1 0.124 4.662	Stop 159 0 157 2 177 1 0.215 4.383						
Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N		Stop 210 4 206 0 233 1 0.28 4.321 Yes	86 44 0 42 96 1 0.124 4.662 Yes	Stop 159 0 157 2 177 1 0.215 4.383 Yes						
Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap		Stop 210 4 206 0 233 1 0.28 4.321 Yes 833	86 44 0 42 96 1 0.124 4.662 Yes 770	Stop 159 0 157 2 177 1 0.215 4.383 Yes 820						
Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time		Stop 210 4 206 0 233 1 0.28 4.321 Yes 833 2.339	86 44 0 42 96 1 0.124 4.662 Yes 770 2.685	Stop 159 0 157 2 177 1 0.215 4.383 Yes 820 2.402						
Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time HCM Lane V/C Ratio		Stop 210 4 206 0 233 1 0.28 4.321 Yes 833 2.339 0.28	86 44 0 42 96 1 0.124 4.662 Yes 770 2.685 0.125	Stop 159 0 157 2 177 1 0.215 4.383 Yes 820 2.402 0.216						
Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time HCM Lane V/C Ratio HCM Control Delay		Stop 210 4 206 0 233 1 0.28 4.321 Yes 833 2.339 0.28 9	86 44 0 42 96 1 0.124 4.662 Yes 770 2.685 0.125 8.3	Stop 159 0 157 2 177 1 0.215 4.383 Yes 820 2.402 0.216 8.6						
Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time HCM Lane V/C Ratio		Stop 210 4 206 0 233 1 0.28 4.321 Yes 833 2.339 0.28	86 44 0 42 96 1 0.124 4.662 Yes 770 2.685 0.125	Stop 159 0 157 2 177 1 0.215 4.383 Yes 820 2.402 0.216						

	*	*	4	†	1	1			
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR			
Lane Configurations	N.			4	1→				
Volume (vph)	51	45	5	176	175	16			
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900			
Lane Width (ft)	13	12	12	12	12	12			
Satd. Flow (prot)	1590	0	0	1692	1661	0			
Flt Permitted	0.974			0.999					
Satd. Flow (perm)	1590	0	0	1692	1661	0			
Link Speed (mph)	30			30	30				
Link Distance (ft)	292			750	716				
Travel Time (s)	6.6			17.0	16.3				
Peak Hour Factor	0.87	0.87	0.87	0.87	0.87	0.87			
Heavy Vehicles (%)	0%	3%	0%	1%	2%	0%			
Shared Lane Traffic (%)									
Lane Group Flow (vph)	111	0	0	208	219	0			
Sign Control	Stop			Stop	Stop				
Intersection Summary	281,210,1	S PERKS	The state of	B 83 - 1	18 10		10 10 10 10 10 10 10 10 10 10 10 10 10 1	CHEST MA	
Area Type:	CBD								
Control Type: Unsignalized									
Intersection Capacity Utiliza	ation 27.6%			IC	U Level o	of Service A			
Analysis Period (min) 15									

Intersection									
Intersection Delay, s/veh Intersection LOS	8.9 A								
Movement	EBU EBL		EBR	NBU	NBL	NBT	SBU	SBT	SBR
Vol, veh/h	0 51		45	0	5	176	0	175	16
Peak Hour Factor	0.92 0.87		0.87	0.92	0.87	0.87	0.92	0.87	0.87
Heavy Vehicles, %	2 0		3	2	0	1	2	2	0
Mvmt Flow	0 59		52	0	6	202	0	201	18
Number of Lanes	0 1		0	0	0	1	0	1	C
Approach	EB				NB			SB	
Opposing Approach					SB			NB	
Opposing Lanes	0				1			1	
Conflicting Approach Left	SB				EB				
Conflicting Lanes Left	1				1			0	
Conflicting Approach Right	NB							EB	
Conflicting Lanes Right	1				0			1	
HCM Control Delay	8.5				8.9			9	
HCM LOS	Α				Α			Α	
Lane	NBLn1	EBLn1	SBLn1					-	
Vol Left, %	3%	53%	0%						
Vol Thru, %	97%	0%	92%						
Vol Right, %	0%	47%	8%						
Sign Control	Stop	Stop	Stop						
Traffic Vol by Lane	181	96	191						
LT Vol	5	51	0						
Through Vol	176	0	175						
RT Vol	0	45	16						
Lane Flow Rate	208	110	220						
Geometry Grp	1	1	1						
Degree of Util (X)	0.255	0.143	0.267						
Departure Headway (Hd)	4.404	4.668	4.372						
	Yes	Yes	Yes						
Cap	816	768	823						
Service Time	2.426	2.697	2.394						
	0.255	0.143							
HCM Lane V/C Ratio	U.ZJJ								
HCM Lane V/C Ratio HCM Control Delay HCM Lane LOS	8.9 A	8.5 A	9 A						
	816 2.426	768 2.697	823						

	۶	*	4	†	Į.	4	
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR	
Lane Configurations	A			र्स	4		
Volume (vph)	44	42	4	206	157	2	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Lane Width (ft)	13	12	12	12	12	12	
Satd. Flow (prot)	1585	0	0	1708	1690	0	
FIt Permitted	0.975			0.999			
Satd. Flow (perm)	1585	0	0	1708	1690	0	
Link Speed (mph)	30			30	30		
Link Distance (ft)	292			750	716		
Travel Time (s)	6.6			17.0	16.3		
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	
Heavy Vehicles (%)	3%	0%	0%	0%	1%	0%	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	96	0	0	233	176	0	
Sign Control	Stop			Stop	Stop		
Intersection Summary		a light of				ATC MILES	
Area Type: Control Type: Unsignalized Intersection Capacity Utiliza Analysis Period (min) 15				10	CU Level o	of Service	A

Intersection										
Intersection Delay, s/veh	8.7									
Intersection LOS	Α									
Movement	EBU	EBL		EBR	NBU	NBL	NBT	SBU	SBT	SBR
Vol. veh/h	0	44		42	0	4	206	0	157	2
Peak Hour Factor	0.92	0.90		0.90	0.92	0.90	0.90	0,92	0.90	0.90
Heavy Vehicles, %	2	3		0	2	0	0	2	1	0
Mvmt Flow	0	49		47	0	4	229	0	174	2
Number of Lanes	0	1		0	0	0	1	0	1	0
Approach		EB		1012		NB			SB	- 3
Opposing Approach						SB			NB	
Opposing Lanes		0				1			1	
Conflicting Approach Left		SB				EB				
Conflicting Lanes Left		1				1			0	
Conflicting Approach Right		NB							EB	
Conflicting Lanes Right		1				0			1	
HCM Control Delay		8.3				9			8.6	
HCM LOS		Α				Α			Α	
Lane		NBLn1	EBLn1	SBLn1	Sec. 1	-				
Vol Left, %		2%	51%	0%						
Vol Thru, %		98%	0%	99%						
Vol Right, %		0%	49%	1%						
Sign Control		Stop	Stop	Stop						
Traffic Vol by Lane		210	86	159						
LT Vol		4	44	0						
Through Vol		206	0	157						
RT Vol		0	42	2						
Lane Flow Rate		233	96	177						
Geometry Grp		1	1	1						
Degree of Util (X)		0.28	0.124	0.215						
Departure Headway (Hd)		4.321	4.662	4.383						
Convergence, Y/N		Yes	Yes	Yes						
Cap		833	770	820						
Service Time		2.339	2.685	2.402						
HCM Lane V/C Ratio		0.28	0.125	0.216						
HCM Control Delay		9	8.3	8.6						
HCM Lane LOS		A	0.5 A	Α						
HCM 95th-tile Q		1.1	0.4	0.8						
TICIVI SOUI-LIIE Q		1.1	0.4	U.O						



	1	→	*	1	-	*	4	†	1	1	↓	1
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Volume (vph)	0	0	0	17	0	33	0	100	20	15	51	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	15	12	12	15	12	12	14	12	12	14	12
Satd. Flow (prot)	0	1881	0	0	1646	0	0	1753	0	0	1750	0
Flt Permitted	_				0.983						0.989	
Satd. Flow (perm)	0	1881	0	0	1646	0	0	1753	0	0	1750	0
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		215			700			704			700	
Travel Time (s)		4.9			15.9			16.0			15.9	
Peak Hour Factor	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Heavy Vehicles (%)	0%	0%	0%	7%	0%	0%	0%	2%	0%	0%	4%	0%
Shared Lane Traffic (%)	• , •											
Lane Group Flow (vph)	0	0	0	0	55	0	0	132	0	0	72	0
Sign Control	v	Stop			Stop			Free			Free	
Intersection Summary								21			- [

Control Type: Unsignalized

Intersection Capacity Utilization 20.6%

CBD

Analysis Period (min) 15

Intersection												
Int Delay, s/veh 2	.5											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	N	BL NBT	NBR	SBL	SBT	SBF
Vol, veh/h	0	0	0	17	0	33		0 100	20	15	51	(
Conflicting Peds, #/hr	0	0	0	0	0	0		0 0	0	0	0	(
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Fr	ee Free	Free	Free	Free	Free
RT Channelized	Otop	0,00	None		-	None			None	-	-	None
Storage Length					-	-			1.0	-	-	-
Veh in Median Storage, #		0		1	0	-		- 0	-		0	
Grade, %		0	-	-	0	-		- 0	-	(4)	0	
Peak Hour Factor	91	91	91	91	91	91		91 91	91	91	91	91
	0	0	0	7	0	0		0 2		0	4	(
Heavy Vehicles, %	0	0	0	19	0	36		0 110		16	56	(
Mvmt Flow	U	U	U	13	Ü	50		0 110		1,500		
Major/Minor	Minor2			Minor1			Majo			Major2	-	
Conflicting Flow All	228	221	56	210	210	121		56 0	0	132	0	(
Stage 1	89	89		121	121	~				*	2 %	
Stage 2	139	132		89	89							
Critical Hdwy	7.1	6.5	6.2	7.17	6.5	6.2	-	4.1	. 4	4.1	-	
Critical Hdwy Stg 1	6.1	5.5		6.17	5.5	-					-	
Critical Hdwy Stg 2	6.1	5.5		6.17	5.5				-		-	
Follow-up Hdwy	3.5	4	3.3	3.563	4	3.3		2.2		2.2		
Pot Cap-1 Maneuver	731	681	1016	737	691	936	15	62	1 -	1466	-	
Stage 1	923	825		871	800	-					4	
Stage 2	869	791	1	906	825	9		12 4			12	
Platoon blocked, %	000	, , ,			0.00000							0
	697	674	1016	731	683	936	15	62		1466	-	
Mov Cap-1 Maneuver	697	674	1010	731	683			-			-	
Mov Cap-2 Maneuver	923	816		871	800	-						
Stage 1		791	_	896	816						-	
Stage 2	835	791	-	030	010							
Approach	EB			WB				NB		SB		
HCM Control Delay, s	0			9.5				0		1.7		
HCM LOS	Ā			Α								
FICIVI LOS	^											
Minor Lane/Major Mvmt	NBL	NBT	NBR E	BLn1WBLn1	SBL	SBT	SBR					
Capacity (veh/h)	1562	-		- 855	1466	4						
HCM Lane V/C Ratio	-		4	- 0.064	0.011	-	-					
HCM Control Delay (s)	0		0	0 9.5	7.5	0						
HCM Lane LOS	Ä	-		A A			- 2					
HCM 95th %tile Q(veh)	0			- 0.2								

	٠	→	*	1	—	*	1	†	-	-	+	1
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Volume (vph)	1	0	0	1	0	24	1	81	16	11	73	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	15	12	12	15	12	12	14	12	12	14	12
Satd. Flow (prot)	0	1787	0	0	1633	0	0	1769	0	0	1813	0
Fit Permitted		0.950			0.998						0.994	
Satd. Flow (perm)	0	1787	0	0	1633	0	0	1769	0	0	1813	0
Link Speed (mph)		30		177	30			30			30	
Link Distance (ft)		215			700			704			700	
Travel Time (s)		4.9			15.9			16.0			15.9	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	0.30	0%	0%	0%	0%	0%	0%	1%	0%	0%	0%	0%
Shared Lane Traffic (%)		7.55										
Lane Group Flow (vph)	0	1	0	0	28	0	0	109	0	0	93	0
Sign Control		Stop			Stop			Free			Free	
Intersection Summary												

CBD

Control Type: Unsignalized

Intersection Capacity Utilization 21.6%

Analysis Period (min) 15

Intersection												
Int Delay, s/veh 1	.6											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBI	NBT	NBR	SBL	SBT	SBF
Vol, veh/h	1	0	0	1	0	24		81	16	11	73	0
Conflicting Peds, #/hr	0	0	0	0	0	0	(0	0	0	0	(
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None			None	+	-	None
Storage Length	-		-						-		-	
Veh in Median Storage, #		0		- 1-	0			- 0	-	-	0	-
Grade, %	2	0			0			- 0	-		0	-
Peak Hour Factor	90	90	90	90	90	90	96	90	90	90	90	90
Heavy Vehicles, %	0	0	0	0	0	0) 1	0	0	0	(
Mvmt Flow	1	0	0	ì	0	27		1 90	18	12	81	(
NA	Mineral			Minor1	-		Major	1		Major2		
Major/Minor	Minor2	040	:04	207	207	99	8		0	108	0	(
Conflicting Flow All	220	216	81	101	101	99	U		U	100	_	
Stage 1	106	106	*		106				-			
Stage 2	114	110	-	106		6.2	4.	1		4.1		
Critical Hdwy	7.1	6.5	6.2	7.1	6.5	0.2	4.			7,1		
Critical Hdwy Stg 1	6.1	5.5		6.1	5.5	- 1			-			
Critical Hdwy Stg 2	6.1	5.5	-	6.1	5.5	- 0.0	0	0		2.2		
Follow-up Hdwy	3.5	4	3.3	3.5	4	3.3	2.		-	1495		
Pot Cap-1 Maneuver	740	685	985	755	693	962	152	9 -	-	1495		
Stage 1	905	811	-	910	815							
Stage 2	896	808	-	905	811							
Platoon blocked, %									*			
Mov Cap-1 Maneuver	715	679	985	750	687	962	152	9 -		1495		
Mov Cap-2 Maneuver	715	679		750	687							
Stage 1	904	805	4	909	814	-			-			
Stage 2	870	807		898	805							
Approach	EB	- 2		WB			N	В		SB		
HCM Control Delay, s	10			8.9			0.			1		
HCM LOS	В			A			,					
Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1WBLn1	SBL	SBT	SBR					
Capacity (veh/h)	1529			715 951	1495	-,4	(%)					
HCM Lane V/C Ratio	0.001	_	-	0.002 0.029			195					
HCM Control Delay (s)	7.4	0		10 8.9	7.4		14.					
HCM Lane LOS	Α.Α	A	- 2	B A			24					
	0	^		0 0.1	0		20					
HCM 95th %tile Q(veh)	U	-	-	0 0.1	U	-						

	٠	→	*	1	-	*	4	†	-	-	↓	1
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		43			4			4			4	
Volume (vph)	0	0	0	18	0	35	0	111	21	16	61	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	15	12	12	15	12	12	14	12	12	14	12
Satd. Flow (prot)	0	1881	0	0	1647	0	0	1756	0	0	1751	0
Flt Permitted					0.983						0.990	
Satd. Flow (perm)	0	1881	0	0	1647	0	0	1756	0	0	1751	0
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		215			700			704			700	
Travel Time (s)		4.9			15.9			16.0			15.9	
Peak Hour Factor	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Heavy Vehicles (%)	0%	0%	0%	7%	0%	0%	0%	2%	0%	0%	4%	0%
Shared Lane Traffic (%)	070	0.70	070	*.**	• • • • • • • • • • • • • • • • • • • •							
Lane Group Flow (vph)	0	0	0	0	58	0	0	145	0	0	85	0
Sign Control	U	Stop	U	J	Stop			Free			Free	

Area Type:

CBD

Control Type: Unsignalized Intersection Capacity Utilization 26.0%

Analysis Period (min) 15

Intersection												
Int Delay, s/veh 2.	.4											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Vol, veh/h	0	0	0	18	0	35	(111	21	16	61	0
Conflicting Peds, #/hr	0	0	0	0	0	0	(0	0	0	0	C
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-		None	·-	-	None			None			None
Storage Length			-		-	100	19			*		
Veh in Median Storage, #		0		4	0			- 0	-		0	
Grade, %	41	0	12.	-	0	_		- 0		- 2	0	
Peak Hour Factor	91	91	91	91	91	91	9	91	91	91	91	91
Heavy Vehicles, %	0	0	0	7	0	0	(2	0	0	4	(
Mvmt Flow	Ŏ	0	0	20	0	38	(122	23	18	67	(
				Minord			Major			Major2		
Major/Minor	Minor2	4.179	07	Minor1	000	424	6		0	145	0	(
Conflicting Flow All	255	247	67	236	236	134	0			145	-	
Stage 1	102	102	-	134	134	-				191		
Stage 2	153	145	*	102	102	-				4.1		
Critical Hdwy	7.1	6.5	6.2	7.17	6.5	6.2	4.					
Critical Hdwy Stg 1	6.1	5.5	-	6.17	5.5	-					-	
Critical Hdwy Stg 2	6.1	5.5	-	6.17	5.5	-				- 0.0		
Follow-up Hdwy	3.5	4	3.3	3.563	4	3.3	2.			2.2		
Pot Cap-1 Maneuver	702	659	1002	708	668	920	154			1450		
Stage 1	909	815	-	857	789	-				*	-	
Stage 2	854	781	197	892	815							
Platoon blocked, %								-	-	1.000		
Mov Cap-1 Maneuver	666	650	1002	701	659	920	154	7 -	-	1450	-	
Mov Cap-2 Maneuver	666	650	-	701	659	-					-	
Stage 1	909	804	-	857	789	-		el je		•	*	
Stage 2	818	781	-	880	804	-				*		
Approach	EB			WB			N	3		SB		
	0			9.7				0		1.6		
HCM Control Delay, s HCM LOS	A			A								
W	A Vers	VID.	LIND P	DI -4WDI -4	CDI	CDT	SBR					
Minor Lane/Major Mvmt	NBL	NBT	NRK F	BLn1WBLn1	SBL	SBT	SDIN				-	
Capacity (veh/h)	1547	7	-	- 832	1450							
HCM Lane V/C Ratio	-				0.012	•	*					
HCM Control Delay (s)	0		-	0 9.7	7.5	0						
HCM Lane LOS	Α	-		A A	Α	Α	~					
HCM 95th %tile Q(veh)	0		1.5	- 0.2	0	-	-					

	٠	-	*	1	-	*	1	†	-	1	↓	1
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Volume (vph)	1	0	0	1	0	26	1	109	17	12	88	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	15	12	12	15	12	12	14	12	12	14	12
Satd. Flow (prot)	0	1787	0	0	1631	0	0	1776	0	0	1813	0
Flt Permitted		0.950			0.998						0.994	
Satd. Flow (perm)	0	1787	0	0	1631	0	0	1776	0	0	1813	0
Link Speed (mph)	· ·	30			30			30			30	
Link Distance (ft)		215			700			704			700	
Travel Time (s)		4.9			15.9			16.0			15.9	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	0%	0%	0%	0%	0%	0%	0%	1%	0%	0%	0%	0%
Shared Lane Traffic (%)	0.70	0.0										
Lane Group Flow (vph)	0	1	0	0	30	0	0	141	0	0	111	0
Sign Control	0	Stop			Stop			Free			Free	

Area Type:

CBD

Control Type: Unsignalized

Intersection Capacity Utilization 24.4%

Analysis Period (min) 15

Intersection												_=1
Int Delay, s/veh 1.	4											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NE	BL NBT	NBR	SBL	SBT	SBR
Vol, veh/h	1	0	0	1	0	26		1 109	17	12	88	C
Conflicting Peds, #/hr	0	0	0	0	0	0		0 0	0	0	0	C
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Fre	e Free	Free	Free	Free	Free
RT Channelized	Otop -	-	None	-	-	None			None			None
Storage Length			-		_	-					-	
Veh in Median Storage, #		0	_		0			- 0		-	0	
Grade, %		0			0	-		- 0	-	-	0	
Peak Hour Factor	90	90	90	90	90	90		90 90		90	90	90
Heavy Vehicles, %	0	0	0	0	0	0		0 1		0	0	(
Mymt Flow	1	0	0	1	0	29		1 121		13	98	(
INIVITAT FIOW	'	U	U		· ·	20						
`				A Maried V			4.4000	and .		Malago	-	
Major/Minor	Minor2		200	Minor1			Majo			Major2		
Conflicting Flow All	271	266	98	257	257	131		98 0		140	0	(
Stage 1	124	124	39	133	133	-						
Stage 2	147	142	-	124	124	-				200		
Critical Hdwy	7.1	6.5	6.2	7.1	6.5	6.2	4	.1 .		4.1	-	
Critical Hdwy Stg 1	6.1	5.5	-	6.1	5.5	-		-			-	
Critical Hdwy Stg 2	6.1	5.5	-	6.1	5.5	-				ė		
Follow-up Hdwy	3.5	4	3.3	3.5	4	3.3				2.2		
Pot Cap-1 Maneuver	686	643	963	700	651	924	15	08		1456	- 4	
Stage 1	885	797	-	875	790	-		*				9
Stage 2	860	783	-	885	797	-		4 .				
Platoon blocked, %								3			-	
Mov Cap-1 Maneuver	659	637	963	695	644	924	15	08		1456	7.	
Mov Cap-2 Maneuver	659	637	_	695	644	-				-		
Stage 1	884	790	-	874	789	-		9		-		
Stage 2	832	782	-	877	790	-						
0												
Approach	EB			WB				VB		SB		
HCM Control Delay, s	10.5			9.1			(0.1		0.9		
HCM LOS	В			Α								
,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,												
Minor Lane/Major Mymt	NBL.	NBT	NBR	EBLn1WBLn1	SBL	SBT	SBR			23		
Capacity (veh/h)	1508	-	4	659 913	1456		+					
HCM Lane V/C Ratio	0.001		-	0.002 0.033	0.009	-	2					
HCM Control Delay (s)	7.4	0		10.5 9.1		0	9					
HCM Lane LOS	Α	Ā		в А		Α	-					
HCM 95th %tile Q(veh)	0			0 0.1								

<i>></i>	\rightarrow	*	1	-	4	4	†	1	1	↓	4
EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
	44			44			4			4	
0	0	0	18	0	35	0	111	21	18	61	. 0
1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
12	15	12	12	15	12	12	14	12	12	14	12
0	1881	0	0	1647	0	0	1756	0	0	1750	0
				0.983						0.989	
0	1881	0	0	1647	0	0	1756	0	0	1750	0
	30			30			30			30	
	215			700			704			700	
	4.9			15.9			16.0			15.9	
0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
0%	0%	0%	7%	0%	0%	0%	2%	0%	0%	4%	0%
0	0	0	0	58	0	0	145	0	0	87	0
	Stop			Stop			Free			Free	
	0 1900 12 0 0	0 0 1900 1900 12 15 0 1881 30 215 4.9 0.91 0.91 0.91 0.91 0.91 0.91 0.90 0 0	0 0 0 0 1900 1900 12 15 12 0 1881 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	0 0 0 18 1900 1900 1900 1900 12 15 12 12 0 1881 0 0 0 1881 0 0 30 215 4.9 0.91 0.91 0.91 0.91 0% 0% 0% 7%	0 0 0 18 0 1900 1900 1900 1900 1900 12 15 12 12 15 0 1881 0 0 1647 0.983 0 1881 0 0 1647 30 30 215 700 4.9 15.9 0.91 0.91 0.91 0.91 0% 0% 0% 7% 0%	Φ Φ 0 0 0 18 0 35 1900 1900 1900 1900 1900 1900 12 15 12 12 15 12 0 1881 0 0 1647 0 0 1881 0 0 1647 0 30 30 30 30 30 215 700 4.9 15.9 0 0.91 0.91 0.91 0.91 0.91 0.91 0% 0% 0% 7% 0% 0% 0 0 0 58 0	Φ Φ 0 0 0 18 0 35 0 1900 1900 1900 1900 1900 1900 1900 12 15 12 12 15 12 12 0 1881 0 0 1647 0 0 0 1881 0 0 1647 0 0 30 <td>♣ ♣ ♣ 0 0 0 18 0 35 0 111 1900 1900 1900 1900 1900 1900 1900 1900 12 15 12 12 15 12 12 14 0 1881 0 0 1647 0 0 1756 30 30 30 30 215 700 704 4.9 15.9 16.0 0.91 0.91 0.91 0.91 0.91 0.91 0.91 0% 0% 0% 7% 0% 0% 0% 2%</td> <td>♣ ♣ ♠ 0 0 0 18 0 35 0 111 21 1900 1756 0</td> <td>♣ ♣ ♣ 0 0 0 18 0 35 0 111 21 18 1900</td> <td>♣ ♣ ♣ ♣ ♣ ♣ ♣ ♣ ♣ ♣ ♣ ♣ ♣ ♠ ♣ ♠</td>	♣ ♣ ♣ 0 0 0 18 0 35 0 111 1900 1900 1900 1900 1900 1900 1900 1900 12 15 12 12 15 12 12 14 0 1881 0 0 1647 0 0 1756 30 30 30 30 215 700 704 4.9 15.9 16.0 0.91 0.91 0.91 0.91 0.91 0.91 0.91 0% 0% 0% 7% 0% 0% 0% 2%	♣ ♣ ♠ 0 0 0 18 0 35 0 111 21 1900 1756 0	♣ ♣ ♣ 0 0 0 18 0 35 0 111 21 18 1900	♣ ♣ ♣ ♣ ♣ ♣ ♣ ♣ ♣ ♣ ♣ ♣ ♣ ♠ ♣ ♠

Area Type:

CBD

Control Type: Unsignalized

Intersection Capacity Utilization 26.1%

Analysis Period (min) 15

Intersection						-						
Int Delay, s/veh	2.5											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Vol, veh/h	0	0	0	18	0	35	0	111	21	18	61	C
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	(
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-		None	-	-	None	-	-	None	-	-	None
Storage Length		- 12	-			-			-	9	-	
Veh in Median Storage, #	-	0	-		0			0			0	
Grade, %		0			0			0			0	
Peak Hour Factor	91	91	91	91	91	91	91	91	91	91	91	91
Heavy Vehicles, %	0	0	0	7	0	0	0	2	0	0	4	C
Mvmt Flow	0	0	0	20	0	38	0	122	23	20	67	C
Major/Minor	Minor2		-011	Minor1		53	Major1	-		Major2		
Conflicting Flow All	260	252	67	241	241	134	67	0	0	145	0	0
Stage 1	107	107	07	134	134	104	- 01	-	-	140	-	
	153	145		107	107			-		3		
Stage 2	7.1	6.5	6.2	7.17	6.5	6.2	4.1		4	4.1		
Critical Identification	6.1	5.5	0.2	6.17	5.5	0.2	7.1	-		7.1		
Critical Holy Stg 1		5.5		6.17	5.5							
Critical Hdwy Stg 2	6.1 3.5	3.5	3.3	3.563	4	3.3	2.2			2.2		
Follow-up Hdwy		655	1002	703	664	920	1547			1450	3	
Pot Cap-1 Maneuver	697		1002	857	789			-		1430		
Stage 1	903	811							-	- 3		
Stage 2	854	781		886	811			-	-		-	
Platoon blocked, %	004	040	4000	205	CEE	000	4547	-		1450		
Mov Cap-1 Maneuver	661	646	1002	695	655	920	1547	-	-	1450	-	
Mov Cap-2 Maneuver	661	646		695	655						- 1	
Stage 1	903	800	*	857	789							
Stage 2	818	781		874	800							
Approach	EB			WB			NB	W		SB		
HCM Control Delay, s	0			9.7			0			1.7		
HCM LOS	Α			Α								
Minor Lane/Major Mvmt	NBL	NBT	NBR F	BLn1WBLn1	SBL	SBT	SBR					
Capacity (veh/h)	1547			- 829	1450							
HCM Lane V/C Ratio	1041		1		0.014							
HCM Control Delay (s)	0			0 9.7	7.5	0						
HCM Lane LOS		7	-	A A	7.5 A	A						
	A 0			- 0.2	0	^	2					
HCM 95th %tile Q(veh)	0	7	- 0	- 0.2	U	-						

	*	\rightarrow	*	-	←	*	4	†	-	-	↓	1
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Volume (vph)	1	0	0	1	0	26	1	109	17	17	88	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	15	12	12	15	12	12	14	12	12	14	12
Satd. Flow (prot)	0	1787	0	0	1631	0	0	1776	0	0	1809	0
Flt Permitted		0.950			0.998						0.992	
Satd. Flow (perm)	0	1787	0	0	1631	0	0	1776	0	0	1809	0
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		215			700			704			700	
Travel Time (s)		4.9			15.9			16.0			15.9	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	0%	0%	0%	0%	0%	0%	0%	1%	0%	0%	0%	0%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	. 0	1	0	0	30	0	0	141	0	0	117	0
Sign Control		Stop			Stop			Free			Free	

Area Type:

CBD

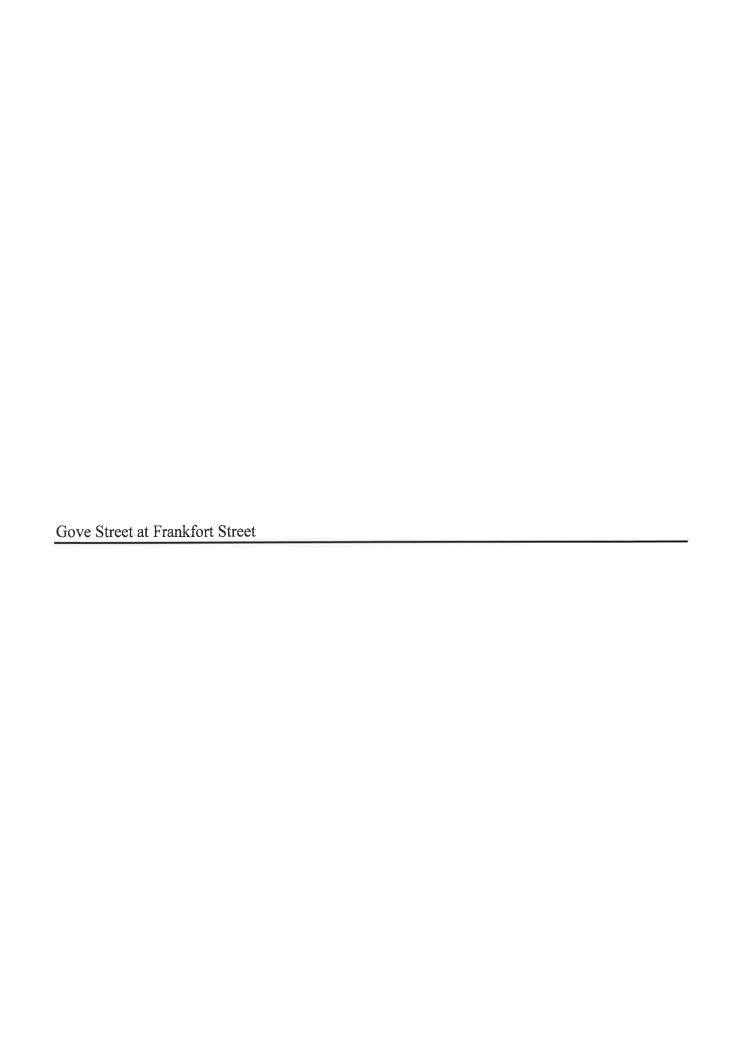
Control Type: Unsignalized

Intersection Capacity Utilization 27.1%

ICU Level of Service A

Analysis Period (min) 15

Intersection														
Int Delay, s/veh 1	.5													
Movement	EBL	EBT	EBR		WBL	WBT	WBR	2,83	NBL	NBT	NBR	SBL	SBT	SBR
Vol, veh/h	1	0	0		1	0	26		1	109	17	17	88	0
Conflicting Peds, #/hr	0	0	0		0	0	0		0	0	0	0	0	C
Sign Control	Stop	Stop	Stop		Stop	Stop	Stop		Free	Free	Free	Free	Free	Free
RT Channelized			None			-	None			- 5	None	-		None
Storage Length	+		*		-		-		-	-	-		- 3	
Veh in Median Storage, #	4	0	-		-	0			-	0	1.0	- 1	0	
Grade, %		0			-	0	-			0	-		0	
Peak Hour Factor	90	90	90		90	90	90		90	90	90	90	90	90
Heavy Vehicles, %	0	0	0		0	0	0		0	1	0	0	0	0
Mvmt Flow	1	0	0		1	0	29		1	121	19	19	98	C
Major/Minor	Minor2			I.N.	linor1			٨	//ajor1			Major2	-	
Conflicting Flow All	283	278	98		269	269	131		98	0	0	140	0	0
Stage 1	136	136	-		133	133			-		-			
Stage 2	147	142			136	136	4						-	
Critical Hdwy	7.1	6.5	6.2		7.1	6.5	6.2		4.1		-	4.1		
Critical Hdwy Stg 1	6.1	5.5	0.2		6.1	5.5	0.2		-			-		
Critical Hdwy Stg 2	6.1	5.5	-		6.1	5.5	2			-				
Follow-up Hdwy	3.5	4	3.3		3.5	4	3.3		2.2			2.2	2	
	673	633	963		688	641	924		1508			1456	2	
Pot Cap-1 Maneuver	872	788	503		875	790	324		1000	- 3		1750		
Stage 1		783			872	788			-					
Stage 2	860	103			0/2	100				-				
Platoon blocked, %	0.45	00.4	000		000	004	004		1508			1456		
Mov Cap-1 Maneuver	645	624	963		680	631	924		1000			1400		
Mov Cap-2 Maneuver	645	624			680	631	-							
Stage 1	871	777			874	789	-							
Stage 2	832	782	*		860	777	-							
Approach	EB			14-5	WB			- 100	NB			SB		35
HCM Control Delay, s	10.6				9.1				0.1			1.2		
HCM LOS	В				Α									
Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1W	/BLn1	SBL	SBT	SBR						- 2
Capacity (veh/h)	1508		-	645	912	1456		(4)						
HCM Lane V/C Ratio	0.001		-	0.002			_							
HCM Control Delay (s)	7.4	0		10.6	9.1	7.5	0	2						
HCM Lane LOS	A	Ä		В	A	A	Ä							
HCM 95th %tile Q(veh)	0			0	0.1	0	, ,							



	•	-	-	1	•	*	4	†	1	-	+	1
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			43			4			4	
Volume (vph)	4	28	3	7	41	28	5	43	7	29	14	4
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	14	12	12	14	12	12	14	12	12	14	12
Satd. Flow (prot)	0	1793	0	0	1654	0	0	1784	0	0	1709	0
Flt Permitted	4	0.995			0.996			0.995			0.970	
Satd. Flow (perm)	0	1793	0	0	1654	0	0	1784	0	0	1709	0
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		700			888			651			694	
Travel Time (s)		15.9			20.2			14.8			15.8	
Peak Hour Factor	0.76	0.76	0.76	0.76	0.76	0.76	0.76	0.76	0.76	0.76	0.76	0.76
Heavy Vehicles (%)	0%	0%	0%	0%	8%	0%	0%	0%	0%	0%	8%	0%
Shared Lane Traffic (%)	0,0	0,0	3.00		2,15							
Lane Group Flow (vph)	0	46	0	0	100	0	0	73	0	0	61	0
Sign Control		Stop			Stop			Stop			Stop	
		_						_				

Area Type:

CBD

Control Type: Unsignalized

Intersection Capacity Utilization 22.1%

Analysis Period (min) 15

Intersection												
Intersection Delay, s/veh	7.6											
Intersection LOS	Α											
Movement	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBF
Vol, veh/h	0	4	28	3	0	7	41	28	0	5	43	
Peak Hour Factor	0.92	0.76	0.76	0.76	0.92	0.76	0.76	0.76	0.92	0.76	0.76	0.76
Heavy Vehicles, %	2	0	0	0	2	0	8	0	2	0	0	(
Mvmt Flow	0	5	37	4	0	9	54	37	0	7	57	
Number of Lanes	0	0	1	0	0	0	1	0	0	0	1	1
Approach		EB				WB				NB		
Opposing Approach		WB				EB				SB		
Opposing Lanes		1				1				1		
Conflicting Approach Left		SB				NB				EB		
Conflicting Lanes Left		1				1				1		
Conflicting Approach Right		NB				SB				WB		
Conflicting Lanes Right		1				1				1		
HCM Control Delay		7.5				7.5				7.6		
TION CONGO BOILEY						Α.				A		
		A				Α				-		
HCM LOS		А				А						
HCM LOS		A NBLn1	EBLn1	WBLn1	SBLn1	A						
HCM LOS			11%	9%	62%	A						
Lane Vol Left, %		NBLn1	11% 80%	9% 54%	62% 30%	A						
Lane Vol Left, % Vol Thru, %		NBLn1 9%	11%	9% 54% 37%	62% 30% 9%	A						
Lane Vol Left, % Vol Thru, % Vol Right, %		NBLn1 9% 78%	11% 80% 9% Stop	9% 54% 37% Stop	62% 30% 9% Stop	A				7		
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control		NBLn1 9% 78% 13%	11% 80% 9%	9% 54% 37%	62% 30% 9% Stop 47	A				-		
HCM LOS Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane		NBLn1 9% 78% 13% Stop	11% 80% 9% Stop 35 4	9% 54% 37% Stop 76 7	62% 30% 9% Stop 47 29	A						
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol		NBLn1 9% 78% 13% Stop 55	11% 80% 9% Stop 35 4 28	9% 54% 37% Stop 76 7	62% 30% 9% Stop 47 29 14	A				7		
HCM LOS Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol		NBLn1 9% 78% 13% Stop 55 5	11% 80% 9% Stop 35 4 28 3	9% 54% 37% Stop 76 7 41 28	62% 30% 9% Stop 47 29 14 4	A						
HCM LOS Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol		NBLn1 9% 78% 13% Stop 55 5 43	11% 80% 9% Stop 35 4 28	9% 54% 37% Stop 76 7	62% 30% 9% Stop 47 29 14	A				7		
HCM LOS Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate		9% 78% 13% Stop 55 5 43 7	11% 80% 9% Stop 35 4 28 3	9% 54% 37% Stop 76 7 41 28	62% 30% 9% Stop 47 29 14 4 62	A						
HCM LOS Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp		9% 78% 13% Stop 55 5 43 7 72	11% 80% 9% Stop 35 4 28 3 46	9% 54% 37% Stop 76 7 41 28 100	62% 30% 9% Stop 47 29 14 4 62 1	A				-70		
HCM LOS Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X)		NBLn1 9% 78% 13% Stop 55 5 43 7 72 1	11% 80% 9% Stop 35 4 28 3 46	9% 54% 37% Stop 76 7 41 28 100	62% 30% 9% Stop 47 29 14 4 62 1 0.074 4.282	A						
HCM LOS Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd)		NBLn1 9% 78% 13% Stop 55 5 43 7 72 1 0.083	11% 80% 9% Stop 35 4 28 3 46 1 0.053	9% 54% 37% Stop 76 7 41 28 100 1	62% 30% 9% Stop 47 29 14 4 62 1 0.074 4.282 Yes	A						
HCM LOS Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N		NBLn1 9% 78% 13% Stop 55 5 43 7 72 1 0.083 4.143	11% 80% 9% Stop 35 4 28 3 46 1 0.053 4.181	9% 54% 37% Stop 76 7 41 28 100 1 0.11 3.964	62% 30% 9% Stop 47 29 14 4 62 1 0.074 4.282 Yes 826	A						
HCM LOS Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap		NBLn1 9% 78% 13% Stop 55 5 43 7 72 1 0.083 4.143 Yes	11% 80% 9% Stop 35 4 28 3 46 1 0.053 4.181 Yes	9% 54% 37% Stop 76 7 41 28 100 1 0.11 3.964 Yes	62% 30% 9% Stop 47 29 14 4 62 1 0.074 4.282 Yes 826 2.364	A						
HCM LOS Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time		9% 78% 13% Stop 55 5 43 7 72 1 0.083 4.143 Yes 853	11% 80% 9% Stop 35 4 28 3 46 1 0.053 4.181 Yes 843	9% 54% 37% Stop 76 7 41 28 100 1 0.11 3.964 Yes 891	62% 30% 9% Stop 47 29 14 4 62 1 0.074 4.282 Yes 826	A						
HCM LOS Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time HCM Lane V/C Ratio		9% 78% 13% Stop 55 5 43 7 72 1 0.083 4.143 Yes 853 2.224	11% 80% 9% Stop 35 4 28 3 46 1 0.053 4.181 Yes 843 2.275	9% 54% 37% Stop 76 7 41 28 100 1 0.11 3.964 Yes 891 2.048	62% 30% 9% Stop 47 29 14 4 62 1 0.074 4.282 Yes 826 2.364	A						
HCM LOS Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time		9% 78% 13% Stop 55 5 43 7 72 1 0.083 4.143 Yes 853 2.224 0.084	11% 80% 9% Stop 35 4 28 3 46 1 0.053 4.181 Yes 843 2.275 0.055	9% 54% 37% Stop 76 7 41 28 100 1 0.11 3.964 Yes 891 2.048 0.112	62% 30% 9% Stop 47 29 14 4 62 1 0.074 4.282 Yes 826 2.364 0.075	A						

Intersection Delay, s/veh										
Intersection LOS										
Movement	SBU	SBL	SBT	SBR	V 19.4	y Page	1700		11 2 25	1051
Vol, veh/h	0	29	14	4						
Peak Hour Factor	0.92	0.76	0.76	0.76						
Heavy Vehicles, %	2	0	8	0						
Mvmt Flow	0	38	18	5						
Number of Lanes	0	0	1 1	0						
4										
Approach		SB	4 8 4		1 / 18 V		377T	-301		Sill.
Opposing Approach		NB								
Opposing Lanes		1								
Conflicting Approach Left		WB								
Conflicting Lanes Left		1								
Conflicting Approach Right		EB								
Conflicting Lanes Right		1								
HCM Control Delay		7.7								
HCM LOS		Α								

	*	→	*	1	←	*	4	†	-	1		1
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Volume (vph)	7	18	2	9	18	29	2	17	6	5	25	5
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	14	12	12	14	12	12	14	12	12	14	12
Satd. Flow (prot)	0	1786	0	0	1683	0	0	1760	0	0	1777	0
Flt Permitted		0.987			0.992			0.997			0.993	
Satd. Flow (perm)	0	1786	0	0	1683	0	0	1760	0	0	1777	0
Link Speed (mph)		30			30			30			30	
Link Opeca (mph)		700			888			651			694	
Travel Time (s)		15.9			20.2			14.8			15.8	
Peak Hour Factor	0.81	0.81	0.81	0.81	0.81	0.81	0.81	0.81	0.81	0.81	0.81	0.81
Heavy Vehicles (%)	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%
Shared Lane Traffic (%)	070	070	0,10	- 19								
Lane Group Flow (vph)	0	33	0	0	69	0	0	30	0	0	43	0
Sign Control	U	Stop	· ·	Ü	Stop			Stop			Stop	
Intersection Summary				-								211

CBD

Control Type: Unsignalized

Intersection Capacity Utilization 14.2%

ICU Level of Service A

Analysis Period (min) 15

Intersection												
Intersection Delay, s/veh	7.2											
Intersection LOS	Α											0.15
Movement	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBF
Vol, veh/h	0	7	18	2	0	9	18	29	0	2	17	(
Peak Hour Factor	0.92	0.81	0.81	0.81	0.92	0.81	0.81	0.81	0.92	0.81	0.81	0.8
Heavy Vehicles, %	2	0	0	0	2	0	0	0	2	0	0	(
Mymt Flow	0	9	22	2	0	11	22	36	0	2	21	
Number of Lanes	0	0	1	0	0	0	1	0	0	0	1	(
Approach		EB				WB				NB	1 5	-
Opposing Approach		WB				EB				SB		
Opposing Lanes		1				1				1		
Conflicting Approach Left		SB				NB				EB		
Conflicting Lanes Left		1				1				1		
Conflicting Approach Right		NB				SB				WB		
Conflicting Lanes Right		1				1				1		
		7.3				7.1				7.2		
LIPINI COLLIOI DEIGA						· A				A		
HCM Control Delay HCM LOS		Α				Α				^		
HCM LOS		Α				А				^		
		A NBLn1	EBLn1	WBLn1	SBLn1	А			3 3	^		
HCM LOS		NBLn1 8%	26%	16%	14%	A			8_8			
HCM LOS		NBLn1 8% 68%	26% 67%	16% 32%	14% 71%	A			3 3			
Lane Vol Left, % Vol Thru, %	- 1	NBLn1 8%	26%	16% 32% 52%	14% 71% 14%	A			8 _8			
Lane Vol Left, %		NBLn1 8% 68% 24% Stop	26% 67% 7% Stop	16% 32% 52% Stop	14% 71% 14% Stop	A			×	^		
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control		8% 68% 24% Stop 25	26% 67% 7% Stop 27	16% 32% 52% Stop 56	14% 71% 14% Stop 35	A			3 3	^		
Lane Vol Left, % Vol Thru, % Vol Right, %		NBLn1 8% 68% 24% Stop 25 2	26% 67% 7% Stop 27	16% 32% 52% Stop 56 9	14% 71% 14% Stop 35 5	A				^		
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol		NBLn1 8% 68% 24% Stop 25 2 17	26% 67% 7% Stop 27 7	16% 32% 52% Stop 56 9	14% 71% 14% Stop 35 5	A				^		
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane		NBLn1 8% 68% 24% Stop 25 2 17	26% 67% 7% Stop 27 7 18	16% 32% 52% Stop 56 9 18	14% 71% 14% Stop 35 5 25 5	A				^		
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol		NBLn1 8% 68% 24% Stop 25 2 17	26% 67% 7% Stop 27 7	16% 32% 52% Stop 56 9 18 29 69	14% 71% 14% Stop 35 5 25 5 43	A				^		
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate		NBLn1 8% 68% 24% Stop 25 2 17	26% 67% 7% Stop 27 7 18 2 33	16% 32% 52% Stop 56 9 18 29 69	14% 71% 14% Stop 35 5 25 5 43	A				^		
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp		NBLn1 8% 68% 24% Stop 25 2 17 6	26% 67% 7% Stop 27 7 18 2	16% 32% 52% Stop 56 9 18 29 69 1	14% 71% 14% Stop 35 5 25 5 43 1	A				^		
HCM LOS Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X)		NBLn1 8% 68% 24% Stop 25 2 17 6 31	26% 67% 7% Stop 27 7 18 2 33	16% 32% 52% Stop 56 9 18 29 69 1 0.072 3.774	14% 71% 14% Stop 35 5 25 5 43 1 0.049 4.043	A				^		
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd)		NBLn1 8% 68% 24% Stop 25 2 17 6 31 1 0.034	26% 67% 7% Stop 27 7 18 2 33 1 0.038 4.087 Yes	16% 32% 52% Stop 56 9 18 29 69 1 0.072 3.774 Yes	14% 71% 14% Stop 35 5 25 5 43 1 0.049 4.043 Yes	A				^		
HCM LOS Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N		NBLn1 8% 68% 24% Stop 25 2 17 6 31 1 0.034 3.982	26% 67% 7% Stop 27 7 18 2 33 1 0.038 4.087	16% 32% 52% Stop 56 9 18 29 69 1 0.072 3.774 Yes 944	14% 71% 14% Stop 35 5 25 5 43 1 0.049 4.043 Yes 882	A						
HCM LOS Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap		NBLn1 8% 68% 24% Stop 25 2 17 6 31 1 0.034 3.982 Yes	26% 67% 7% Stop 27 7 18 2 33 1 0.038 4.087 Yes	16% 32% 52% Stop 56 9 18 29 69 1 0.072 3.774 Yes	14% 71% 14% Stop 35 5 25 5 43 1 0.049 4.043 Yes 882 2.086	A						
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time		NBLn1 8% 68% 24% Stop 25 2 17 6 31 1 0.034 3.982 Yes 894	26% 67% 7% Stop 27 7 18 2 33 1 0.038 4.087 Yes 872	16% 32% 52% Stop 56 9 18 29 69 1 0.072 3.774 Yes 944	14% 71% 14% Stop 35 5 25 5 43 1 0.049 4.043 Yes 882 2.086 0.049	A						
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time HCM Lane V/C Ratio		NBLn1 8% 68% 24% Stop 25 2 17 6 31 1 0.034 3.982 Yes 894 2.029	26% 67% 7% Stop 27 7 18 2 33 1 0.038 4.087 Yes 872 2.132	16% 32% 52% Stop 56 9 18 29 69 1 0.072 3.774 Yes 944 1.816	14% 71% 14% Stop 35 5 25 5 43 1 0.049 4.043 Yes 882 2.086	A						
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time		NBLn1 8% 68% 24% Stop 25 2 17 6 31 1 0.034 3.982 Yes 894 2.029 0.035	26% 67% 7% Stop 27 7 18 2 33 1 0.038 4.087 Yes 872 2.132 0.038	16% 32% 52% Stop 56 9 18 29 69 1 0.072 3.774 Yes 944 1.816 0.073	14% 71% 14% Stop 35 5 25 5 43 1 0.049 4.043 Yes 882 2.086 0.049	A						

Intersection Delay aluah										
Intersection Delay, s/veh Intersection LOS										
Movement	SBU	SBL	SBT	SBR		Y HIXL		li ili	N. Car	
Vol, veh/h	0	5	25	5						
Peak Hour Factor	0.92	0.81	0.81	0.81						
Heavy Vehicles, %	2	0	0	0						
Mvmt Flow	0	6	31	6						
Number of Lanes	0	0	1	0						
Approach	IIX.	SB	and at	- Prof.		Suit in	1990	h I	N. S. F. III	M.
Opposing Approach		NB								
Opposing Lanes		1								
Conflicting Approach Left		WB								
Conflicting Lanes Left		1								
Conflicting Approach Right		EB								
Conflicting Lanes Right		1								
HCM Control Delay		7.3								
HCM LOS		Α								

	٤	→	*	•	-	4	1	†	-	-	↓	1
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Volume (vph)	4	30	3	7	44	30	5	46	8	31	15	4
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	14	12	12	14	12	12	14	12	12	14	12
Satd. Flow (prot)	0	1795	0	0	1653	0	0	1782	0	0	1710	0
Flt Permitted		0.995			0.996			0.996			0.970	
Satd. Flow (perm)	0	1795	0	0	1653	0	0	1782	0	0	1710	0
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		700			888			651			694	
Travel Time (s)		15.9			20.2			14.8			15.8	
Peak Hour Factor	0.76	0.76	0.76	0.76	0.76	0.76	0.76	0.76	0.76	0.76	0.76	0.76
Heavy Vehicles (%)	0%	0%	0%	0%	8%	0%	0%	0%	0%	0%	8%	0%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	48	0	0	106	0	0	79	. 0	0	66	0
Sign Control		Stop			Stop			Stop			Stop	

Area Type:

Control Type: Unsignalized

Intersection Capacity Utilization 22.7%

CBD

Analysis Period (min) 15

Intersection												
Intersection Delay, s/veh Intersection LOS	7.7 A											
Movement	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBR
Vol, veh/h	0	4	30	3	0	7	44	30	0	5	46	8
Peak Hour Factor	0.92	0.76	0.76	0.76	0.92	0.76	0.76	0.76	0.92	0.76	0.76	0.76
Heavy Vehicles, %	2	0	0	0	2	0	8	0	2	0	0	(
Mymt Flow	0	5	39	4	0	9	58	39	0	7	61	11
Number of Lanes	0	0	1	0	0	0	1	0	0	0	1	(
Approach		EB				WB	100			NB		
Opposing Approach		WB				EB				SB		
Opposing Lanes		1				1				1		
Conflicting Approach Left		SB				NB				EB		
Conflicting Lanes Left		1				1				1		
Conflicting Approach Right		NB				SB				WB		
Conflicting Lanes Right		1				1				1		
Commonly Editor right						7.6				7.7		
		7.6				1.0						
HCM Control Delay		7.6 A				Α.				Α		
HCM Control Delay HCM LOS			EBLn1	WBLn1	SBLn1							
HCM Control Delay HCM LOS Lane		Α	EBLn1 11%	WBLn1 9%	62%		0.0					
HCM Control Delay HCM LOS Lane Vol Left, %		A NBLn1	_		62% 30%							
HCM Control Delay HCM LOS Lane Vol Left, % Vol Thru, %		A NBLn1 8%	11%	9%	62%							
HCM Control Delay HCM LOS Lane Vol Left, % Vol Thru, % Vol Right, %		A NBLn1 8% 78%	11% 81%	9% 54%	62% 30%							
HCM Control Delay HCM LOS Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control		A NBLn1 8% 78% 14%	11% 81% 8%	9% 54% 37%	62% 30% 8% Stop 50							
HCM Control Delay HCM LOS Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane		8% 78% 14% Stop	11% 81% 8% Stop	9% 54% 37% Stop	62% 30% 8% Stop 50 31							
HCM Control Delay HCM LOS Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol		A NBLn1 8% 78% 14% Stop 59	11% 81% 8% Stop 37	9% 54% 37% Stop 81	62% 30% 8% Stop 50							
HCM Control Delay HCM LOS Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol		A NBLn1 8% 78% 14% Stop 59 5	11% 81% 8% Stop 37 4	9% 54% 37% Stop 81	62% 30% 8% Stop 50 31							
HCM Control Delay HCM LOS Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol		A NBLn1 8% 78% 14% Stop 59 5 46	11% 81% 8% Stop 37 4 30	9% 54% 37% Stop 81 7 44	62% 30% 8% Stop 50 31							
HCM Control Delay HCM LOS Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate		A NBLn1 8% 78% 14% Stop 59 5 46 8	11% 81% 8% Stop 37 4 30 3	9% 54% 37% Stop 81 7 44 30	62% 30% 8% Stop 50 31 15							
HCM Control Delay HCM LOS Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp		A NBLn1 8% 78% 14% Stop 59 5 46 8 78	11% 81% 8% Stop 37 4 30 3	9% 54% 37% Stop 81 7 44 30 107	62% 30% 8% Stop 50 31 15 4							
HCM Control Delay HCM LOS Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X)		A NBLn1 8% 78% 14% Stop 59 5 46 8 78 1	11% 81% 8% Stop 37 4 30 3 49	9% 54% 37% Stop 81 7 44 30 107	62% 30% 8% Stop 50 31 15 4 66							
HCM Control Delay HCM LOS Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd)		A NBLn1 8% 78% 14% Stop 59 5 46 8 78 1 0.09 4.155	11% 81% 8% Stop 37 4 30 3 49 1	9% 54% 37% Stop 81 7 44 30 107 1	62% 30% 8% Stop 50 31 15 4 66 1							
HCM Control Delay HCM LOS Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N		NBLn1 8% 78% 14% Stop 59 5 46 8 78 1 0.09	11% 81% 8% Stop 37 4 30 3 49 1 0.058 4.307	9% 54% 37% Stop 81 7 44 30 107 1 0.118 3.979	62% 30% 8% Stop 50 31 15 4 66 1 0.079 4.306 Yes 819							
HCM Control Delay HCM LOS Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap		A NBLn1 8% 78% 14% Stop 59 5 46 8 78 1 0.09 4.155 Yes	11% 81% 8% Stop 37 4 30 3 49 1 0.058 4.307 Yes	9% 54% 37% Stop 81 7 44 30 107 1 0.118 3.979 Yes	62% 30% 8% Stop 50 31 15 4 66 1 0.079 4.306 Yes							
HCM Control Delay HCM LOS Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time		A NBLn1 8% 78% 14% Stop 59 5 46 8 78 1 0.09 4.155 Yes 849	11% 81% 8% Stop 37 4 30 3 49 1 0.058 4.307 Yes 836	9% 54% 37% Stop 81 7 44 30 107 1 0.118 3.979 Yes 886	62% 30% 8% Stop 50 31 15 4 66 1 0.079 4.306 Yes 819							
HCM Control Delay HCM LOS Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time HCM Lane V/C Ratio		A NBLn1 8% 78% 14% Stop 59 5 46 8 78 1 0.09 4.155 Yes 849 2.248	11% 81% 8% Stop 37 4 30 3 49 1 0.058 4.307 Yes 836 2.307	9% 54% 37% Stop 81 7 44 30 107 1 0.118 3.979 Yes 886 2.074	62% 30% 8% Stop 50 31 15 4 66 1 0.079 4.306 Yes 819 2.399							
HCM Control Delay HCM LOS Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time		NBLn1 8% 78% 14% Stop 59 5 46 8 78 1 0.09 4.155 Yes 849 2.248 0.092	11% 81% 8% Stop 37 4 30 3 49 1 0.058 4.307 Yes 836 2.307 0.059	9% 54% 37% Stop 81 7 44 30 107 1 0.118 3.979 Yes 886 2.074 0.121	62% 30% 8% Stop 50 31 15 4 66 1 0.079 4.306 Yes 819 2.399 0.081							

Internation Delay shiph								
Intersection Delay, s/veh Intersection LOS								
Movement	SBU	SBL	SBT	SBR	178 35	, arm	11	N TOWN
Vol, veh/h	0	31	15	4				
Peak Hour Factor	0.92	0.76	0.76	0.76				
Heavy Vehicles, %	2	0	8	0				
Mvmt Flow	0	41	20	5				
Number of Lanes	0	0	1	0				
Approach	1883 W	SB	11.00	18 0		N O THE		
Opposing Approach		NB						
Opposing Lanes		1						
Conflicting Approach Left		WB						
Conflicting Lanes Left		_ 1						
Conflicting Approach Right		EB						
Conflicting Lanes Right		1						
HCM Control Delay		7.8						
HCM LOS		Α						

	*	→	*	1	←	4	4	†	1	-	ļ	1
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			44			4			4	
Volume (vph)	8	19	2	10	19	31	2	18	6	5	27	5
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	14	12	12	14	12	12	14	12	12	14	12
Satd. Flow (prot)	0	1784	0	0	1683	0	0	1764	0	0	1779	0
FIt Permitted		0.986			0.992			0.997			0.993	
Satd. Flow (perm)	0	1784	0	0	1683	0	0	1764	0	0	1779	0
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		700			888			651			694	
Travel Time (s)		15.9			20.2			14.8			15.8	
Peak Hour Factor	0.81	0.81	0.81	0.81	0.81	0.81	0.81	0.81	0.81	0.81	0.81	0.81
Heavy Vehicles (%)	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	35	0	0	73	0	0	31	0	0	45	0
Sign Control		Stop			Stop			Stop			Stop	

Area Type:

CBD

Control Type: Unsignalized

Intersection Capacity Utilization 14.6%

Analysis Period (min) 15

Intersection												
Intersection Delay, s/veh	7.2											
Intersection LOS	Α											
Movement	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBF
Vol, veh/h	0	8	19	2	0	10	19	31	0	2	18	0.0
Peak Hour Factor	0.92	0.81	0.81	0.81	0.92	0.81	0.81	0.81	0.92	0.81	0.81	0.8
Heavy Vehicles, %	2	0	0	0	2	0	0	0	2	0	0	
Mymt Flow	0	10	23	2	0	12	23	38	0	2	22	
Number of Lanes	0	0	1	0	0	0	1	0	0	0	1	(
Approach		EB		=		WB				NB		
Opposing Approach		WB				EB				SB		
Opposing Lanes		1				1				1		
Conflicting Approach Left		SB				NB				EB		
Conflicting Lanes Left		1				1				1		
Conflicting Approach Right		NB				SB				WB		
Conflicting Lanes Right		1				1				1		
						7.2				7.2		
		7.3										
HCM Control Delay HCM LOS		7.3 A				Α.Α				Α		
HCM Control Delay												
HCM Control Delay			EBLn1	WBLn1	SBLn1							
HCM Control Delay HCM LOS Lane	- 0	A NBLn1 8%	28%	17%	14%							
HCM Control Delay HCM LOS		A NBLn1 8% 69%	28% 66%	17% 32%	14% 73%							-
HCM Control Delay HCM LOS Lane Vol Left, % Vol Thru, %		A NBLn1 8%	28% 66% 7%	17% 32% 52%	14% 73% 14%							
HCM Control Delay HCM LOS Lane Vol Left, % Vol Thru, % Vol Right, %	- 0	A NBLn1 8% 69%	28% 66% 7% Stop	17% 32% 52% Stop	14% 73% 14% Stop							
HCM Control Delay HCM LOS Lane Vol Left, % Vol Thru, %		A NBLn1 8% 69% 23% Stop 26	28% 66% 7% Stop 29	17% 32% 52% Stop 60	14% 73% 14% Stop 37							
HCM Control Delay HCM LOS Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control		A NBLn1 8% 69% 23% Stop 26 2	28% 66% 7% Stop 29	17% 32% 52% Stop 60 10	14% 73% 14% Stop 37 5							
HCM Control Delay HCM LOS Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT. Vol		A NBLn1 8% 69% 23% Stop 26 2 18	28% 66% 7% Stop 29 8 19	17% 32% 52% Stop 60 10	14% 73% 14% Stop 37 5 27							
HCM Control Delay HCM LOS Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane		A NBLn1 8% 69% 23% Stop 26 2 18 6	28% 66% 7% Stop 29 8 19	17% 32% 52% Stop 60 10 19 31	14% 73% 14% Stop 37 5 27							
HCM Control Delay HCM LOS Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol		A NBLn1 8% 69% 23% Stop 26 2 18	28% 66% 7% Stop 29 8 19	17% 32% 52% Stop 60 10	14% 73% 14% Stop 37 5 27 5 46							
HCM Control Delay HCM LOS Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate		A NBLn1 8% 69% 23% Stop 26 2 18 6	28% 66% 7% Stop 29 8 19	17% 32% 52% Stop 60 10 19 31 74	14% 73% 14% Stop 37 5 27 5 46							
HCM Control Delay HCM LOS Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp		A NBLn1 8% 69% 23% Stop 26 2 18 6 32	28% 66% 7% Stop 29 8 19 2 36 1	17% 32% 52% Stop 60 10 19 31 74 1	14% 73% 14% Stop 37 5 27 5 46 1							
HCM Control Delay HCM LOS Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X)		A NBLn1 8% 69% 23% Stop 26 2 18 6 32 1	28% 66% 7% Stop 29 8 19 2 36	17% 32% 52% Stop 60 10 19 31 74	14% 73% 14% Stop 37 5 27 5 46 1 0.052 4.062							
HCM Control Delay HCM LOS Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd)		NBLn1 8% 69% 23% Stop 26 2 18 6 32 1 0.036	28% 66% 7% Stop 29 8 19 2 36 1	17% 32% 52% Stop 60 10 19 31 74 1	14% 73% 14% Stop 37 5 27 5 46 1 0.052 4.062 Yes							
HCM Control Delay HCM LOS Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N		A NBLn1 8% 69% 23% Stop 26 2 18 6 32 1 0.036 4.003	28% 66% 7% Stop 29 8 19 2 36 1 0.041 4.107	17% 32% 52% Stop 60 10 19 31 74 1 0.078 3.787 Yes 941	14% 73% 14% Stop 37 5 27 5 46 1 0.052 4.062 Yes 877							
HCM Control Delay HCM LOS Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap		A NBLn1 8% 69% 23% Stop 26 2 18 6 32 1 0.036 4.003 Yes	28% 66% 7% Stop 29 8 19 2 36 1 0.041 4.107 Yes	17% 32% 52% Stop 60 10 19 31 74 1 0.078 3.787 Yes	14% 73% 14% Stop 37 5 27 5 46 1 0.052 4.062 Yes 877 2.109							
HCM Control Delay HCM LOS Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time		A NBLn1 8% 69% 23% Stop 26 2 18 6 32 1 0.036 4.003 Yes 888	28% 66% 7% Stop 29 8 19 2 36 1 0.041 4.107 Yes 867	17% 32% 52% Stop 60 10 19 31 74 1 0.078 3.787 Yes 941	14% 73% 14% Stop 37 5 27 5 46 1 0.052 4.062 Yes 877 2.109 0.052							
HCM Control Delay HCM LOS Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time HCM Lane V/C Ratio		NBLn1 8% 69% 23% Stop 26 2 18 6 32 1 0.036 4.003 Yes 888 2.054	28% 66% 7% Stop 29 8 19 2 36 1 0.041 4.107 Yes 867 2.153	17% 32% 52% Stop 60 10 19 31 74 1 0.078 3.787 Yes 941 1.83	14% 73% 14% Stop 37 5 27 5 46 1 0.052 4.062 Yes 877 2.109							
HCM Control Delay HCM LOS Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time		NBLn1 8% 69% 23% Stop 26 2 18 6 32 1 0.036 4.003 Yes 888 2.054 0.036	28% 66% 7% Stop 29 8 19 2 36 1 0.041 4.107 Yes 867 2.153 0.042	17% 32% 52% Stop 60 10 19 31 74 1 0.078 3.787 Yes 941 1.83 0.079	14% 73% 14% Stop 37 5 27 5 46 1 0.052 4.062 Yes 877 2.109 0.052							

Intersection Delay s/yeb					
Intersection Delay, s/veh Intersection LOS					
Movement	SBU	SBL	SBT	SBR	
Vol, veh/h	0	5	27	5	
Peak Hour Factor	0.92	0.81	0.81	0.81	
Heavy Vehicles, %	2	0	0	0	
Mvmt Flow	0	6	33	6	
Number of Lanes	0	0	1	0	
Approach		SB	Maria II		
Opposing Approach		NB			
Opposing Lanes		1			
Conflicting Approach Left		WB			
Conflicting Lanes Left		1			
Conflicting Approach Right		EB			
Conflicting Lanes Right		1			
HCM Control Delay		7.3			
HCM LOS		Α			

	٠	→	*	1	←	*	4	†	-	-	+	1
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Volume (vph)	4	30	5	7	44	30	5	54	8	31	17	4
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	14	12	12	14	12	12	14	12	12	14	12
Satd. Flow (prot)	0	1780	0	0	1653	0	0	1786	0	0	1709	0
Flt Permitted		0.995			0.996			0.996			0.971	
Satd. Flow (perm)	0	1780	0	0	1653	0	0	1786	0	0	1709	0
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		700			437			211			694	
Travel Time (s)		15.9			9.9			4.8			15.8	
Peak Hour Factor	0.76	0.76	0.76	0.76	0.76	0.76	0.76	0.76	0.76	0.76	0.76	0.76
Heavy Vehicles (%)	0%	0%	0%	0%	8%	0%	0%	0%	0%	0%	8%	0%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	51	0	0	106	0	0	89	0	0	68	0
Sign Control		Stop			Stop			Stop			Stop	
Intersection Summary					1 7			-				

CBD

Control Type: Unsignalized

Intersection Capacity Utilization 22.9%

Analysis Period (min) 15

Intersection												
Intersection Delay, s/veh Intersection LOS	7.7 A											
Movement	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBR
Vol, veh/h	0	4	30	5	0	7	44	30	0	5	54	8
Peak Hour Factor	0.92	0.76	0.76	0.76	0.92	0.76	0.76	0.76	0.92	0.76	0.76	0.76
Heavy Vehicles, %	2	0	0	0	2	0	8	0	2	0	0	0
Mvmt Flow	0	5	39	7	0	9	58	39	0	7	71	11
Number of Lanes	0	0	1	0	0	0	1	0	0	0	1	0
Approach	- 14	EB	00		-	WB			200	NB		
Opposing Approach		WB				EB				SB		
Opposing Lanes		1				1				1		
Conflicting Approach Left		SB				NB				EB		
Conflicting Lanes Left		1				1				1		
Conflicting Approach Right		NB				SB				WB		
Conflicting Lanes Right		1				1				1		
HCM Control Delay		7.6				7.7				7.7		
		Α				Α				Α		
HCM LOS						Α				А		
	:	A NBLn1	EBLn1	WBLn1	SBLn1	A				А	1.3	
HCM LOS	:	A NBLn1 7%	10%	9%	60%	А				A	1.3	
HCM LOS	:	A NBLn1 7% 81%	10% 77%	9% 54%	60% 33%	A		0		A	7/-3	
Lane Vol Left, % Vol Thru, %	:	A NBLn1 7%	10% 77% 13%	9% 54% 37%	60% 33% 8%	A				A	1.3	
Lane Vol Left, % Vol Thru, % Vol Right, %	:	A NBLn1 7% 81% 12% Stop	10% 77% 13% Stop	9% 54% 37% Stop	60% 33% 8% Stop	A				A	1-2	
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control		A NBLn1 7% 81% 12% Stop 67	10% 77% 13%	9% 54% 37% Stop 81	60% 33% 8% Stop 52	A				A		
Lane Vol Left, % Vol Thru, % Vol Right, %		A NBLn1 7% 81% 12% Stop 67 5	10% 77% 13% Stop 39	9% 54% 37% Stop 81	60% 33% 8% Stop 52 31	A				A		
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane		A NBLn1 7% 81% 12% Stop 67 5 54	10% 77% 13% Stop 39 4 30	9% 54% 37% Stop 81 7	60% 33% 8% Stop 52 31 17	A				A	70-2	
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol		A NBLn1 7% 81% 12% Stop 67 5 54 8	10% 77% 13% Stop 39 4 30 5	9% 54% 37% Stop 81 7 44 30	60% 33% 8% Stop 52 31 17 4	A				A	7(=2)	
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol		A NBLn1 7% 81% 12% Stop 67 5 54	10% 77% 13% Stop 39 4 30	9% 54% 37% Stop 81 7 44 30 107	60% 33% 8% Stop 52 31 17 4 68	A				A	7(-2)	
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate	: .	A NBLn1 7% 81% 12% Stop 67 5 54 8	10% 77% 13% Stop 39 4 30 5	9% 54% 37% Stop 81 7 44 30 107	60% 33% 8% Stop 52 31 17 4 68	A				A	7 -2	
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp	:	A NBLn1 7% 81% 12% Stop 67 54 8 88	10% 77% 13% Stop 39 4 30 5 51 1	9% 54% 37% Stop 81 7 44 30 107 1	60% 33% 8% Stop 52 31 17 4 68 1	A				A	70 - 20	
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X)		A NBLn1 7% 81% 12% Stop 67 5 54 8 88 1	10% 77% 13% Stop 39 4 30 5	9% 54% 37% Stop 81 7 44 30 107 1 0.119 4.105	60% 33% 8% Stop 52 31 17 4 68 1 0.082 4.317	A				A	7 -2	
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd)		A NBLn1 7% 81% 12% Stop 67 5 54 8 88 1 0.102	10% 77% 13% Stop 39 4 30 5 51 1	9% 54% 37% Stop 81 7 44 30 107 1 0.119 4.105 Yes	60% 33% 8% Stop 52 31 17 4 68 1 0.082 4.317 Yes	A				A		
HCM LOS Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N		A NBLn1 7% 81% 12% Stop 67 5 54 8 88 1 0.102 4.171	10% 77% 13% Stop 39 4 30 5 51 1 0.061 4.308	9% 54% 37% Stop 81 7 44 30 107 1 0.119 4.105 Yes 878	60% 33% 8% Stop 52 31 17 4 68 1 0.082 4.317 Yes 816	A				A		
HCM LOS Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap		A NBLn1 7% 81% 12% Stop 67 5 54 8 88 1 0.102 4.171 Yes	10% 77% 13% Stop 39 4 30 5 51 1 0.061 4.308 Yes	9% 54% 37% Stop 81 7 44 30 107 1 0.119 4.105 Yes	60% 33% 8% Stop 52 31 17 4 68 1 0.082 4.317 Yes 816 2.415	A				A		
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time		A NBLn1 7% 81% 12% Stop 67 5 54 8 88 1 0.102 4.171 Yes 845	10% 77% 13% Stop 39 4 30 5 51 1 0.061 4.308 Yes 836	9% 54% 37% Stop 81 7 44 30 107 1 0.119 4.105 Yes 878	60% 33% 8% Stop 52 31 17 4 68 1 0.082 4.317 Yes 816 2.415 0.083	A				A		
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time HCM Lane V/C Ratio		A NBLn1 7% 81% 12% Stop 67 54 8 88 1 0.102 4.171 Yes 845 2.266	10% 77% 13% Stop 39 4 30 5 51 1 0.061 4.308 Yes 836 2.308	9% 54% 37% Stop 81 7 44 30 107 1 0.119 4.105 Yes 878 2.105	60% 33% 8% Stop 52 31 17 4 68 1 0.082 4.317 Yes 816 2.415	A				A		
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time		A NBLn1 7% 81% 12% Stop 67 54 8 88 1 0.102 4.171 Yes 845 2.266 0.104	10% 77% 13% Stop 39 4 30 5 51 1 0.061 4.308 Yes 836 2.308 0.061	9% 54% 37% Stop 81 7 44 30 107 1 0.119 4.105 Yes 878 2.105 0.122	60% 33% 8% Stop 52 31 17 4 68 1 0.082 4.317 Yes 816 2.415 0.083	A				A		

Intersection Delay, s/veh												
Intersection LOS												
Movement	SBU	SBL	SBT	SBR	110			1250P	EV 181		(No. 1)	
Vol, veh/h	0	31	17	4								
Peak Hour Factor	0.92	0.76	0.76	0.76								
Heavy Vehicles, %	2	0	8	0								
Mymt Flow	0	41	22	5								
Number of Lanes	0	0	1	0								
Approach	1750/1	SB		Jan Salar		aVI III o			Tion III	WE SHIP		B
Opposing Approach		NB										
Opposing Lanes		1										
Conflicting Approach Left		WB										
Conflicting Lanes Left		1										
Conflicting Approach Right		EB										
Conflicting Lanes Right		1										
HCM Control Delay		7.8										
HCM LOS		A										

	٠	-	*	1	-	*	4	†	-	-	1	1
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Volume (vph)	8	19	7	10	19	31	2	25	6	5	32	5
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	14	12	12	14	12	12	14	12	12	14	12
Satd. Flow (prot)	0	1750	0	0	1683	0	0	1777	0	0	1784	0
FIt Permitted		0.988			0.992			0.998			0.994	
Satd. Flow (perm)	0	1750	0	0	1683	0	0	1777	0	0	1784	0
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		700			450			211			694	
Travel Time (s)		15.9			10.2			4.8			15.8	
Peak Hour Factor	0.81	0.81	0.81	0.81	0.81	0.81	0.81	0.81	0.81	0.81	0.81	0.81
Heavy Vehicles (%)	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%
Shared Lane Traffic (%)	070	0.0										
Lane Group Flow (vph)	0	42	0	0	73	0	0	40	0	0	52	0
Sign Control	. 0	Stop			Stop			Stop			Stop	
Intersection Summary										- 0.4		

Anna Tunai

Area Type:

CBD

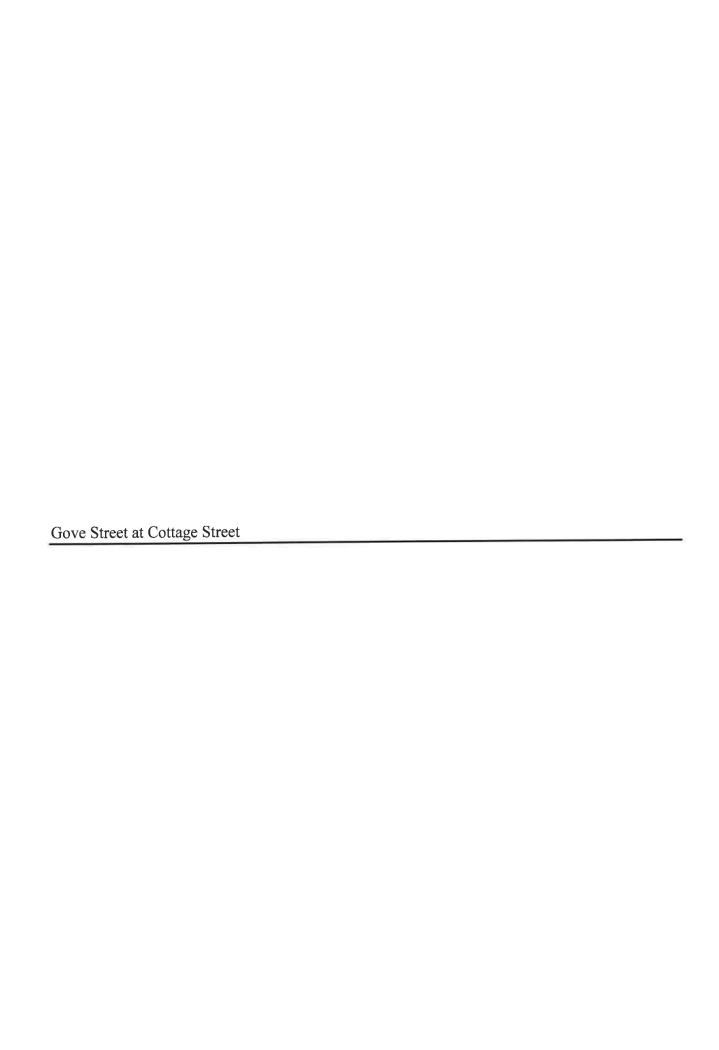
Control Type: Unsignalized

Intersection Capacity Utilization 15.3%

Analysis Period (min) 15

Intersection Delay, s/veh	7.3											
Intersection LOS	Α											
Movement	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBF
Vol, veh/h	0	8	19	7	0	10	19	31	0	2	25	6
Peak Hour Factor	0.92	0.81	0.81	0.81	0.92	0.81	0.81	0.81	0.92	0.81	0.81	0.8
Heavy Vehicles, %	2	0	0	0	2	0	0	0	2	0	0	(
Mvmt Flow	0	10	23	9	0	12	23	38	0	2	31	7
Number of Lanes	0	0	1	0	0	0	1	0	0	0	1	(
Approach	2007	EB			33	WB	200			NB		
Opposing Approach		WB				EB				SB		
Opposing Lanes		1				.1				1		
Conflicting Approach Left		SB				NB				EB		
Conflicting Lanes Left	*,	1				1				1		
Conflicting Approach Right	\$.	NB				SB				WB		
Conflicting Lanes Right		1				1				1		
HCM Control Delay		7.3				7.2				7.3		
HCM LOS		Α				Α				A		
TIOM LOO												
	-	NBLn1	EBLn1	WBLn1	SBLn1							
Lane		INDEILL	his her hast 1. 1	AADEILI	ODLIT							
Lane Vol Left. %		6%	24%	17%	12%							
Vol Left, %					12% 76%							
Vol Left, % Vol Thru, %		6%	24%	17%	12%							
Vol Left, % Vol Thru, % Vol Right, %		6% 76%	24% 56%	17% 32%	12% 76% 12% Stop							
Vol Left, % Vol Thru, % Vol Right, % Sign Control		6% 76% 18%	24% 56% 21%	17% 32% 52%	12% 76% 12%							
Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane		6% 76% 18% Stop	24% 56% 21% Stop	17% 32% 52% Stop	12% 76% 12% Stop							
Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol		6% 76% 18% Stop 33	24% 56% 21% Stop 34	17% 32% 52% Stop 60	12% 76% 12% Stop 42							
Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol		6% 76% 18% Stop 33 2	24% 56% 21% Stop 34 8	17% 32% 52% Stop 60 10	12% 76% 12% Stop 42 5 32							
Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol		6% 76% 18% Stop 33 2	24% 56% 21% Stop 34 8	17% 32% 52% Stop 60 10	12% 76% 12% Stop 42 5							
Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate		6% 76% 18% Stop 33 2 25 6	24% 56% 21% Stop 34 8 19	17% 32% 52% Stop 60 10 19	12% 76% 12% Stop 42 5 32							
Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp		6% 76% 18% Stop 33 2 25 6	24% 56% 21% Stop 34 8 19 7	17% 32% 52% Stop 60 10 19 31 74	12% 76% 12% Stop 42 5 32 5							
Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X)		6% 76% 18% Stop 33 2 25 6 41	24% 56% 21% Stop 34 8 19 7 42	17% 32% 52% Stop 60 10 19 31 74	12% 76% 12% Stop 42 5 32 5 52							
Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd)		6% 76% 18% Stop 33 2 25 6 41 1	24% 56% 21% Stop 34 8 19 7 42 1	17% 32% 52% Stop 60 10 19 31 74 1 0.078	12% 76% 12% Stop 42 5 32 5 52 1 0.059							
Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N		6% 76% 18% Stop 33 2 25 6 41 1 0.046 4.043	24% 56% 21% Stop 34 8 19 7 42 1 0.047 4.04	17% 32% 52% Stop 60 10 19 31 74 1 0.078 3.815	12% 76% 12% Stop 42 5 32 5 52 1 0.059 4.084 Yes 871							
Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap		6% 76% 18% Stop 33 2 25 6 41 1 0.046 4.043 Yes	24% 56% 21% Stop 34 8 19 7 42 1 0.047 4.04 Yes	17% 32% 52% Stop 60 10 19 31 74 1 0.078 3.815 Yes	12% 76% 12% Stop 42 5 32 5 52 1 0.059 4.084 Yes							
Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time		6% 76% 18% Stop 33 2 25 6 41 1 0.046 4.043 Yes 879	24% 56% 21% Stop 34 8 19 7 42 1 0.047 4.04 Yes 879	17% 32% 52% Stop 60 10 19 31 74 1 0.078 3.815 Yes 932	12% 76% 12% Stop 42 5 32 5 52 1 0.059 4.084 Yes 871							
Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time HCM Lane V/C Ratio		6% 76% 18% Stop 33 2 25 6 41 1 0.046 4.043 Yes 879 2.098	24% 56% 21% Stop 34 8 19 7 42 1 0.047 4.04 Yes 879 2.096	17% 32% 52% Stop 60 10 19 31 74 1 0.078 3.815 Yes 932 1.868	12% 76% 12% Stop 42 5 32 5 52 1 0.059 4.084 Yes 871 2.136							
Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time		6% 76% 18% Stop 33 2 25 6 41 1 0.046 4.043 Yes 879 2.098 0.047	24% 56% 21% Stop 34 8 19 7 42 1 0.047 4.04 Yes 879 2.096 0.048	17% 32% 52% Stop 60 10 19 31 74 1 0.078 3.815 Yes 932 1.868 0.079	12% 76% 12% Stop 42 5 32 5 52 1 0.059 4.084 Yes 871 2.136 0.06							

Intersection	I Autoria	Here	S Wall	Ap Show 15.	AL WEST	1 1 1 - 1 - 1	eiti ji er 8	- 1/0 10	TOTAL.	Wichs In the St
Intersection Delay, s/veh										
Movement	SBU	SBL	SBT	SBR		11 77 5	S' Jo this	E 4881	1/13	旅游主息 经证明
Vol, veh/h	0	5	32	5						
Peak Hour Factor	0.92	0.81	0.81	0.81						
Heavy Vehicles, %	2	0	0	0						
Mymt Flow	0	6	40	6						
Number of Lanes	0	0	1	0						
Approach	NOTE TO BE	SB				(Ush)	THE STATE	Tail of		
Opposing Approach		NB								
Opposing Lanes		1								
Conflicting Approach Left		WB								
Conflicting Lanes Left		1								
Conflicting Approach Right		EB								
Conflicting Lanes Right		1								
HCM Control Delay		7.4								
HCM LOS		Α								
Lane	SOCIETY VI	- FOR W	West State	K KATE	T TO WHAT	(Auto/68)	A A	11 20 20	Stor 8	10 120 KG 1 1 1



8: Cottage	Street	&	Gove	Street

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		12			4						4	
Volume (vph)	0	3	54	1	12	0	0	0	0	15	252	17
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	14	12	12	14	12	12	12	12	12	14	12
Satd. Flow (prot)	0	1561	0	0	1817	0	0	0	0	0	1788	0
Flt Permitted					0.996						0.997	
Satd. Flow (perm)	0	1561	0	0	1817	0	0	0	0	0	1788	0
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		888			309			587			677	
Travel Time (s)		20.2			7.0			13.3			15.4	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	0%	2%	0%	0%	0%	2%	2%	2%	0%	1%	0%
Shared Lane Traffic (%)	0.10	0 10		707								
Lane Group Flow (vph)	0	62	0	0	14	0	0	0	0	0	308	0
Sign Control		Stop			Stop			Stop			Stop	

Area Type:

CBD

Control Type: Unsignalized

Intersection Capacity Utilization 27.4%

ICU Level of Service A

Intersection Delay shiph	8.8											
ntersection Delay, s/veh	Α											
		rm)	EDT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBF
Movement	EBU	EBL	EBT 3	54	0 0	1	12	0	0	0	0	(
Vol, veh/h	0	0	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.9
Peak Hour Factor	0.92	0.92		2	2	0.32	0.52	0	2	2	2	
Heavy Vehicles, %	2	0	0	59	0	1	13	0	0	ō	0	
Mvmt Flow	0	0	3 1	0	0	0	1	0	0	0	0	
Number of Lanes	0	0		U	U	U		Ū	Ů,			
Approach			EB		116	WB						
Opposing Approach			WB			EB						
Opposing Lanes			1			1						
Conflicting Approach Left			SB									
Conflicting Lanes Left			1			0						
Conflicting Approach Right						SB						
Conflicting Lanes Right			.0			1						
HCM Control Delay			7.3			7.7						
ncivi control Delay												
HCM LOS			Α			Α						
						Α						
		EBLn1	A WBLn1	SBLn1		A						
HCM LOS		0%	A WBLn1 8%	5%		A						
HCM LOS		0% 5%	A WBLn1 8% 92%	5% 89%		A						
Lane Vol Left, %		0% 5% 95%	A WBLn1 8% 92% 0%	5% 89% 6%		A						
Lane Vol Left, % Vol Thru, %		0% 5% 95% Stop	A WBLn1 8% 92% 0% Stop	5% 89% 6% Stop		A						
Lane Vol Left, % Vol Thru, % Vol Right, %		0% 5% 95% Stop 57	A WBLn1 8% 92% 0% Stop 13	5% 89% 6% Stop 284		A						
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control		0% 5% 95% Stop 57	A WBLn1 8% 92% 0% Stop 13 1	5% 89% 6% Stop 284 15		A						
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane		0% 5% 95% Stop 57 0	A WBLn1 8% 92% 0% Stop 13 1	5% 89% 6% Stop 284 15 252		A						
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol		0% 5% 95% Stop 57 0 3 54	A WBLn1 8% 92% 0% Stop 13 1 12 0	5% 89% 6% Stop 284 15 252		A						
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol		0% 5% 95% Stop 57 0 3 54 62	A WBLn1 8% 92% 0% Stop 13 1 12 0 14	5% 89% 6% Stop 284 15 252 17 309		A						
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol		0% 5% 95% Stop 57 0 3 54 62 1	A WBLn1 8% 92% 0% Stop 13 1 12 0 14 1	5% 89% 6% Stop 284 15 252 17 309		A						
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate		0% 5% 95% Stop 57 0 3 54 62 1 0.069	MBLn1 8% 92% 0% Stop 13 1 12 0 14 1 0.018	5% 89% 6% Stop 284 15 252 17 309 1		A						
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd)		0% 5% 95% Stop 57 0 3 54 62 1 0.069 4.024	A WBLn1 8% 92% 0% Stop 13 1 12 0 14 1 0.018 4.66	5% 89% 6% Stop 284 15 252 17 309 1 0.344 4.007		A						
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X)		0% 5% 95% Stop 57 0 3 54 62 1 0.069 4.024 Yes	MBLn1 8% 92% 0% Stop 13 1 12 0 14 1 0.018 4.66 Yes	5% 89% 6% Stop 284 15 252 17 309 1 0.344 4.007 Yes		A						
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd)		0% 5% 95% Stop 57 0 3 54 62 1 0.069 4.024 Yes 895	MBLn1 8% 92% 0% Stop 13 1 12 0 14 1 0.018 4.66 Yes 772	5% 89% 6% Stop 284 15 252 17 309 1 0.344 4.007 Yes 895		A						
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N		0% 5% 95% Stop 57 0 3 54 62 1 0.069 4.024 Yes 895 2.024	MBLn1 8% 92% 0% Stop 13 1 12 0 14 1 0.018 4.66 Yes 772 2.662	5% 89% 6% Stop 284 15 252 17 309 1 0.344 4.007 Yes 895 2.044		A						
HCM LOS Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap		0% 5% 95% Stop 57 0 3 54 62 1 0.069 4.024 Yes 895 2.024 0.069	MBLn1 8% 92% 0% Stop 13 1 12 0 14 1 0.018 4.66 Yes 772 2.662 0.018	5% 89% 6% Stop 284 15 252 17 309 1 0.344 4.007 Yes 895 2.044 0.345		A						
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time		0% 5% 95% Stop 57 0 3 54 62 1 0.069 4.024 Yes 895 2.024 0.069 7.3	MBLn1 8% 92% 0% Stop 13 1 12 0 14 1 0.018 4.66 Yes 772 2.662 0.018 7.7	5% 89% 6% Stop 284 15 252 17 309 1 0.344 4.007 Yes 895 2.044 0.345 9.1		A						
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time HCM Lane V/C Ratio		0% 5% 95% Stop 57 0 3 54 62 1 0.069 4.024 Yes 895 2.024 0.069	MBLn1 8% 92% 0% Stop 13 1 12 0 14 1 0.018 4.66 Yes 772 2.662 0.018	5% 89% 6% Stop 284 15 252 17 309 1 0.344 4.007 Yes 895 2.044 0.345		A						

Intersection	War and Sa	A BANK TA	1,00		
Intersection Delay, s/veh Intersection LOS					
Movement	SBU	SBL	SBT	SBR	
Vol, veh/h	0	15	252	17	
Peak Hour Factor	0.92	0.92	0.92	0.92	
Heavy Vehicles, %	2	0	1	0	
Mvmt Flow	0	16	274	18	
Number of Lanes	0	0	1	0	
Approach	Wiles H	SB	Total Control	THE S	
Opposing Approach					
Opposing Lanes		0			
Conflicting Approach Left		WB			
Conflicting Lanes Left		1			
Conflicting Approach Right		EB			
Conflicting Lanes Right		1			
HCM Control Delay		9.1			
		Α			

8: Cottage Street & Gove Street

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		1>			4						4	
Volume (vph)	0	3	22	- 5	10	0	0	0	0	19	166	21
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	14	12	12	14	12	12	12	12	12	14	12
Satd. Flow (prot)	0	1605	0	0	1797	0	0	0	0	0	1775	0
Flt Permitted					0.985						0.995	
Satd. Flow (perm)	0	1605	0	0	1797	0	0	0	0	0	1775	0
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		888			309			587			677	
Travel Time (s)		20.2			7.0			13.3			15.4	
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Heavy Vehicles (%)	0%	0%	0%	0%	0%	0%	2%	2%	2%	0%	1%	0%
Shared Lane Traffic (%)	77.50											
Lane Group Flow (vph)	0	27	0	0	16	0	0	0	0	0	221	0
Sign Control		Stop			Stop			Stop			Stop	

Intersection Summary

Area Type:

CBD

Control Type: Unsignalized

Intersection Capacity Utilization 24.5%

Analysis Period (min) 15

Intersection											36.6	
Intersection Delay, s/veh	8											
Intersection LOS	: A											
Movement	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBF
Vol, veh/h	0	0	3	22	0	5	10	0	0	0	0	0.00
Peak Hour Factor	0.92	0.93	0.93	0.93	0.92	0.93	0.93	0.93	0.92	0.93	0.93	0.9
Heavy Vehicles, %	2	0	0	0	2	0	0	0	2	2	2	
Mvmt Flow	0	0	3	24	0	5	11	0	0	0	0	
Number of Lanes	0	0	1	0	0	0	1	0	0	0	0	
Approach			EB	-		WB			5			
Opposing Approach			WB			EB						
Opposing Lanes			1			1						
Conflicting Approach Left			SB									
Conflicting Lanes Left			1			0						
Conflicting Approach Right						SB						
Conflicting Lanes Right			0			1						
			7			7.6						
HCM Control Delay												
HCM Control Delay HCM LOS			Α			Α						
			Α			А						
HCM LOS Lane		EBLn1	A WBLn1	SBLn1		Α	3 3					
Lane Vol Left, %		0%	A WBLn1 33%	9%		A	3 3					
Lane Vol Left, % Vol Thru, %		0% 1 2%	A WBLn1 33% 67%	9% 81%		Α	3 1					
HCM LOS Lane Vol Left, % Vol Thru, % Vol Right, %		0% 12% 88%	A WBLn1 33% 67% 0%	9% 81% 10%		A						
HCM LOS Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control		0% 12% 88% Stop	A WBLn1 33% 67% 0% Stop	9% 81% 10% Stop		A						
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane		0% 12% 88% Stop 25	WBLn1 33% 67% 0% Stop 15	9% 81% 10% Stop 206		A						
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol		0% 12% 88% Stop 25 0	A WBLn1 33% 67% 0% Stop 15 5	9% 81% 10% Stop 206 19		A						
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol		0% 12% 88% Stop 25 0	A WBLn1 33% 67% 0% Stop 15 5 10	9% 81% 10% Stop 206 19		A						
HCM LOS Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol		0% 12% 88% Stop 25 0 3 22	A WBLn1 33% 67% 0% Stop 15 5 10 0	9% 81% 10% Stop 206 19 166 21		A						
HCM LOS Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate		0% 12% 88% Stop 25 0 3 22 27	A WBLn1 33% 67% 0% Stop 15 5 10 0 16	9% 81% 10% Stop 206 19 166 21 222		A						
HCM LOS Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp	<u>-</u>	0% 12% 88% Stop 25 0 3 22 27	A WBLn1 33% 67% 0% Stop 15 5 10 0 16 1	9% 81% 10% Stop 206 19 166 21 222		A						
HCM LOS Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X)		0% 12% 88% Stop 25 0 3 22 27 1 0.028	A WBLn1 33% 67% 0% Stop 15 5 10 0 16 1 0.02	9% 81% 10% Stop 206 19 166 21 222 1		A						
HCM LOS Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp		0% 12% 88% Stop 25 0 3 22 27 1 0.028 3.771	A WBLn1 33% 67% 0% Stop 15 5 10 0 16 1 0.02 4.377	9% 81% 10% Stop 206 19 166 21 222 1 0.242 3.931		A						
HCM LOS Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X)		0% 12% 88% Stop 25 0 3 22 27 1 0.028 3.771 Yes	A WBLn1 33% 67% 0% Stop 15 5 10 0 16 1 0.02 4.377 Yes	9% 81% 10% Stop 206 19 166 21 222 1 0.242 3.931 Yes		A						
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd)		0% 12% 88% Stop 25 0 3 22 27 1 0.028 3.771 Yes 932	A WBLn1 33% 67% 0% Stop 15 5 10 0 16 1 0.02 4.377 Yes 806	9% 81% 10% Stop 206 19 166 21 222 1 0.242 3.931 Yes 915		A						
HCM LOS Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N		0% 12% 88% Stop 25 0 3 22 27 1 0.028 3.771 Yes 932 1.865	A WBLn1 33% 67% 0% Stop 15 5 10 0 16 1 0.02 4.377 Yes 806 2.47	9% 81% 10% Stop 206 19 166 21 222 1 0.242 3.931 Yes 915 1.951		A						
HCM LOS Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap	1	0% 12% 88% Stop 25 0 3 22 27 1 0.028 3.771 Yes 932	A WBLn1 33% 67% 0% Stop 15 5 10 0 16 1 0.02 4.377 Yes 806 2.47 0.02	9% 81% 10% Stop 206 19 166 21 222 1 0.242 3.931 Yes 915 1.951 0.243		A						
HCM LOS Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time		0% 12% 88% Stop 25 0 3 22 27 1 0.028 3.771 Yes 932 1.865	A WBLn1 33% 67% 0% Stop 15 5 10 0 16 1 0.02 4.377 Yes 806 2.47 0.02 7.6	9% 81% 10% Stop 206 19 166 21 222 1 0.242 3.931 Yes 915 1.951 0.243 8.2		A						
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time HCM Lane V/C Ratio		0% 12% 88% Stop 25 0 3 22 27 1 0.028 3.771 Yes 932 1.865 0.029	A WBLn1 33% 67% 0% Stop 15 5 10 0 16 1 0.02 4.377 Yes 806 2.47 0.02	9% 81% 10% Stop 206 19 166 21 222 1 0.242 3.931 Yes 915 1.951 0.243		A						

ntersection Delay, s/veh					
ntersection LOS					
Movement	SBU	SBL	SBT	SBR	
/ol, veh/h	0	19	166	21	
Peak Hour Factor	0.92	0.93	0.93	0.93	
Heavy Vehicles, %	2	0	1	0	
Mvmt Flow	0	20	178	23	
Number of Lanes	0	0	1	0	
Approach	3X 9 4	SB		THE PARTY OF	
Opposing Approach					
Opposing Lanes		0			
Conflicting Approach Left		WB			
Conflicting Lanes Left		1			
Conflicting Approach Right		EB			
Conflicting Lanes Right		1			
HCM Control Delay		8.2			
HCM LOS		Α			

g.	Cottage	Street	ጲ	Gove	Street
Ο.	Collage	Ollect	OX.	COVC	Olloct

	•	→	7	1	←	4	4	†	1	1	↓	1
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		1>			4						4	
Volume (vph)	0	3	58	1	13	0	0	0	0	16	271	18
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	14	12	12	14	12	12	12	12	12	14	12
Satd. Flow (prot)	0	1559	0	0	1819	0	0	0	0	0	1788	0
FIt Permitted					0.997						0.997	
Satd. Flow (perm)	0	1559	0	0	1819	0	0	0	0	0	1788	0
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		888			309			587			677	
Travel Time (s)		20.2			7.0			13.3			15.4	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	0%	2%	0%	0%	0%	2%	2%	2%	0%	1%	0%
Shared Lane Traffic (%)	0.10	7.77										
Lane Group Flow (vph)	0	66	0	0	15	0	0	0	0	0	332	0
Sign Control		Stop			Stop			Stop			Stop	

Area Type:

CBD

Control Type: Unsignalized

Intersection Capacity Utilization 28.9%

Analysis Period (min) 15

ntersection												
ntersection Delay, s/veh	9											
ntersection LOS	Α								A1 A27-4 A1 A1 A	200-00		
Movement	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBF
Vol, veh/h	0	0	3	58	0	1	13	0	0	0	0	0.00
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles, %	2	0	0	2	2	0	0	0	2	2	2	2
Mvmt Flow	0	0	3	63	0	1	14	0	0	0	0	(
Number of Lanes	0	0	1	0	0	0	1	0	0	0	0	(
Approach			EB			WB						
Opposing Approach			WB			EB						
Opposing Lanes			1			1						
Conflicting Approach Left			SB									
Conflicting Lanes Left			1			0						
Conflicting Approach Right						SB						
Conflicting Lanes Right			0			1						
			7.4			7.8						
HUNLLODITOLDEIAV												
HCM Control Delay HCM LOS			Α			Α						
HCM Control Delay HCM LOS												
		EBLn1	A WBLn1	SBLn1								
HCM LOS		0%	A WBLn1 7%	5%								
Lane Vol Left, %		0% 5%	A WBLn1 7% 93%	5% 89%								
Lane Vol Left, % Vol Thru, %		0%	A WBLn1 7%	5% 89% 6%								
Lane Vol Left, % Vol Thru, % Vol Right, %		0% 5% 95% Stop	A WBLn1 7% 93% 0% Stop	5% 89% 6% Stop								
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control		0% 5% 95%	A WBLn1 7% 93% 0%	5% 89% 6% Stop 305								
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane		0% 5% 95% Stop	WBLn1 7% 93% 0% Stop 14 1	5% 89% 6% Stop 305 16								
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol		0% 5% 95% Stop 61 0	A WBLn1 7% 93% 0% Stop 14 1 13	5% 89% 6% Stop 305 16 271								
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol		0% 5% 95% Stop 61 0	A WBLn1 7% 93% 0% Stop 14 1 13	5% 89% 6% Stop 305 16 271 18								
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol		0% 5% 95% Stop 61 0	A WBLn1 7% 93% 0% Stop 14 1 13	5% 89% 6% Stop 305 16 271								
HCM LOS Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate		0% 5% 95% Stop 61 0 3 58	A WBLn1 7% 93% 0% Stop 14 1 13 0 15 1	5% 89% 6% Stop 305 16 271 18 332								
HCM LOS Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp		0% 5% 95% Stop 61 0 3 58 66 1	A WBLn1 7% 93% 0% Stop 14 1 13 0 15 1 10.02	5% 89% 6% Stop 305 16 271 18 332 1								
HCM LOS Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X)		0% 5% 95% Stop 61 0 3 58 66	A WBLn1 7% 93% 0% Stop 14 1 13 0 15 1	5% 89% 6% Stop 305 16 271 18 332 1 0.37 4.017								
HCM LOS Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd)		0% 5% 95% Stop 61 0 3 58 66 1	A WBLn1 7% 93% 0% Stop 14 1 13 0 15 1 0.02 4.715 Yes	5% 89% 6% Stop 305 16 271 18 332 1 0.37 4.017 Yes								
HCM LOS Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N		0% 5% 95% Stop 61 0 3 58 66 1 0.075 4.074 Yes 885	A WBLn1 7% 93% 0% Stop 14 1 13 0 15 1 0.02 4.715 Yes 763	5% 89% 6% Stop 305 16 271 18 332 1 0.37 4.017 Yes 892								
HCM LOS Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap		0% 5% 95% Stop 61 0 3 58 66 1 0.075 4.074 Yes	A WBLn1 7% 93% 0% Stop 14 1 13 0 15 1 0.02 4.715 Yes 763 2.718	5% 89% 6% Stop 305 16 271 18 332 1 0.37 4.017 Yes 892 2.06								
HCM LOS Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time		0% 5% 95% Stop 61 0 3 58 66 1 0.075 4.074 Yes 885	A WBLn1 7% 93% 0% Stop 14 1 13 0 15 1 0.02 4.715 Yes 763	5% 89% 6% Stop 305 16 271 18 332 1 0.37 4.017 Yes 892 2.06 0.372								
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time HCM Lane V/C Ratio		0% 5% 95% Stop 61 0 3 58 66 1 0.075 4.074 Yes 885 2.075	A WBLn1 7% 93% 0% Stop 14 1 13 0 15 1 0.02 4.715 Yes 763 2.718	5% 89% 6% Stop 305 16 271 18 332 1 0.37 4.017 Yes 892 2.06								
HCM LOS Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time		0% 5% 95% Stop 61 0 3 58 66 1 0.075 4.074 Yes 885 2.075 0.075	A WBLn1 7% 93% 0% Stop 14 1 13 0 15 1 0.02 4.715 Yes 763 2.718 0.02	5% 89% 6% Stop 305 16 271 18 332 1 0.37 4.017 Yes 892 2.06 0.372								

ntersection Delay, s/veh ntersection LOS					
Movement	SBU	SBL	SBT	SBR	13.1
/ol, veh/h	0	16	271	18	
Peak Hour Factor	0.92	0.92	0.92	0.92	
Heavy Vehicles, %	2	0	1	0	
Nymt Flow	0	17	295	20	
Number of Lanes	0	0	1	0	
Approach	Fe 1 F 3 8	SB	May	14667	0 - 4
Opposing Approach					
Opposing Lanes		0			
Conflicting Approach Left		WB			
Conflicting Lanes Left		1			
Conflicting Approach Right		EB			
Conflicting Lanes Right		1			
ICM Control Delay		9.4			
HCM LOS		A			

8: Cottage Street & Gove Street

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		1>			4						4	
Volume (vph)	0	3	24	5	11	0	0	0	0	20	181	23
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	14	12	12	14	12	12	12	12	12	14	12
Satd. Flow (prot)	0	1603	0	0	1798	0	0	0	0	0	1775	0
Fit Permitted					0.986						0.995	
Satd. Flow (perm)	0	1603	0	0	1798	0	0	0	0	0	1775	0
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		888			309			587			677	
Travel Time (s)		20.2			7.0			13.3			15.4	
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Heavy Vehicles (%)	0%	0%	0%	0%	0%	0%	2%	2%	2%	0%	1%	0%
Shared Lane Traffic (%)	0,0	14.65										
Lane Group Flow (vph)	0	29	0	0	17	0	0	0	0	0	242	0
Sign Control		Stop			Stop			Stop			Stop	

Intersection Summary

Area Type:

CBD

Control Type: Unsignalized

Intersection Capacity Utilization 25.6%

ICU Level of Service A

Intersection												
Intersection Delay, s/veh	8.2											
Intersection LOS	Α											
Movement	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBF
Vol, veh/h	0	0	3	24	0	5	11	0	0	0	0	
Peak Hour Factor	0.92	0.93	0.93	0.93	0.92	0.93	0.93	0.93	0.92	0.93	0.93	0.9
Heavy Vehicles, %	2	0	0	0	2	0	0	0	2	2	2	
Mymt Flow	0	0	3	26	0	5	12	0	0	0	0	
Number of Lanes	0	0	1	0	0	0	1	0	0	0	0	
Approach			EB			WB						
Opposing Approach			WB			EB						
Opposing Lanes			1			1						
Conflicting Approach Left			SB									
Conflicting Lanes Left			1			0						
Conflicting Approach Right						SB						
Conflicting Lanes Right			0			1						
						7.6						
HCM Control Delay			7			1.0						
HCM Control Delay HCM LOS			Á			Α						
		EBLn1	A WBLn1	SBLn1								
HCM LOS		EBLn1 0%	WBLn1 31%	9%								
Lane Vol Left, %			A WBLn1 31% 69%	9% 81%								
Lane Vol Left, % Vol Thru, %		0%	WBLn1 31%	9% 81% 10%								
Lane Vol Left, % Vol Thru, % Vol Right, %		0% 11%	A WBLn1 31% 69% 0% Stop	9% 81% 10% Stop								
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control		0% 11% 89%	MBLn1 31% 69% 0% Stop 16	9% 81% 10% Stop 224								
Lane Vol Left, % Vol Thru, % Vol Right, %		0% 11% 89% Stop	MBLn1 31% 69% 0% Stop 16 5	9% 81% 10% Stop 224 20								
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol		0% 11% 89% Stop 27 0 3	MBLn1 31% 69% 0% Stop 16 5 11	9% 81% 10% Stop 224 20 181								
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane		0% 11% 89% Stop 27 0 3 24	A WBLn1 31% 69% 0% Stop 16 5 11	9% 81% 10% Stop 224 20 181 23								
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol		0% 11% 89% Stop 27 0 3	MBLn1 31% 69% 0% Stop 16 5 11	9% 81% 10% Stop 224 20 181								
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate		0% 11% 89% Stop 27 0 3 24	A WBLn1 31% 69% 0% Stop 16 5 11 0 17 1	9% 81% 10% Stop 224 20 181 23 241								
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp		0% 11% 89% Stop 27 0 3 24 29	A WBLn1 31% 69% 0% Stop 16 5 11 0 17 1 0.022	9% 81% 10% Stop 224 20 181 23 241 1								
HCM LOS Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X)		0% 11% 89% Stop 27 0 3 24 29	A WBLn1 31% 69% 0% Stop 16 5 11 0 17 1	9% 81% 10% Stop 224 20 181 23 241								
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd)		0% 11% 89% Stop 27 0 3 24 29 1	MBLn1 31% 69% 0% Stop 16 5 11 0 17 1 0.022 4.511 Yes	9% 81% 10% Stop 224 20 181 23 241 1 0.263 3.936 Yes								
HCM LOS Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N		0% 11% 89% Stop 27 0 3 24 29 1 0.031 3.902	MBLn1 31% 69% 0% Stop 16 5 11 0 17 1 0.022 4.511 Yes 798	9% 81% 10% Stop 224 20 181 23 241 1 0.263 3.936 Yes 913								
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd)		0% 11% 89% Stop 27 0 3 24 29 1 0.031 3.902 Yes	MBLn1 31% 69% 0% Stop 16 5 11 0 17 1 0.022 4.511 Yes	9% 81% 10% Stop 224 20 181 23 241 1 0.263 3.936 Yes 913 1.959								
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time		0% 11% 89% Stop 27 0 3 24 29 1 0.031 3.902 Yes 923	MBLn1 31% 69% 0% Stop 16 5 11 0 17 1 0.022 4.511 Yes 798	9% 81% 10% Stop 224 20 181 23 241 1 0.263 3.936 Yes 913 1.959 0.264								
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time HCM Lane V/C Ratio		0% 11% 89% Stop 27 0 3 24 29 1 0.031 3.902 Yes 923 1.903	A WBLn1 31% 69% 0% Stop 16 5 11 0 17 1 0.022 4.511 Yes 798 2.511	9% 81% 10% Stop 224 20 181 23 241 1 0.263 3.936 Yes 913 1.959								
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time		0% 11% 89% Stop 27 0 3 24 29 1 0.031 3.902 Yes 923 1.903 0.031	A WBLn1 31% 69% 0% Stop 16 5 11 0 17 1 0.022 4.511 Yes 798 2.511 0.021	9% 81% 10% Stop 224 20 181 23 241 1 0.263 3.936 Yes 913 1.959 0.264								

ntersection									
ntersection Delay, s/veh									
Movement	SBU	SBL	SBT	SBR	e fation e	418481		SECTION AND ADDRESS.	
Vol, veh/h	0	20	181	23					
Peak Hour Factor	0.92	0.93	0.93	0.93					
Heavy Vehicles, %	2	0	1	0					
Mvmt Flow	0	22	195	25					
Number of Lanes	0	0	1	0					
Approach	Ja Janu W	SB			TRUE!		7 1		
Opposing Approach									
Opposing Lanes		0							
Conflicting Approach Left	.,	WB							
Conflicting Lanes Left		1							
Conflicting Approach Right		EB							
Conflicting Lanes Right		1.							
HCM Control Delay		8.4							
HCM LOS		A							

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		1>			4						43+	
Volume (vph)	0	3	58	1	13	0	0	0	0	16	271	18
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	14	12	12	14	12	12	12	12	12	14	12
Satd. Flow (prot)	0	1559	0	0	1819	0	0	0	0	0	1788	0
Flt Permitted					0.997						0.997	
Satd. Flow (perm)	0	1559	0	0	1819	0	0	0	0	0	1788	0
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		451			309			587			677	
Travel Time (s)		10.3			7.0			13.3			15.4	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	0%	2%	0%	0%	0%	2%	2%	2%	0%	1%	0%
Shared Lane Traffic (%)	070	0.0										
Lane Group Flow (vph)	0	66	0	0	15	0	0	0	0	0	332	0
Sign Control		Stop			Stop			Stop			Stop	
Intersection Summary		(Ekimples			TO COME	FILES !					Sugar.	
Area Type: Control Type: Unsignalized	CBD	15.50	4.41									

Intersection Capacity Utilization 28.9%

ICU Level of Service A

Intersection	Ann the		1,34 3	IIWII S TO	Let us			11 S. P. S.	alu Adam	en oney.	1	
Intersection Delay, s/veh	9											
Intersection LOS	A											
Movement	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBF
Vol, veh/h	0	0	3	58	0	1	13	0	0	0	0	(
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles, %	2	0	0	2	2	0	0	0	2	2	2	3
Mvmt Flow	0	0	3	63	0	1	14	0	0	0	0	(
Number of Lanes	0	0	1	0	0	0	1	0	0	0	0	<u> </u>
Approach		1 1 9	EB	grafia.		WB	-N. 1976		ial groop	vállalja:		152
Opposing Approach			WB			EB						
Opposing Lanes			1			1						
Conflicting Approach Left			SB			12						
Conflicting Lanes Left			1			0						
Conflicting Approach Right						SB						
Conflicting Lanes Right			0			1						
HCM Control Delay			7.4			7.8						
HCM LOS			Α			Α						
			dreneshii swi	22.72.V = 0.10				No. of the last	70.10.11			
Lane	0.5/Miles	EBLn1	WBLn1	SBLn1		10,548				-000	The state of the s	
Vol Left, %		0%	7%	5%								
Vol Thru, %		5%	93%	89%								
				001								
Vol Right, %		95%	0%	6%								
Sign Control		Stop	Stop	Stop								
Sign Control Traffic Vol by Lane		Stop 61	Stop 14	Stop 305								
Sign Control Traffic Vol by Lane LT Vol		Stop 61 0	Stop 14 1	Stop 305 16								
Sign Control Traffic Vol by Lane LT Vol Through Vol		Stop 61 0 3	Stop 14 1 13	Stop 305 16 271								
Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol		Stop 61 0 3 58	Stop 14 1 13 0	Stop 305 16 271 18								
Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate		61 0 3 58 66	Stop 14 1 13 0 15	Stop 305 16 271 18 332								
Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp		Stop 61 0 3 58 66 1	Stop 14 1 13 0 15	Stop 305 16 271 18 332 1								
Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X)		Stop 61 0 3 58 66 1 0.075	Stop 14 1 13 0 15 1 0.02	Stop 305 16 271 18 332 1 0.37								
Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp		Stop 61 0 3 58 66 1 0.075 4.074	Stop 14 1 13 0 15 1 0.02 4.715	Stop 305 16 271 18 332 1 0.37 4.017								
Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N		Stop 61 0 3 58 66 1 0.075 4.074 Yes	Stop 14 1 13 0 15 1 0.02 4.715 Yes	Stop 305 16 271 18 332 1 0.37 4.017 Yes								
Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap		Stop 61 0 3 58 66 1 0.075 4.074 Yes 885	Stop 14 1 13 0 15 1 0.02 4.715 Yes 763	Stop 305 16 271 18 332 1 0.37 4.017 Yes 892								
Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N		Stop 61 0 3 58 66 1 0.075 4.074 Yes 885 2.075	Stop 14 1 13 0 15 1 0.02 4.715 Yes 763 2.718	Stop 305 16 271 18 332 1 0.37 4.017 Yes 892 2.06								
Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap		Stop 61 0 3 58 66 1 0.075 4.074 Yes 885 2.075 0.075	Stop 14 1 13 0 15 1 0.02 4.715 Yes 763 2.718 0.02	Stop 305 16 271 18 332 1 0.37 4.017 Yes 892 2.06 0.372								
Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time		Stop 61 0 3 58 66 1 0.075 4.074 Yes 885 2.075 0.075 7.4	Stop 14 1 13 0 15 1 0.02 4.715 Yes 763 2.718 0.02 7.8	Stop 305 16 271 18 332 1 0.37 4.017 Yes 892 2.06 0.372 9.4								
Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time HCM Lane V/C Ratio		Stop 61 0 3 58 66 1 0.075 4.074 Yes 885 2.075 0.075	Stop 14 1 13 0 15 1 0.02 4.715 Yes 763 2.718 0.02	Stop 305 16 271 18 332 1 0.37 4.017 Yes 892 2.06 0.372								

Intersection					
Intersection Delay, s/veh Intersection LOS					
Movement	SBU	SBL	SBT	SBR	
Vol, veh/h	0	16	271	18	
Peak Hour Factor	0.92	0.92	0.92	0.92	
Heavy Vehicles, %	2	0	1	0	
Mvmt Flow	0	17	295	20	
Number of Lanes	0	0	1	0	
Approach		SB	- 153		
Opposing Approach					
Opposing Lanes		0			
Conflicting Approach Left		WB			
Conflicting Lanes Left		1			
Conflicting Approach Right		EB			
Conflicting Lanes Right		1			
HCM Control Delay		9.4			
HCM LOS		Α			

	*	→	*	1	-	*	4	†	-	1	↓	1
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		1			4						4	
Volume (vph)	0	3	24	5	11	0	0	0	0	20	181	23
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	14	12	12	14	12	12	12	12	12	14	12
Satd. Flow (prot)	0	1603	0	0	1798	0	0	0	0	0	1775	0
Flt Permitted					0.986						0.995	
Satd. Flow (perm)	0	1603	0	0	1798	0	0	0	0	0	1775	0
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		439			309			587			677	
Travel Time (s)		10.0			7.0			13.3			15.4	
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Heavy Vehicles (%)	0%	0%	0%	0%	0%	0%	2%	2%	2%	0%	1%	0%
Shared Lane Traffic (%)			13.43									
Lane Group Flow (vph)	0	29	0	0	17	0	0	0	0	0	242	0
Sign Control		Stop			Stop			Stop			Stop	

Area Type:

CBD

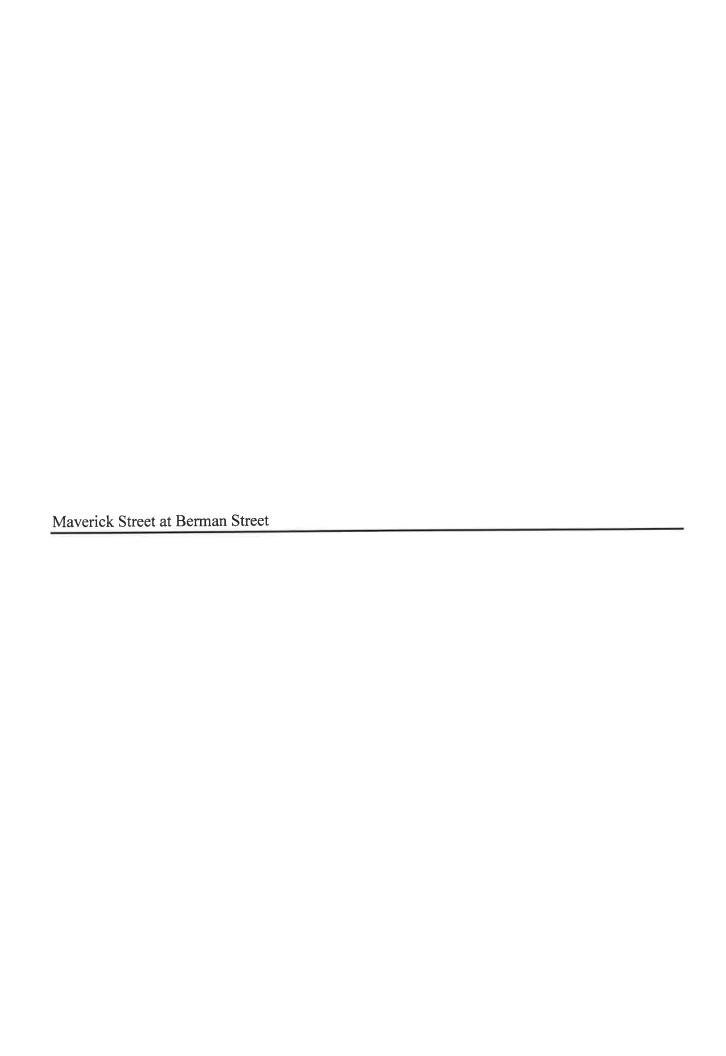
Control Type: Unsignalized

Intersection Capacity Utilization 25.6%

ICU Level of Service A

Intersection Intersection Delay, s/veh	8.2										
Intersection LOS	A A										
Movement	EBU EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBF
Vol, veh/h	0 0	3	24	0	5	11	0	0	0	0	(
Peak Hour Factor	0.92 0.93	0.93	0.93	0,92	0.93	0.93	0.93	0.92	0.93	0.93	0.93
Heavy Vehicles, %	2 0	0	0	2	0	0	0	2	2	2	2
Mymt Flow	0 0	3	26	0	5	12	0	0	0	0	(
Number of Lanes	0 0	1	0	0	0	1	0	0	0	0	C
Approach	with the last the same	EB			WB	", y 10	- Wilde		n s		TVA-1
Opposing Approach		WB			EB						
Opposing Lanes		1			1						
Conflicting Approach Left		SB									
Conflicting Lanes Left		. 1			0						
Conflicting Approach Right					SB						
Conflicting Lanes Right		0			1						
HCM Control Delay		7			7.6						
HCM LOS		Α			Α						
HCM LOS		Α			Α						
Lane	EBLn1	WBLn1	SBLn1	W.W. 2 E	. A			Wiston			
	0%	WBLn1 31%	9%	N.被. 17	A		- 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1	N atom			
Lane	0% 11%	WBLn1 31% 69%	9% 81%		A	MARK.		NIVETON			
Lane Vol Left, %	0%	WBLn1 31%	9% 81% 10%		A	1000年		NIVERSET.			
Lane Vol Left, % Vol Thru, %	0% 11% 89% Stop	WBLn1 31% 69% 0% Stop	9% 81% 10% Stop		A						
Lane Vol Left, % Vol Thru, % Vol Right, %	0% 11% 89%	WBLn1 31% 69% 0% Stop	9% 81% 10% Stop 224		A			Will Proper			
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control	0% 11% 89% Stop 27	WBLn1 31% 69% 0% Stop 16 5	9% 81% 10% Stop 224 20		A						
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane	0% 11% 89% Stop 27 0	WBLn1 31% 69% 0% Stop 16 5 11	9% 81% 10% Stop 224 20 181	NOW I F	A						
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol	0% 11% 89% Stop 27	WBLn1 31% 69% 0% Stop 16 5 11	9% 81% 10% Stop 224 20 181 23		A						
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol	0% 11% 89% Stop 27 0	WBLn1 31% 69% 0% Stop 16 5 11	9% 81% 10% Stop 224 20 181		A						
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate	0% 11% 89% Stop 27 0 3	WBLn1 31% 69% 0% Stop 16 5 11	9% 81% 10% Stop 224 20 181 23 241		A						
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol	0% 11% 89% Stop 27 0 3 24	WBLn1 31% 69% 0% Stop 16 5 11 0	9% 81% 10% Stop 224 20 181 23		A						
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X)	0% 11% 89% Stop 27 0 3 24 29	WBLn1 31% 69% 0% Stop 16 5 11 0 17 1 0.022	9% 81% 10% Stop 224 20 181 23 241		A						
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp	0% 11% 89% Stop 27 0 3 24 29 1	WBLn1 31% 69% 0% Stop 16 5 11 0 17 1 0.022 4.511	9% 81% 10% Stop 224 20 181 23 241 1 0.263 3.936 Yes		A						
Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N	0% 11% 89% Stop 27 0 3 24 29 1 0.031 3.902	WBLn1 31% 69% 0% Stop 16 5 11 0 17 1 0.022 4.511 Yes	9% 81% 10% Stop 224 20 181 23 241 1 0.263 3.936		A						
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd)	0% 11% 89% Stop 27 0 3 24 29 1 0.031 3.902 Yes	WBLn1 31% 69% 0% Stop 16 5 11 0 17 1 0.022 4.511 Yes 798	9% 81% 10% Stop 224 20 181 23 241 1 0.263 3.936 Yes		A						
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap	0% 11% 89% Stop 27 0 3 24 29 1 0.031 3.902 Yes	WBLn1 31% 69% 0% Stop 16 5 11 0 17 1 0.022 4.511 Yes 798	9% 81% 10% Stop 224 20 181 23 241 1 0.263 3.936 Yes 913		A						
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time HCM Lane V/C Ratio	0% 11% 89% Stop 27 0 3 24 29 1 0.031 3.902 Yes 923 1.903	WBLn1 31% 69% 0% Stop 16 5 11 0 17 1 0.022 4.511 Yes 798 2.511	9% 81% 10% Stop 224 20 181 23 241 1 0.263 3.936 Yes 913 1.959		A						
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time	0% 11% 89% Stop 27 0 3 24 29 1 0.031 3.902 Yes 923 1.903 0.031	WBLn1 31% 69% 0% Stop 16 5 11 0 17 1 0.022 4.511 Yes 798 2.511 0.021 7.6	9% 81% 10% Stop 224 20 181 23 241 1 0.263 3.936 Yes 913 1.959 0.264		A						

Intersection Delay, s/veh Intersection LOS					
Movement	SBU	SBL	SBT	SBR	
Vol, veh/h	0	20	181	23	
Peak Hour Factor	0.92	0.93	0.93	0.93	
Heavy Vehicles, %	2	0	1	0	
Mvmt Flow	0	22	195	25	
Number of Lanes	0	0	1	0	
Approach	X9,50 1154	SB			
Opposing Approach					
Opposing Lanes		0			
Conflicting Approach Left		WB			
Conflicting Lanes Left		1			
Conflicting Approach Right		EB			
Conflicting Lanes Right		1			
HCM Control Delay		8.4			
HCM LOS		A			



	*		*	1	←	*	4	†	1	1		1
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	4100				4			4			7	
Volume (vph)	0	0	0	14	269	74	26	71	0	0	81	58
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	14	12	12	15	12	12	12	12
Satd. Flow (prot)	0	0	0	0	1718	0	0	1817	0	0	1556	0
Flt Permitted	, and the second				0.998			0.987				
Satd. Flow (perm)	0	0	0	0	1718	0	0	1817	0	0	1556	0
Link Speed (mph)	ŭ	30			30			30			30	
Link Opeed (mph)		491			751			343			750	
Travel Time (s)		11.2			17.1			7.8			17.0	
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Heavy Vehicles (%)	2%	2%	2%	0%	4%	0%	0%	3%	0%	0%	5%	2%
Shared Lane Traffic (%)	270	270										
Lane Group Flow (vph)	0	0	0	0	384	0	0	104	0	0	149	.0
Sign Control	Ü	Stop			Stop			Stop			Stop	

Area Type:

CBD

Control Type: Unsignalized

Intersection Capacity Utilization 46.0%

Analysis Period (min) 15

Intersection	-51,557	1307			Nes 111		HI WILL	1, 111111	-XI II A			
Intersection Delay, s/veh	10.4											
Intersection LOS	В											
Movement	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBR
Vol, veh/h	0	0	0	0	0	14	269	74	0	26	71	0
Peak Hour Factor	0.92	0.93	0.93	0.93	0.92	0.93	0.93	0.93	0.92	0.93	0.93	0.93
Heavy Vehicles, %	2	2	2	2	2	0	4	0	2	0	3	0
Mymt Flow	0	0	0	0	0	15	289	80	0	28	76	0
Number of Lanes	0	0	0	0	0	0	1	0	0	0	1	0

Approach	WB	NB
Opposing Approach		SB
Opposing Lanes	0	1
Conflicting Approach Left	NB	
Conflicting Lanes Left	1	0
Conflicting Approach Right	SB	WB
Conflicting Lanes Right		
HCM Control Delay	11,3	9
HCM LOS	В В	A

Lane	NBLn1	WBLn1	SBLn1
Vol Left, %	27%		0%
Vol Thru, %	73%		58%
Vol Right, %	0%	21%	42%
Sign Control	Stop	Stop	Stop
Traffic Vol by Lane	97	357	139
LT Vol	26	14	0
Through Vol	71	269	81
RT Vol	0	74	58
Lane Flow Rate	104	384	149
Geometry Grp	1	1	1
Degree of Util (X)	0.146	0.469	0.198
Departure Headway (Hd)	5.033	4.4	4.762
Convergence, Y/N	Yes	Yes	Yes
Сар	709	818	750
Service Time	3.084		2.809
HCM Lane V/C Ratio	0.147	0.469	0.199
HCM Control Delay	9	11.3	9
HCM Lane LOS	Α		Α
HCM 95th-tile Q	0.5	2.5	0.7

Intersection	Jal of y		W MADEL	100	- 51 (1)	THE PLEASE	11 21 3	WAR S	41	& Sime
Intersection Delay, s/veh Intersection LOS										
Movement	SBU	SBL	SBT	SBR	1. 1. 28 Year	, May				
Vol, veh/h	0	0	81	58						
Peak Hour Factor	0.92	0.93	0.93	0.93						
Heavy Vehicles, %	2	0	5	2						
Mvmt Flow	0	0	87	62						
Number of Lanes	0	0	1	0						
Approach	ne Va		SB	2 10	1507 8118	· Comits		312	184 127	
Opposing Approach			NB							
Opposing Lanes			1							
Conflicting Approach Left			WB							
Conflicting Lanes Left			1							
Conflicting Approach Right										
Conflicting Lanes Right			0							
HCM Control Delay			9							
HCM LOS			Α							

	*	-	*	1	←	*	4	†	1	-	1	1
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations					4			4			7	
Volume (vph)	0	0	0	9	200	69	27	86	0	0	112	66
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	14	12	12	15	12	12	12	12
Satd. Flow (prot)	0	0	0	0	1735	0	0	1858	0	0	1624	0
Flt Permitted					0.998			0.988				
Satd. Flow (perm)	0	0	0	0	1735	0	0	1858	0	0	1624	0
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		491			751			343			750	
Travel Time (s)		11.2			17.1			7.8			17.0	
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Heavy Vehicles (%)	2%	2%	2%	0%	2%	0%	0%	0%	0%	0%	0%	0%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	0	0	0	296	0	0	120	0	0	189	0
Sign Control		Stop			Stop			Stop			Stop	

Area Type:

CBD

Control Type: Unsignalized

Intersection Capacity Utilization 44.6%

Analysis Period (min) 15

Conflicting Approach Right Conflicting Lanes Right

HCM Control Delay

HCM Lane LOS

HCM 95th-tile Q

Intersection Delay, s/veh	9.5											
Intersection LOS	Α											
Movement	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBR
Vol, veh/h	0	0	0	0	0	9	200	69	0	27	86	0
Peak Hour Factor	0.92	0.94	0.94	0.94	0.92	0.94	0.94	0.94	0.92	0.94	0.94	0.94
Heavy Vehicles, %	2	2	2	2	2	0	2	0	2	0	0	0
Mymt Flow	0	0	0	0	0	10	213	73	0	29	91	0
Number of Lanes	0	0	0	0	0	0	1	0	0	0	1	0
Approach						WB				NB		
Opposing Approach										SB		
Opposing Lanes						0				1		
Conflicting Approach Left						NB						
Conflicting Lanes Left						1				0		
Conflicting Approach Right						SB				WB		

10.1

HCM LOS				Ь	
Lane	NBLn1	WBLn1	SBLn1		
Vol Left, %	24%	3%	0%		
Vol Thru, %	76%	72%	63%		
/ol Right, %	0%	25%	37%		
Sign Control	Stop	Stop	Stop		
Fraffic Vol by Lane	113	278	178		
_T Vol	27	9	0		
Through Vol	86	200	112		
RT Vol	0	69	66		
ane Flow Rate	120	296	189		
Geometry Grp	1	1:	1		
Degree of Util (X)	0.162	0.368	0.238		
Departure Headway (Hd)	4.865	4.475	4.521		
Convergence, Y/N	Yes	Yes	Yes		
Cap	735	801	792		
Service Time	2.91	2.512	2.562		
HCM Lane V/C Ratio	0.163	0.37	0.239		
HCM Control Delay	8.9		9		
Tom condict Dollar	Α.	n	٨		

В

1.7

Α

0.6

Α

0.9

8.9

Intersection			L)EXE				
Intersection Delay, s/veh Intersection LOS							
Movement	SBU	SBL	SBT	SBR	III TA XIX	08 - M	
Vol, veh/h	0	0	112	66			
Peak Hour Factor	0.92	0.94	0.94	0.94			
Heavy Vehicles, %	2	0	0	0			
Mymt Flow	0	0	119	70			
Number of Lanes	0	0	1	0			
Approach	511511		SB	Tunité I	Territoria Est		
Opposing Approach			NB				
Opposing Lanes			1				
Conflicting Approach Left			WB				
Conflicting Lanes Left			1				
Conflicting Approach Right			0				
Conflicting Lanes Right			0				
HCM Control Delay			9				
HCM LOS			Α				

Lane Configurations Volume (vph) 0 0 1900 1900		٠	→	*	1	←	*	1	†	-	-		1
Lane Configurations	Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Volume (vph) 0 0 0 18 292 81 28 80 0 0 88 Ideal Flow (vphpl) 1900 1816 0 0 1552 12 12 12 12 12 12 12 1816 0 0						4			र्स			1	
Ideal Flow (vphpl) 1900 <td>-</td> <td>0</td> <td>0</td> <td>0</td> <td>18</td> <td>292</td> <td>81</td> <td>28</td> <td>80</td> <td>0</td> <td>0</td> <td>88</td> <td>69</td>	-	0	0	0	18	292	81	28	80	0	0	88	69
Lane Width (ft) 12 12 12 14 12 15 12 12 12 12 12		1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Satd. Flow (prot) 0 0 0 0 1718 0 0 1816 0 0 1552 Flt Permitted 0.998 0.987				12	12	14	12	12	15	12	12	12	12
Fit Permitted 0.998 0.987 Satd. Flow (perm) 0 0 0 1718 0 0 1816 0 0 1552 Link Speed (mph) 30 30 30 30 30 Link Distance (ft) 491 751 343 750 Travel Time (s) 11.2 17.1 7.8 17.0 Peak Hour Factor 0.93 <td< td=""><td>• •</td><td></td><td></td><td>0</td><td>0</td><td>1718</td><td>0</td><td>0</td><td>1816</td><td>0</td><td>0</td><td>1552</td><td>0</td></td<>	• •			0	0	1718	0	0	1816	0	0	1552	0
Satd. Flow (perm) 0 0 0 0 1718 0 0 1816 0 0 1552 Link Speed (mph) 30 30 30 30 30 Link Distance (ft) 491 751 343 750 Travel Time (s) 11.2 17.1 7.8 17.0 Peak Hour Factor 0.93						0.998			0.987				
Link Speed (mph) 30 30 30 30 Link Distance (ft) 491 751 343 750 Travel Time (s) 11.2 17.1 7.8 17.0 Peak Hour Factor 0.93 0.		0	0	0	0	1718	0	0	1816	0	0	1552	0
Link Distance (ft) 491 751 343 750 Travel Time (s) 11.2 17.1 7.8 17.0 Peak Hour Factor 0.93 0.93 0.93 0.93 0.93 0.93 0.93 0.93	***		30			30			30			30	
Travel Time (s) 11.2 17.1 7.8 17.0 Peak Hour Factor 0.93 0.93 0.93 0.93 0.93 0.93 0.93 0.93						751			343			750	
Peak Hour Factor 0.93 0.9	` '					17.1			7.8			17.0	
Heavy Vehicles (%) 2% 2% 2% 0% 4% 0% 0% 3% 0% 0% 5% Shared Lane Traffic (%) Lane Group Flow (vph) 0 0 0 420 0 0 116 0 0 169	\ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \	0.93		0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Shared Lane Traffic (%) Lane Group Flow (yph) 0 0 0 420 0 0 116 0 0 169						4%	0%	0%	3%	0%	0%	5%	2%
Lane Group Flow (vph) 0 0 0 0 420 0 0 116 0 0 169			77.13	=	15-10								
	, ,	0	0	0	0	420	0	0	116	0	0	169	0
Sign Control Stop Stop Stop Stop			1000			Stop			Stop			Stop	

Area Type: CBD Control Type: Unsignalized

Intersection Capacity Utilization 49.9%

ICU Level of Service A

Intersection											-	
Intersection Delay, s/veh	11.2											
Intersection LOS	В											
Movement	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBR
Vol, veh/h	0	0	0	0	0	18	292	81	0	28	80	0
Peak Hour Factor	0.92	0.93	0.93	0.93	0.92	0.93	0.93	0.93	0.92	0,93	0.93	0.93
Heavy Vehicles, %	2	2	2	2	2	0	4	0	2	0	3	0
Mymt Flow	0	0	0	0	0	19	314	87	0	30	86	0
Number of Lanes	0	0	0	0	0	0	1	0	0	0	1	0
Approach					900	WB	3	11-		NB		- 3
Opposing Approach										SB		
Opposing Lanes						0				1		
Conflicting Approach Left						NB						
Conflicting Lanes Left	. F. F. F.					1				0		
Commoning Edition Edit						0.0				MAID		

SB

12.4

Lane	NBLn1	WBLn1	SBLn1
Vol Left, %	26%	5%	0%
Vol Thru, %	74%	75%	56%
Vol Right, %	0%	21%	44%
Sign Control	Stop	Stop	Stop
Traffic Vol by Lane	108	391	157
LT Vol	28	18	0
Through Vol	80	292	88
RT Vol	0	81	69
Lane Flow Rate	116	420	169
Geometry Grp	1	1	1
Degree of Util (X)	0.166	0.524	0.228
Departure Headway (Hd)	5.158	4.484	4.864
Convergence, Y/N	Yes	Yes	Yes
Cap	691	803	733
Service Time	3.227	2.528	2.928
HCM Lane V/C Ratio	0.168		0.231
HCM Control Delay	9.3	12.4	9.4
HCM Lane LOS	Α	В	Α
HCM 95th-tile Q	0.6	3.1	0.9

Conflicting Approach Right
Conflicting Lanes Right

HCM Control Delay

HCM LOS

WB

9.3

Intersection Intersection Delay, s/veh					
Intersection LOS					
Intersection Loo					
Movement	SBU	SBL	SBT	SBR	
Vol, veh/h	0	0	88	69	
Peak Hour Factor	0.92	0.93	0.93	0.93	
Heavy Vehicles, %	2	0	5	2	
Mvmt Flow	0	0	95	74	
Number of Lanes	0	0	1	0	
Approach	STANE		SB	10° y	
Opposing Approach			NB		
Opposing Lanes			1		
Conflicting Approach Left			WB		
Conflicting Lanes Left			1		
Conflicting Approach Right					
Conflicting Lanes Right			0		
HCM Control Delay			9.4		
HCM LOS			Α		

-	۶	→	*	•	←	4	4	†	-	-	↓	1
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations					4			4			1>	
Volume (vph)	0	0	0	15	216	78	29	101	0	0	122	77
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	14	12	12	15	12	12	12	12
Satd. Flow (prot)	0	0	0	0	1734	0	0	1860	0	0	1621	0
FIt Permitted					0.998			0.989				
Satd. Flow (perm)	0	0	0	0	1734	0	0	1860	0	0	1621	0
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		491			751			343			750	
Travel Time (s)		11.2			17.1			7.8			17.0	
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Heavy Vehicles (%)	2%	2%	2%	0%	2%	0%	0%	0%	0%	0%	0%	0%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	0	0	0	329	0	0	138	0	0	212	0
Sign Control	•	Stop			Stop			Stop			Stop	

Area Type:

CBD

Control Type: Unsignalized

Intersection Capacity Utilization 48.9%

Analysis Period (min) 15

Intersection			-									12
Intersection Delay, s/veh	10.1											
Intersection LOS	В											
Movement	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBR
Vol, veh/h	0	0	0	0	0	15	216	78	0	29	101	0
Peak Hour Factor	0.92	0.94	0.94	0.94	0.92	0.94	0.94	0.94	0.92	0.94	0.94	0.94
Heavy Vehicles, %	2	2	2	2	2	0	2	0	2	0	0	0
Mvmt Flow	0	0	0	0	0	16	230	83	0	31	107	0
Number of Lanes	0	0	0	0	0	0	1	0	0	0	1	0
Approach				l'a	_0	WB		10 -		NB		
Opposing Approach										SB		
Opposing Lanes						0				1		
Conflicting Approach Left						NB						
Conflicting Lanes Left						1				0		
Conflicting Approach Right						SB				WB		
Conflicting Lanes Right						1				1		
HCM Control Delay						10.9				9.2		
HCM LOS						В				Α		
Lane		NBLn1	WBLn1	SBLn1		1.3-5						55 B.II
Vol Left, %		22%	5%	0%								
Vol Thru, %		78%	70%	61%								
Vol Right, %		0%	25%	39%								
Sign Control		Stop	Stop	Stop								
Traffic Vol by Lane		130	309	199								
LT Vol		29	15	0								
Through Vol		101	216	122								
RT Vol		0	78	77								
111 701			000	040								

1

138

0.192

4.985

Yes 716

3.044

0.193

9.2

0.7

Α

329

0.418

4.579

Yes

782

2.628

0.421

10.9

В

2.1

1

212

0.272

4.63

Yes

773

2.684

0.274

9.4

1.1

Α

1

Lane Flow Rate

Geometry Grp

Service Time

Cap

Degree of Util (X)

Convergence, Y/N

HCM Lane V/C Ratio

HCM Control Delay

HCM Lane LOS

HCM 95th-tile Q

Departure Headway (Hd)

Intersection					
Intersection Delay, s/veh Intersection LOS					
Movement	SBU	SBL	SBT	SBR	Hi sucultar
Vol, veh/h	0	0	122	77	
Peak Hour Factor	0.92	0.94	0.94	0.94	
Heavy Vehicles, %	2	0	0	0	
Mvmt Flow	0	0	130	82	
Number of Lanes	0	0	1	0	
Approach	Till wi		SB		
Opposing Approach			NB		
Opposing Lanes			1		
Conflicting Approach Left			WB		
Conflicting Lanes Left			1		
Conflicting Approach Right					
Conflicting Lanes Right			0		
HCM Control Delay			9.4		
HCM LOS			Α		

	٠	→	*	•	←	*	4	†	-	-	↓	1
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations					4			4			7	
Volume (vph)	0	0	0	18	294	81	28	80	0	0	88	69
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	14	12	12	15	12	12	12	12
Satd. Flow (prot)	0	0	0	0	1718	0	0	1816	0	0	1552	0
Flt Permitted					0.998			0.987				
Satd. Flow (perm)	0	0	0	0	1718	0	0	1816	0	0	1552	0
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		491			751			343			750	
Travel Time (s)		11.2			17.1			7.8			17.0	
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Heavy Vehicles (%)	2%	2%	2%	0%	4%	0%	0%	3%	0%	0%	5%	2%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	- 0	0	0	0	422	0	0	116	0	0	169	0
Sign Control		Stop			Stop			Stop			Stop	
Intersection Summary	" 是从1981		250.19	SO DEVI	39 13 79	(LEBS)	118,000 000	SHEX	DE BE	PROV	HEE IN	
Area Type:	CBD											
Control Type: Unsignalized												
Intersection Capacity Utiliza	ation 50.0%			10	CU Level	of Service	Α					
Analysis Period (min) 15												

Intersection Delay, s/veh	11.2											
Intersection LOS	В											
Movement	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBR
Vol, veh/h	0	0	0	0	0	18	294	81	0	28	80	0
Peak Hour Factor	0.92	0,93	0.93	0.93	0.92	0.93	0.93	0.93	0.92	0.93	0.93	0.93
Heavy Vehicles, %	2	2	2	2	2	0	4	0	2	0	3	0
Mymt Flow	0	0	0	0	0	19	316	87	0	30	86	0
Number of Lanes	0	0	0	0	0	0	1	0	0	0	11	0
Approach		1017	Patricia de	Agresage	The Air	WB	MI THE		9-745:V	NB		
Opposing Approach										SB		
Opposing Lanes						0				1		
Conflicting Approach Left						NB						
Conflicting Lanes Left						1				0		
Conflicting Approach Right						SB				WB		
Conflicting Lanes Right						1.				1		
HCM Control Delay						12.4				9.3		
						В				Α		
HCM LOS										Α		
		NBLn1	WBLn1	SBLn1	Maile VZ		nelle mid			Α		
HCM LOS	(W.)	NBLn1 26%	5%	0%	otelle VZ		nelle mid	K 15-41	1555-1	A	47-03	
HCM LOS	W.J.	26% 74%	5% 75%	0% 56%	otelle vz					A		9/3/3/19
Lane Vol Left, % Vol Thru, %	V-1	26%	5%	0% 56% 44%						A	67-13	
Lane Vol Left, % Vol Thru, % Vol Right, %	W.J	26% 74%	5% 75% 21% Stop	0% 56% 44% Stop						A		
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control	#3 1	26% 74% 0%	5% 75% 21% Stop 393	0% 56% 44% Stop 157						A		
Lane Vol Left, % Vol Thru, % Vol Right, %		26% 74% 0% Stop	5% 75% 21% Stop 393 18	0% 56% 44% Stop 157						A		
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol	W	26% 74% 0% Stop 108	5% 75% 21% Stop 393 18 294	0% 56% 44% Stop 157 0 88						A		
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane		26% 74% 0% Stop 108 28 80 0	5% 75% 21% Stop 393 18 294 81	0% 56% 44% Stop 157 0 88 69						A		
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol		26% 74% 0% Stop 108 28 80	5% 75% 21% Stop 393 18 294	0% 56% 44% Stop 157 0 88						A		
HCM LOS Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate		26% 74% 0% Stop 108 28 80 0	5% 75% 21% Stop 393 18 294 81	0% 56% 44% Stop 157 0 88 69						A		
HCM LOS Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp		26% 74% 0% Stop 108 28 80 0	5% 75% 21% Stop 393 18 294 81 423	0% 56% 44% Stop 157 0 88 69 169						A		
HCM LOS Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X)		26% 74% 0% Stop 108 28 80 0 116	5% 75% 21% Stop 393 18 294 81 423 1 0.526 4.485	0% 56% 44% Stop 157 0 88 69 169						A		
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd)		26% 74% 0% Stop 108 28 80 0 116 1	5% 75% 21% Stop 393 18 294 81 423 1 0.526	0% 56% 44% Stop 157 0 88 69 169 1						A		
HCM LOS Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N		26% 74% 0% Stop 108 28 80 0 116 1 0.167 5.164	5% 75% 21% Stop 393 18 294 81 423 1 0.526 4.485	0% 56% 44% Stop 157 0 88 69 169 1 0.228 4.871						A		
HCM LOS Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap		26% 74% 0% Stop 108 28 80 0 116 1 0.167 5.164 Yes	5% 75% 21% Stop 393 18 294 81 423 1 0.526 4.485 Yes	0% 56% 44% Stop 157 0 88 69 169 1 0.228 4.871 Yes						A		
HCM LOS Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time		26% 74% 0% Stop 108 28 80 0 116 1 0.167 5.164 Yes 690	5% 75% 21% Stop 393 18 294 81 423 1 0.526 4.485 Yes 803	0% 56% 44% Stop 157 0 88 69 169 1 0.228 4.871 Yes 733						A		
HCM LOS Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time HCM Lane V/C Ratio		26% 74% 0% Stop 108 28 80 0 116 1 0.167 5.164 Yes 690 3.231	5% 75% 21% Stop 393 18 294 81 423 1 0.526 4.485 Yes 803 2.53	0% 56% 44% Stop 157 0 88 69 169 1 0.228 4.871 Yes 733 2.933						A		
HCM LOS Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time		26% 74% 0% Stop 108 28 80 0 116 1 0.167 5.164 Yes 690 3.231 0.168	5% 75% 21% Stop 393 18 294 81 423 1 0.526 4.485 Yes 803 2.53 0.527	0% 56% 44% Stop 157 0 88 69 169 1 0.228 4.871 Yes 733 2.933 0.231						A		

ntersection	News				
ntersection Delay, s/veh Intersection LOS					
Movement	SBU	SBL	SBT	SBR	
Vol, veh/h	0	0	88	69	
Peak Hour Factor	0.92	0.93	0.93	0.93	
Heavy Vehicles, %	2	0	5	2	
Mvmt Flow	0	0	95	74	
Number of Lanes	0	0	1	0	
XXIII					
Approach	Same!	4.00	SB	- VIII - VIII	
Opposing Approach			NB		
Opposing Lanes			1		
Conflicting Approach Left			WB		
Conflicting Lanes Left			1		
Conflicting Approach Right					
Outlinding Approach Light			0		
Conflicting Lanes Right					
Conflicting Lanes Right HCM Control Delay			9.4		
Conflicting Lanes Right HCM Control Delay			9.4 A		
Conflicting Lanes Right					

	*	-	*	•	—	*	4	†	1	-	Į.	1
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations					4			सी			7	
Volume (vph)	0	0	0	15	217	78	29	101	0	0	122	77
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	14	12	12	15	12	12	12	12
Satd. Flow (prot)	0	0	0	0	1734	0	0	1860	0	0	1621	0
FIt Permitted					0.998			0.989				
Satd. Flow (perm)	0	0	0	0	1734	0	0	1860	0	0	1621	0
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		491			751			343			750	
Travel Time (s)		11.2			17.1			7.8			17.0	
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Heavy Vehicles (%)	2%	2%	2%	0%	2%	0%	0%	0%	0%	0%	0%	0%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	0	0	0	330	0	0	138	0	0	212	0
Sign Control		Stop			Stop			Stop			Stop	
Intersection Summary	Harris Mar	W SYSTEM	nis li V		NI SY -	180	1,0119	995	lyr V', S	1 Sept.	Re la	32.09
Area Type:	CBD											
Control Type: Unsignalized												
Intersection Capacity Utiliza	tion 48.9%			IC	CU Level of	of Service	: A					

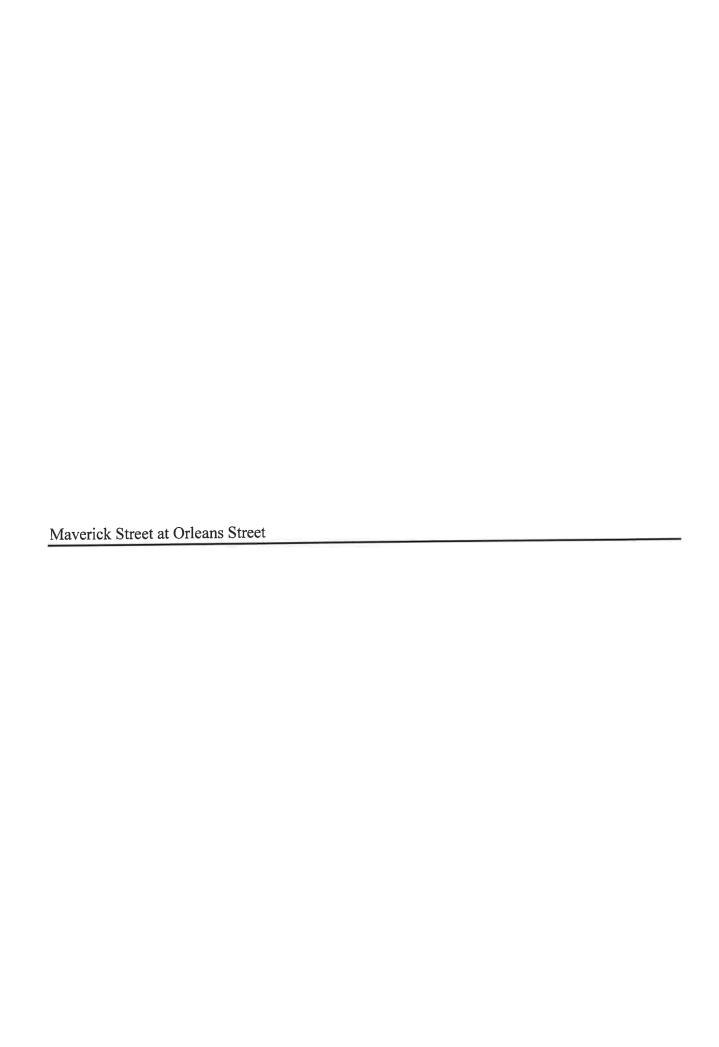
Intersection			×									- 3
Intersection Delay, s/veh Intersection LOS	10.1 B											
Movement	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBR
Vol, veh/h	0	0	0	0	0	15	217	78	0	29	101	0
Peak Hour Factor	0.92	0.94	0.94	0.94	0.92	0.94	0.94	0.94	0.92	0.94	0.94	0.94
Heavy Vehicles, %	2	2	2	2	2	0	2	0	2	0	0	0
Mvmt Flow	0	0	0	0	0	16	231	83	0	31	107	0
Number of Lanes	0	0	0	0	0	0	1	0	0	0	1	0
Approach						WB			- 2-17	NB		- 11
Opposing Approach										SB		
Opposing Lanes						0				1		
Conflicting Approach Left						NB						
Conflicting Lanes Left						1				0		
Conflicting Approach Right						SB				WB		
Conflicting Lanes Right						1				1		
HCM Control Delay						10.9				9.2		
110W Control Boldy						D				Λ		

В

Lane	NBLn1	WBLn1	SBLn1	
Vol Left, %	22%	5%	0%	
Vol Thru, %	78%	70%	61%	
Vol Right, %	0%	25%	39%	
Sign Control	Stop	Stop	Stop	
Traffic Vol by Lane	130	310	199	
LT Vol	29	15	0	
Through Vol	101	217	122	
RT Vol	0	78	77	
Lane Flow Rate	138	330	212	
Geometry Grp	1	1	1	
Degree of Util (X)	0.192	0.42	0.272	
Departure Headway (Hd)	4.988	4.581	4.632	
Convergence, Y/N	Yes	Yes	Yes	
Сар	715	782	771	
Service Time	3.049	2.628	2.688	
HCM Lane V/C Ratio	0.193	0.422	0.275	
HCM Control Delay	9.2	10.9	9.4	
HCM Lane LOS	A	В	Α	
HCM 95th-tile Q	0.7	2.1	1.1	

HCM LOS

ntersection					
ntersection Delay, s/veh					
Movement	SBU	SBL	SBT	SBR	
Vol, veh/h	0	0	122	77	
Peak Hour Factor	0.92	0.94	0.94	0.94	
Heavy Vehicles, %	2	0	0	0	
Mvmt Flow	0	0	130	82	
Number of Lanes	0	0	1	0	
Annonah	min tem	101100	SB	u 1 - 0 - 1	Nomination of State Stat
Approach	III = 1.00	KHRUA	NB		AND THE RESIDENCE OF THE PROPERTY OF THE PARTY OF THE PAR
Opposing Approach			IND		
Opposing Lanes			I		
Conflicting Approach Left			WB		
Conflicting Lanes Left			1.000		
Conflicting Approach Right			0		
Conflicting Lanes Right			0		
HCM Control Delay HCM LOS			9.4		
LIT'NA I TAC			Α		



	*	→	*	1	←	*	4	†	-	-	+	1
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations					4			4			1	
Volume (vph)	0	0	0	12	302	36	35	66	0	0	32	20
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	13	12	12	13	12	12	14	12
Satd. Flow (prot)	0	0	0	0	1704	0	0	1714	0	0	1630	0
Flt Permitted					0.998			0.983				
Satd. Flow (perm)	0	0	0	0	1704	0	0	1714	0	0	1630	0
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		751			688			285			704	
Travel Time (s)		17.1			15.6			6.5			16.0	
Peak Hour Factor	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85
Heavy Vehicles (%)	2%	2%	2%	0%	2%	3%	0%	2%	0%	0%	3%	11%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	0	0	0	411	0	0	119	0	0	62	0
Sign Control		Stop			Stop			Stop			Stop	

Area Type: CBD Control Type: Unsignalized

Intersection Capacity Utilization 40.2%

Analysis Period (min) 15

Intersection Delay, s/veh Intersection LOS	10.6 B											
Movement	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBR
Vol, veh/h	0	0	0	0	0	12	302	36	0	35	66	0
Peak Hour Factor	0.92	0.85	0.85	0.85	0.92	0.85	0.85	0.85	0.92	0.85	0.85	0.85
Heavy Vehicles, %	2	2	2	2	2	.0	2	3	2	0	2	0
Mvmt Flow	0	0	0	0	0	14	355	42	0	41	78	0
Number of Lanes	0	0	0	0	0	0	1	0	0	0	1	0

WB	NB
	SB
0	1
NB	
1	0
SB	WB
1	1
11.4	9
В	A
	0 NB 1 SB 1 11.4

Lane	NBLn1	WBLn1	SBLn1	
Vol Left, %	35%	3%	0%	
Vol Thru, %	65%	86%	62%	
Vol Right, %	0%	10%	38%	
Sign Control	Stop	Stop	Stop	
Traffic Vol by Lane	101	350	52	
LT Vol	35	12	0	
Through Vol	66	302	32	
RT Vol	0	36	20	
Lane Flow Rate	119	412	61	
Geometry Grp	1	1	1	
Degree of Util (X)	0.165	0.491	0.082	
Departure Headway (Hd)	4.987	4.294	4.818	
Convergence, Y/N	Yes	Yes	Yes	
Сар	719	840	742	
Service Time	3.024	2.318	2.859	
HCM Lane V/C Ratio	0.166	0.49	0.082	
HCM Control Delay	9	11.4	8.3	
HCM Lane LOS	Α	В	Α	
HCM 95th-tile Q	0.6	2.8	0.3	

ntersection			-11/10/17	VIIE	
ntersection Delay, s/veh ntersection LOS					
Movement	SBU	SBL	SBT	SBR	
Vol, veh/h	0	0	32	20	
Peak Hour Factor	0.92	0.85	0.85	0.85	
Heavy Vehicles, %	2	0	3	11	
Mvmt Flow	0	0	38	24	
Number of Lanes	0	0	1	0	
Approach			SB	1000	
Opposing Approach			NB		
Opposing Lanes			1		
Conflicting Approach Left			WB		
Conflicting Lanes Left			. 1		
Conflicting Approach Right					
Conflicting Lanes Right			0		
HCM Control Delay			8.3		
			Α		

	*	\rightarrow	*	1	←	*	4	†	1	-	↓	1
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations					4			स			1>	
Volume (vph)	0	0	0	13	225	42	39	51	0	0	49	14
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	13	12	12	13	12	12	14	12
Satd. Flow (prot)	0	0	0	0	1701	0	0	1730	0	0	1767	0
Flt Permitted					0.998			0.979				
Satd. Flow (perm)	0	0	0	0	1701	0	0	1730	0	0	1767	0
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		751			688			285			704	
Travel Time (s)		17.1			15.6			6.5			16.0	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	2%	0%	2%	0%	0%	0%	0%	0%	0%	0%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	0	0	0	311	0	0	100	0	0	70	0
Sign Control		Stop			Stop			Stop			Stop	
halaman the original to the control of the control	4 10 10	THE PARTY OF THE P		ATT	III C III C		COLUMN TO SERVICE STATE OF THE	VIII	100		1000 1200	A

Area Type:

CBD

Control Type: Unsignalized

Intersection Capacity Utilization 35.5%

Analysis Period (min) 15

Intersection Delay, s/veh Intersection LOS	9.2											
	A	ED)	EDT	EDD	MATERIA	WDI	WOT	WDD	MOLL	LID!	LIDT	Man
Movement Val. web/b	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBR
Vol, veh/h Peak Hour Factor	0	0	0	0	0	13	225	42	0	39	51	0
	0.92	0.90	0.90 2	0,90 2	0.92	0.90	0.90	0.90	0.92	0.90	0.90	0.90
Heavy Vehicles, % Mvmt Flow	2	0		0	2	0	2 250	0	2	0	0	0
Number of Lanes	0 0	0	0 0	0	0 0	14 0	250	47 0	0	43	57 1	0
Number of Lanes	U	U	U	U	U	U		U	U	U	- 1	0
Approach						WB				NB	100	- 4
Opposing Approach										SB		
Opposing Lanes						0				1		
Conflicting Approach Left						NB						
Conflicting Lanes Left						1				0		
Conflicting Approach Right						SB				WB		
Conflicting Lanes Right						1				1		
HCM Control Delay						9.7				8.5		
						0.7				0.0		
HCM LOS						A				A		
HCM LOS	N	VBLn1	WBLn1	SBLn1								
Lane Vol Left, %	N	43%	WBLn1	0%								
Lane Vol Left, % Vol Thru, %	N											
Lane Vol Left, % Vol Thru, %	N	43%	5%	0% 78% 22%								
Lane Vol Left, % Vol Thru, % Vol Right, %	N	43% 57%	5% 80%	0% 78% 22%								
ACM LOS Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control	۸	43% 57% 0%	5% 80% 15%	0% 78%								
ACM LOS Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Fraffic Vol by Lane	N	43% 57% 0% Stop	5% 80% 15% Stop	0% 78% 22% Stop								
HCM LOS Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Fraffic Vol by Lane LT Vol	N	43% 57% 0% Stop 90	5% 80% 15% Stop 280	0% 78% 22% Stop 63								
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Fraffic Vol by Lane LT Vol Fhrough Vol RT Vol	N	43% 57% 0% Stop 90 39	5% 80% 15% Stop 280 13	0% 78% 22% Stop 63 0								
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Fraffic Vol by Lane LT Vol Fhrough Vol RT Vol	N	43% 57% 0% Stop 90 39 51	5% 80% 15% Stop 280 13 225	0% 78% 22% Stop 63 0 49								
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate	N	43% 57% 0% Stop 90 39 51 0	5% 80% 15% Stop 280 13 225 42	0% 78% 22% Stop 63 0 49								
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp		43% 57% 0% Stop 90 39 51 0	5% 80% 15% Stop 280 13 225 42 311	0% 78% 22% Stop 63 0 49 14 70								
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Fraffic Vol by Lane LT Vol Fhrough Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X)		43% 57% 0% Stop 90 39 51 0	5% 80% 15% Stop 280 13 225 42 311	0% 78% 22% Stop 63 0 49 14 70								
ACM LOS Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Fraffic Vol by Lane LT Vol Fhrough Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd)		43% 57% 0% Stop 90 39 51 0 100 1	5% 80% 15% Stop 280 13 225 42 311 1 0.365	0% 78% 22% Stop 63 0 49 14 70 1 0.089								
HCM LOS Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Fraffic Vol by Lane LT Vol Frough Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N		43% 57% 0% Stop 90 39 51 0 100 1 0.133 4.771	5% 80% 15% Stop 280 13 225 42 311 1 0.365 4.228	0% 78% 22% Stop 63 0 49 14 70 1 0.089 4.592								
ACM LOS Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Fraffic Vol by Lane LT Vol Fhrough Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap		43% 57% 0% Stop 90 39 51 0 100 1 0.133 4.771 Yes 752	5% 80% 15% Stop 280 13 225 42 311 1 0.365 4.228 Yes 852	0% 78% 22% Stop 63 0 49 14 70 1 0.089 4.592 Yes 781								
ACM LOS Lane Vol Left, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time		43% 57% 0% Stop 90 39 51 0 100 1 0.133 4.771 Yes 752 2.795	5% 80% 15% Stop 280 13 225 42 311 1 0.365 4.228 Yes 852 2.245	0% 78% 22% Stop 63 0 49 14 70 1 0.089 4.592 Yes 781 2.617								
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time HCM Lane V/C Ratio		43% 57% 0% Stop 90 39 51 0 100 1 0.133 4.771 Yes 752 2.795 0.133	5% 80% 15% Stop 280 13 225 42 311 1 0.365 4.228 Yes 852 2.245 0.365	0% 78% 22% Stop 63 0 49 14 70 1 0.089 4.592 Yes 781 2.617 0.09								
HCM LOS		43% 57% 0% Stop 90 39 51 0 100 1 0.133 4.771 Yes 752 2.795	5% 80% 15% Stop 280 13 225 42 311 1 0.365 4.228 Yes 852 2.245	0% 78% 22% Stop 63 0 49 14 70 1 0.089 4.592 Yes 781 2.617								

Intersection					
Intersection Delay, s/veh Intersection LOS					
Movement	SBU	SBL	SBT	SBR	
Vol, veh/h	0	0	49	14	
Peak Hour Factor	0.92	0.90	0.90	0.90	
Heavy Vehicles, %	2	0	0	0	
Mvmt Flow	0	0	54	16	
Number of Lanes	0	0	1	0	
Approach			SB		
Opposing Approach			NB		
Opposing Lanes			1		
Conflicting Approach Left			WB		
Conflicting Lanes Left			1		
Conflicting Approach Right					
Conflicting Lanes Right			0		
HCM Control Delay			8.1		
			A		

	<i>•</i>	\rightarrow	*	1	←	*	4	†	-	-	↓	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations					4			र्स			7>	
Volume (vph)	0	0	0	13	332	42	38	71	0	0	38	21
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	13	12	12	13	12	12	14	12
Satd. Flow (prot)	0	0	0	0	1702	0	0	1715	0	0	1640	0
Flt Permitted					0.998			0.983				
Satd. Flow (perm)	0	0	0	0	1702	0	0	1715	0	0	1640	0
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		751			688			285			704	
Travel Time (s)		17.1			15.6			6.5			16.0	
Peak Hour Factor	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85
Heavy Vehicles (%)	2%	2%	2%	0%	2%	3%	0%	2%	0%	0%	3%	11%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	0	0	- 0	455	0	0	129	0	0	70	0
Sign Control		Stop			Stop			Stop			Stop	

Area Type:

CBD

Control Type: Unsignalized

Intersection Capacity Utilization 42.9%

Analysis Period (min) 15

V 1												
Intersection Intersection Delay, s/veh	11.5											
Intersection LOS	В.											
Movement	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBF
Vol, veh/h	0	0	0	0	0	13	332	42	0	38	71	
Peak Hour Factor	0.92	0.85	0.85	0.85	0.92	0.85	0.85	0.85	0.92	0.85	0.85	0.8
Heavy Vehicles, %	2	2	2	2	2	0	2	3	2	0	2	
Mvmt Flow	0	0	0	0	0	15	391	49	0	45	84	-
Number of Lanes	0	0	0	0	0	0	1	0	0	0	1	
Approach						WB				NB		
Opposing Approach										SB		
Opposing Lanes						0				1		
Conflicting Approach Left						NB						
		:				1				0		
Conflicting Approach Right	*, * .					SB				WB		
Conflicting Lanes Right						1				1		
HCM Control Delay						12.6				9.3		
HCM LOS						В				Α		
Lane		NBLn1	WBLn1	SBLn1								-
Lane Vol Left, %		35%	3%	0%								
		35% 65%	3% 86%	0% 64%								
Vol Left, % Vol Thru, %		35%	3% 86% 11%	0% 64% 36%								
Vol Left, %		35% 65%	3% 86% 11% Stop	0% 64% 36% Stop								
Vol Left, % Vol Thru, % Vol Right, %		35% 65% 0% Stop 109	3% 86% 11% Stop 387	0% 64% 36% Stop 59								
Vol Left, % Vol Thru, % Vol Right, % Sign Control		35% 65% 0% Stop	3% 86% 11% Stop 387 13	0% 64% 36% Stop 59								
Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol		35% 65% 0% Stop 109	3% 86% 11% Stop 387 13 332	0% 64% 36% Stop 59 0								
Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane		35% 65% 0% Stop 109 38 71 0	3% 86% 11% Stop 387 13 332 42	0% 64% 36% Stop 59 0 38 21								
Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol		35% 65% 0% Stop 109 38 71	3% 86% 11% Stop 387 13 332	0% 64% 36% Stop 59 0								
Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate		35% 65% 0% Stop 109 38 71 0	3% 86% 11% Stop 387 13 332 42 455	0% 64% 36% Stop 59 0 38 21 69								
Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol		35% 65% 0% Stop 109 38 71 0	3% 86% 11% Stop 387 13 332 42 455 1	0% 64% 36% Stop 59 0 38 21 69 1 0.096								
Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp		35% 65% 0% Stop 109 38 71 0 128 1	3% 86% 11% Stop 387 13 332 42 455 1 0.549 4.342	0% 64% 36% Stop 59 0 38 21 69 1 0.096 4.961								
Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X)		35% 65% 0% Stop 109 38 71 0 128 1	3% 86% 11% Stop 387 13 332 42 455 1	0% 64% 36% Stop 59 0 38 21 69 1 0.096 4.961 Yes								
Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N		35% 65% 0% Stop 109 38 71 0 128 1 0.182 5.109	3% 86% 11% Stop 387 13 332 42 455 1 0.549 4.342 Yes 829	0% 64% 36% Stop 59 0 38 21 69 1 0.096 4.961 Yes 719								
Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd)		35% 65% 0% Stop 109 38 71 0 128 1 0.182 5.109 Yes	3% 86% 11% Stop 387 13 332 42 455 1 0.549 4.342 Yes	0% 64% 36% Stop 59 0 38 21 69 1 0.096 4.961 Yes 719 3.012								
Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap		35% 65% 0% Stop 109 38 71 0 128 1 0.182 5.109 Yes 700	3% 86% 11% Stop 387 13 332 42 455 1 0.549 4.342 Yes 829	0% 64% 36% Stop 59 0 38 21 69 1 0.096 4.961 Yes 719								
Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time		35% 65% 0% Stop 109 38 71 0 128 1 0.182 5.109 Yes 700 3.156	3% 86% 11% Stop 387 13 332 42 455 1 0.549 4.342 Yes 829 2.372	0% 64% 36% Stop 59 0 38 21 69 1 0.096 4.961 Yes 719 3.012								
Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time HCM Lane V/C Ratio		35% 65% 0% Stop 109 38 71 0 128 1 0.182 5.109 Yes 700 3.156 0.183	3% 86% 11% Stop 387 13 332 42 455 1 0.549 4.342 Yes 829 2.372 0.549	0% 64% 36% Stop 59 0 38 21 69 1 0.096 4.961 Yes 719 3.012 0.096								

Intersection					
Intersection Delay, s/veh Intersection LOS					
Movement	SBU	SBL	SBT	SBR	
Vol, veh/h	0	0	38	21	
Peak Hour Factor	0.92	0.85	0.85	0.85	
Heavy Vehicles, %	2	0	3	11	
Mvmt Flow	0	0	45	25	
Number of Lanes	0	0	1	0	
Approach	APT A	100	SB		
Opposing Approach			NB		
Opposing Lanes			1		
Conflicting Approach Left			WB		
Conflicting Lanes Left			1		
Conflicting Approach Right					
Conflicting Lanes Right			0		
HCM Control Delay			8.5		
HCM LOS			Α		

	1	→	*	1	←	*	4	†	-	-	↓	1
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations					4			सी			7	
Volume (vph)	0	0	0	14	252	48	43	55	0	0	61	15
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	13	12	12	13	12	12	14	12
Satd. Flow (prot)	0	0	0	0	1699	0	0	1728	0	0	1775	0
Flt Permitted					0.998			0.978				
Satd. Flow (perm)	0	0	0	0	1699	0	0	1728	0	0	1775	0
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		751			688			285			704	
Travel Time (s)	·	17.1			15.6			6.5			16.0	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	2%	0%	2%	0%	0%	0%	0%	0%	0%	0%
Shared Lane Traffic (%)	270	_,,										
Lane Group Flow (vph)	. 0	0	0	0	349	0	0	109	0	0	85	0
Sign Control		Stop			Stop			Stop			Stop	

Area Type:

CBD

Control Type: Unsignalized

Intersection Capacity Utilization 38.0%

Analysis Period (min) 15

Intersection Intersection Delay, s/veh	9.7											
Intersection LOS	A											
Movement	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBF
Vol, veh/h	: 0:	0	0	0	0	14	252	48	0	43	55	(
Peak Hour Factor	0.92	0.90	0.90	0.90	0.92	0,90	0.90	0.90	0.92	0.90	0.90	0.90
Heavy Vehicles, %	2	2	2	2	2	0	2	0	2	0	0	(
Mvmt Flow	0	0	0	0	0	16	280	53	0	48	61	(
Number of Lanes	0	0	0	0	0	0	1	0	0	0	1	(
Approach	-			6 3		WB	- 2	-1		NB		
Opposing Approach										SB		
Opposing Lanes						0				1		
Conflicting Approach Left						NB						
Conflicting Lanes Left						1				0		
Conflicting Approach Right						SB				WB		
Conflicting Lanes Right						1				1		
HCM Control Delay						10.3				8.8		
HCM LOS						В				Α		
			12.60.1.1.1									
Lane		NBLn1		SBLn1								
Vol Left, %		44%	4%	0%								
Vol Thru, %		56%	80%	80%								
Vol Right, %		0%	15%	20%								
Sign Control		Stop	Stop	Stop								
Traffic Vol by Lane		0.0	244	76								
		98	314									
		43	14	0								
LT Vol Through Vol		43 55	14 252	0 61								
LT Vol Through Vol		43 55 0	14 252 48	0 61 15								
LT Vol Through Vol RT Vol		43 55 0 109	252 48 349	0 61 15 84								
LT Vol Through Vol RT Vol Lane Flow Rate		43 55 0 109 1	252 48 349 1	0 61 15 84 1								
LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp		43 55 0 109 1 0.148	14 252 48 349 1 0.415	0 61 15 84 1 0.111								
LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp		43 55 0 109 1	252 48 349 1 0.415 4.287	0 61 15 84 1 0.111 4.715								
LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd)		43 55 0 109 1 0.148 4.887 Yes	252 48 349 1 0.415 4.287 Yes	0 61 15 84 1 0.111 4.715 Yes								
LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N		43 55 0 109 1 0.148 4.887 Yes 734	14 252 48 349 1 0.415 4.287 Yes 841	0 61 15 84 1 0.111 4.715 Yes 759								
LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap		43 55 0 109 1 0.148 4.887 Yes 734 2.919	252 48 349 1 0.415 4.287 Yes 841 2.311	0 61 15 84 1 0.111 4.715 Yes 759 2.748								
LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time		43 55 0 109 1 0.148 4.887 Yes 734	252 48 349 1 0.415 4.287 Yes 841 2.311 0.415	0 61 15 84 1 0.111 4.715 Yes 759 2.748 0.111								
LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time HCM Lane V/C Ratio		43 55 0 109 1 0.148 4.887 Yes 734 2.919	252 48 349 1 0.415 4.287 Yes 841 2.311	0 61 15 84 1 0.111 4.715 Yes 759 2.748								
LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X)		55 0 109 1 0.148 4.887 Yes 734 2.919 0.149	252 48 349 1 0.415 4.287 Yes 841 2.311 0.415	0 61 15 84 1 0.111 4.715 Yes 759 2.748 0.111								

Intersection					
Intersection Delay, s/veh					
Movement	SBU	SBL	SBT	SBR	
Vol, veh/h	0	0	61	15	
Peak Hour Factor	0.92	0.90	0.90	0.90	
Heavy Vehicles, %	2	0	0	0	
Mvmt Flow	0	0	68	17	
Number of Lanes	0	0	1	0	
A			SB	NEW THE	
Approach			LI CATHE		
Opposing Approach			NB		
Opposing Lanes			1		
Conflicting Approach Left			WB		
Conflicting Lanes Left			- 3 12		
Conflicting Approach Right					
Conflicting Lanes Right			0		
HCM Control Delay			8.3		
HCM LOS			Α		

	*	-	*	-	←	1	4	†	-	1	↓	1
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations					4			स			7	
Volume (vph)	0	0	0	15	334	42	38	71	0	0	38	21
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	13	12	12	13	12	12	14	12
Satd. Flow (prot)	0	0	0	0	1704	0	0	1715	0	0	1640	0
Flt Permitted					0.998			0.983				
Satd. Flow (perm)	0	0	0	0	1704	0	0	1715	0	0	1640	0
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		751			688			285			704	
Travel Time (s)		17.1			15.6			6.5			16.0	
Peak Hour Factor	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85
Heavy Vehicles (%)	2%	2%	2%	0%	2%	3%	0%	2%	0%	0%	3%	11%
Shared Lane Traffic (%)			- 15									
Lane Group Flow (vph)	0	0	0	0	460	0	0	129	0	0	70	0
Sign Control		Stop			Stop			Stop			Stop	
for the Columns		_	-	-		- 1						

Intersection Summary
Area Type:

CBD

Control Type: Unsignalized

Intersection Capacity Utilization 43.1%

Analysis Period (min) 15

Intersection	TON STATE	Tele IV	C CALL STA			Mu m	17 P					1100
Intersection Delay, s/veh	11.6											
Intersection LOS	В											
Movement	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBR
Vol, veh/h	0	0	0	0	0	15	334	42	0	38	71	0
Peak Hour Factor	0.92	0.85	0.85	0.85	0.92	0.85	0.85	0.85	0.92	0.85	0.85	0.85
Heavy Vehicles, %	2	2	2	2	2	0	2	3	2	0	2	0
Mvmt Flow	0	0	0	0	0	18	393	49	0	45	84	0
Number of Lanes	0	0	0	0	0	0	1	0	0	0	1	0
Approach	E NEVER	75/3/9) mej	W 1.	er Sins	WB	walst		Viji jan	NB	white	ō,w)
Opposing Approach										SB		
Opposing Lanes						0				1		
Conflicting Approach Left						NB						
Conflicting Lanes Left						1				0		
Conflicting Approach Right						SB				WB		
Conflicting Lanes Right						1				1		
						12.7				9.3		
HCM Control Delay												
HCM LOS						В				Α		
						В		1 7		А		
		NBLn1	WBLn1	SBLn1		В	W. S. W.			A		
HCM LOS		35%	4%	0%		В	n/alw)		1 19 19 19 19 19 19 19 19 19 19 19 19 19	A		
HCM LOS		35% 65%	4% 85%	0% 64%		В	10 20 100			Α.	4	
Lane Vol Left, %		35%	4% 85% 11%	0% 64% 36%)	В	11(3)W)			A		
Lane Vol Left, % Vol Thru, %		35% 65%	4% 85% 11% Stop	0% 64% 36% Stop		В				A		
Lane Vol Left, % Vol Thru, % Vol Right, %		35% 65% 0% Stop 109	4% 85% 11% Stop 391	0% 64% 36% Stop 59		В				A	SH ST	
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control		35% 65% 0% Stop 109 38	4% 85% 11% Stop 391 15	0% 64% 36% Stop 59		В	ni si o		7	A		
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane		35% 65% 0% Stop 109	4% 85% 11% Stop 391 15 334	0% 64% 36% Stop 59 0		В				A		
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol		35% 65% 0% Stop 109 38	4% 85% 11% Stop 391 15	0% 64% 36% Stop 59 0 38 21		В				A		
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol		35% 65% 0% Stop 109 38 71	4% 85% 11% Stop 391 15 334	0% 64% 36% Stop 59 0		В				A		
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate		35% 65% 0% Stop 109 38 71 0	4% 85% 11% Stop 391 15 334 42	0% 64% 36% Stop 59 0 38 21		В			7. T. J.	A		
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp		35% 65% 0% Stop 109 38 71 0	4% 85% 11% Stop 391 15 334 42 460	0% 64% 36% Stop 59 0 38 21 69		В			7. 7	A		
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate		35% 65% 0% Stop 109 38 71 0 128	4% 85% 11% Stop 391 15 334 42 460	0% 64% 36% Stop 59 0 38 21 69		В				A		
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd)		35% 65% 0% Stop 109 38 71 0 128 1	4% 85% 11% Stop 391 15 334 42 460 1	0% 64% 36% Stop 59 0 38 21 69 1 0.096		В				A		
HCM LOS Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N		35% 65% 0% Stop 109 38 71 0 128 1 0.182 5.119	4% 85% 11% Stop 391 15 334 42 460 1 0.555 4.344	0% 64% 36% Stop 59 0 38 21 69 1 0.096 4.971		В				A		
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap		35% 65% 0% Stop 109 38 71 0 128 1 0.182 5.119 Yes	4% 85% 11% Stop 391 15 334 42 460 1 0.555 4.344 Yes	0% 64% 36% Stop 59 0 38 21 69 1 0.096 4.971 Yes		В				A		
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time		35% 65% 0% Stop 109 38 71 0 128 1 0.182 5.119 Yes 699	4% 85% 11% Stop 391 15 334 42 460 1 0.555 4.344 Yes 828	0% 64% 36% Stop 59 0 38 21 69 1 0.096 4.971 Yes 718		В				A		
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time HCM Lane V/C Ratio		35% 65% 0% Stop 109 38 71 0 128 1 0.182 5.119 Yes 699 3.168	4% 85% 11% Stop 391 15 334 42 460 1 0.555 4.344 Yes 828 2.374	0% 64% 36% Stop 59 0 38 21 69 1 0.096 4.971 Yes 718 3.025		В			7. 7	A		
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time		35% 65% 0% Stop 109 38 71 0 128 1 0.182 5.119 Yes 699 3.168 0.183	4% 85% 11% Stop 391 15 334 42 460 1 0.555 4.344 Yes 828 2.374 0.556	0% 64% 36% Stop 59 0 38 21 69 1 0.096 4.971 Yes 718 3.025 0.096		В				A		

Intersection					
Intersection Delay, s/veh Intersection LOS					
Movement	SBU	SBL	SBT	SBR	
Vol, veh/h	0	0	38	21	
Peak Hour Factor	0.92	0.85	0.85	0.85	
Heavy Vehicles, %	2	0	3	11	
Mvmt Flow	0	0	45	25	
Number of Lanes	0	0	1	0	
Approach	8/8 5/8	stallen i	SB	114 AN 11	
Opposing Approach			NB		
Opposing Lanes			1		
Conflicting Approach Left			WB		
Conflicting Lanes Left			1		
Conflicting Approach Right					
Conflicting Lanes Right			0		
HCM Control Delay			8.6		
HCM LOS			Α		

	*	→	*	1	←	*	1	†	-	1	1	1
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations					4			4			1	
Volume (vph)	0	0	0	16	253	48	43	55	0	0	61	15
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	13	12	12	13	12	12	14	12
Satd. Flow (prot)	0	0	0	0	1699	0	0	1728	0	0	1775	0
Flt Permitted	·				0.997			0.978				
Satd. Flow (perm)	0	0	0	0	1699	0	0	1728	0	0	1775	0
Link Speed (mph)	· ·	30			30			30			30	
Link Distance (ft)		751			688			285			704	
Travel Time (s)		17.1			15.6			6.5			16.0	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	2%	2%	2%	0%	2%	0%	0%	0%	0%	0%	0%	0%
Shared Lane Traffic (%)	270	=,0	_,-	2012								
Lane Group Flow (vph)	0	0	0	0	352	0	0	109	0	0	85	0
Sign Control		Stop			Stop			Stop			Stop	

Area Type:

CBD

Control Type: Unsignalized

Intersection Capacity Utilization 38.2% Analysis Period (min) 15

Intersection Delay, s/veh	9.8											
Intersection LOS	Α											
Movement	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBR
Vol, veh/h	0	0	0	0	0	16	253	48	0	43	55	0
Peak Hour Factor	0.92	0.90	0.90	0.90	0.92	0.90	0.90	0.90	0.92	0.90	0.90	0.90
Heavy Vehicles, %	2	2	2	2	2	0	2	0	2	0	0	0
Mvmt Flow	0	0	0	0	0	18	281	53	0	48	61	0
Number of Lanes	0_	0	0	0	0	0	1	0	0	0	1	0
Approach		8 34	0.24		no mega	WB			133 FW	NB	W. M	Barry.
Opposing Approach										SB		
Opposing Lanes						0				1		
Conflicting Approach Left						NB						
Conflicting Lanes Left						1				0		
Conflicting Approach Right						SB				WB		
Conflicting Lanes Right						1				1		
HCM Control Delay						10.4				8.8		
HCM LOS						В				Α		
Lane	N	VBLn1	WBLn1	SBLn1		- CW.			Santa 8	10 A TOTAL III	z 15/88	3/82
Vol Left, %	7,	44%	5%	0%								
Vol Thru, %		56%	80%	80%								
Vol Right, %		0%	15%	20%								
Sign Control		Stop	Stop	Stop								
Traffic Vol by Lane		98	317	76								
LT Vol		43	16	0								
Through Vol		55	253	61								
RT Vol		0	48	15								
Lane Flow Rate		109	352	84								
Geometry Grp		1	1	1								
Geometry Gip			0.42	0.111								
		0.148	0.42	0.111								
Degree of Util (X)		0.148 4.895	4.289	4.723								
Degree of Util (X) Departure Headway (Hd)												
Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap		4.895	4.289	4.723								

Service Time

HCM Lane V/C Ratio

HCM Control Delay

HCM Lane LOS

HCM 95th-tile Q

2.929

0.149

8.8

A 0.5 2.314

0.42 10.4

В

2.1

2.759

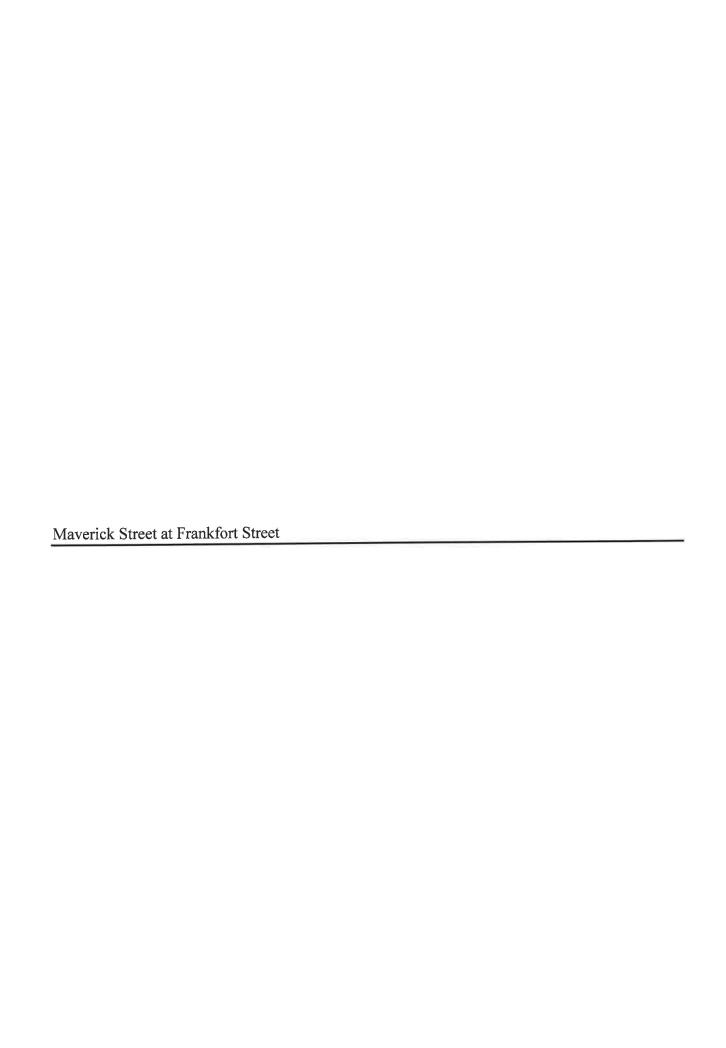
0.111

8.4

0.4

Α

Intersection Delay, s/veh Intersection LOS		. 1154			December 1
Intersection LOS					
Movement	SBU	SBL	SBT	SBR	A CONTRACTOR OF THE STATE OF TH
Vol, veh/h	0	0	61	15	
Peak Hour Factor	0.92	0.90	0.90	0.90	
Heavy Vehicles, %	2	0	0	0	
Mvmt Flow	0	0	68	17	
Number of Lanes	0	0	1	0	
Approach	17 m		SB	WALL STATE	
Opposing Approach			NB		
Opposing Lanes			1		
Conflicting Approach Left			WB		
Conflicting Lanes Left			1		
Conflicting Approach Right					
Conflicting Lanes Right			0		
HCM Control Delay			8.4		
HCM LOS			Α		



	•	→	*	1	4-	*	4	†	-	-	↓	1
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	15%(11)				4			4			7	
Volume (vph)	0	0	0	3	324	47	1	4	0	0	3	25
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	13	12	12	12	12	12	14	12
Satd. Flow (prot)	0	0	0	0	1707	0	0	1696	0	0	1545	0
FIt Permitted								0.992				
Satd. Flow (perm)	0	0	0	0	1707	0	0	1696	0	0	1545	0
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		688			881			230			651	
Travel Time (s)		15.6			20.0			5.2			14.8	
Peak Hour Factor	0.86	0.86	0.86	0.86	0.86	0.86	0.86	0.86	0.86	0.86	0.86	0.86
Heavy Vehicles (%)	2%	2%	2%	0%	2%	0%	0%	0%	0%	0%	0%	4%
Shared Lane Traffic (%)			-,-									
Lane Group Flow (vph)	0	0	0	0	435	0	0	6	0	0	32	0
Sign Control		Free			Free			Stop			Stop	
Intersection Summary			1772	Ag Ha	WAS:	/4-4 E	ST TAS .	A. Carlo	S. Park	Bit.	Ar and	X GIL
Area Type:	CBD											
Control Type: Unsignalized												

Control Type: Unsignalized

Intersection Capacity Utilization 32.3%

ICU Level of Service A

Analysis Period (min) 15

ection 0.1	7	-										-
elay, s/veh 0.												
ement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	S
veh/h	0	0	0	3	324	47	1	4	0	0	3	
flicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	
Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	
Channelized	-	-	None	-	-	None	-	- 10	None	Оюр	Olop	No
rage Length	+				-	-			110110			146
h in Median Storage, #	-	0			0	_		0	_		0	
ade, %		0			0			0				
ak Hour Factor	86	86	86	86	86	86	86	86	86	- 00	0	
eavy Vehicles, %	2	2	2	0	2	0	0	0		86	86	
ivmt Flow	0	0	0	3	377	55	1	5	0	0	0	
Williams Services					077	00	!	5	0	0	3	
VajoriMinor				Major2			Minor1	-	100	Minor2		
Conficting Flow All				0	0	0	427	438	0	413	411	4
Stage 1					14		0	0	-	411	411	4
Stage 2				-	-		427	438		2		
citical Hdwy							6.4	6.5			0	
Critical Hdwy Stg 1							-	0.5		6.4	6.5	6.
Critical Hdwy Stg 2				-			5.4	5.5	-	5.4	5.5	
Follow-up Hdwy							3.5		-	-	-	-
Pot Cap-1 Maneuver						-		4		3.5		3.33
Stage 1					-	-	588	515		599	534	64
Stage 2						-	-		*	674	598	
Platoon blocked, %					-	*	662	582	-	-	-	
Nov Cap-1 Maneuver						*						
Nov Cap-2 Maneuver				*		•	588	0	-	599	0	64
Stage 1				*		*	588	0	-	599	0	
Stage 2				7.	-		-	0	- 7	674	0	
olage 2					-	*	662	0	*	+	0	
proach				WB			NB					
CM Control Delay, s				110		_	IND			SB		
CM LOS										10.9		
							-			В		
nor Lane/Major Mvmt N	VBLn1 V	VBL \	WBT WE	D CDI nd								
pacity (veh/h)	WEIT Y	ADE I	VVDI VVE	R SBLn1								
M Lane V/C Ratio		-	7	- 642								
M Control Delay (s)	7	2		- 0.051								
M Lane LOS	*	*	7	- 10.9								
M 95th %tile Q(veh)		*	*	- B								
VI JULI WILLE Q(Veh)	-	5.0	2	- 0.2								

	<i>•</i>	\rightarrow	*	1	←	*	4	†	-	-	ļ	1
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations					4			4			1>	
Volume (vph)	0	0	0	9	251	20	2	6	0	0	6	27
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	13	12	12	12	12	12	14	12
Satd. Flow (prot)	0	0	0	0	1715	0	0	1691	0	0	1622	0
Flt Permitted					0.998			0.989				
Satd. Flow (perm)	0	0	0	0	1715	0	0	1691	0	0	1622	0
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		688			881			230			651	
Travel Time (s)		15.6			20.0			5.2			14.8	
Peak Hour Factor	0.83	0.83	0.83	0.83	0.83	0.83	0.83	0.83	0.83	0.83	0.83	0.83
Heavy Vehicles (%)	2%	2%	2%	0%	2%	0%	0%	0%	0%	0%	0%	0%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	0	0	0	337	0	0	9	0	0	40	0
Sign Control		Free			Free			Stop			Stop	

Area Type:

CBD

Control Type: Unsignalized

Intersection Capacity Utilization 26.6% Analysis Period (min) 15

Intersection												9-1
Int Delay, s/veh	1											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Vol, veh/h	0	0	0	9	251	20	2	6	0	0	6	27
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	÷	:=	None	-	-	None
Storage Length	-	-	-	-				*	-		-	
Veh in Median Storage, #	-	0	-	*	0			0			0	
Grade, %		0	-	4	0	-	-	0	.2		0	
Peak Hour Factor	83	83	83	83	83	83	83	83	83	83	83	83
Heavy Vehicles, %	2	2	2	0	2	0	0	0	0	0	0	0
Mvmt Flow	0	0	0	11	302	24	2	7	0	0	7	33
Major/Minor				Major2			Minor1	100		Minor2	_3	
Conflicting Flow All				0	0	0	356	348	0	340	336	314
Stage 1					-	-	0	0	-	336	336	
Stage 2				-			356	348		4	0	
Critical Hdwy							6.4	6.5		6.4	6.5	6.2
Critical Hdwy Stg 1						2	0.1	-		5.4	5.5	
					- 2		5.4	5.5		-	-	
Critical Hdwy Stg 2							3.5	4		3.5	4	3.3
Follow-up Hdwy							646	579		660	588	731
Pot Cap-1 Maneuver				-			010	0,0		728	645	
Stage 1							713	638		120	-	
Stage 2							710	000				
Platoon blocked, %							646	0	4	660	0	731
Mov Cap-1 Maneuver							646	0		660	0	
Mov Cap-2 Maneuver				-			040	0		728	0	
Stage 1				-	•	-	713	0		120	0	
Stage 2				•			713	0	3		U	
Approach				WB			NB			SB		
HCM Control Delay, s										10.2		
HCM LOS										В		
Minor Lane/Major Mvmt	NBLn1	WBL	WBT	WBR SBLn1								. 5
Capacity (veh/h)			-	- 731								
HCM Lane V/C Ratio				- 0.054								
HCM Control Delay (s)			-	- 10.2								
HCM Lane LOS				- B								
HCM 95th %tile Q(veh)	123			- 0.2								
HOM SOUL WILLS (ACIL)				0.2								

	*	-	*	1	←	*	4	†	1	-	ļ	1
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations				- 1111	4			4			1	
Volume (vph)	0	0	0	3	359	50	1	4	0	0	3	27
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	13	12	12	12	12	12	14	12
Satd. Flow (prot)	0	0	0	0	1709	0	0	1696	0	0	1543	0
Flt Permitted	ŭ							0.992				
Satd. Flow (perm)	0	0	0	0	1709	0	0	1696	0	0	1543	0
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		688			881			230			651	
Travel Time (s)		15.6			20.0			5.2			14.8	
Peak Hour Factor	0.86	0.86	0.86	0.86	0.86	0.86	0.86	0.86	0.86	0.86	0.86	0.86
Heavy Vehicles (%)	2%	2%	2%	0%	2%	0%	0%	0%	0%	0%	0%	4%
Shared Lane Traffic (%)	270	_,,	_,,									
Lane Group Flow (vph)	0	0	0	0	478	0	0	6	0	0	34	0
Sign Control	Ů	Free			Free			Stop			Stop	

Area Type:

CBD

Control Type: Unsignalized

Intersection Capacity Utilization 34.5%

Analysis Period (min) 15

Intersection												
Int Delay, s/veh	0.8											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SB
Vol, veh/h	0	0	0	3	359	50	1	4	0	0	3	2
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Sto
RT Channelized		-	None		-	None	-	-	None	-	-	Non
Storage Length			-		-	+		-	-		2	
Veh in Median Storage, #		0		1.0	0	- 1	- 2	0	-	2	0	
Grade, %	-	0			0	-		0	-		0	
Peak Hour Factor	86	86	86	86	86	86	86	86	86	86	86	8
Heavy Vehicles, %	2	2	2	0	2	0	0	0	0	0	0	
Mvmt Flow	0	0	0	3	417	58	1	5	0	0	3	3
3 34 7 3												
Major/Minor				Major2			Minor1		المت	Minor2		
Conflicting Flow All				0	0	0	471	483	0	455	453	44
Stage 1							0	0		453	453	
Stage 2							471	483		2	- 0	
Critical Hdwy				-	-		6.4	6.5		6.4	6.5	6.2
Critical Hdwy Stg 1								-	-	5.4	5.5	
Critical Hdwy Stg 2							5.4	5.5	-	-	-	
Follow-up Hdwy							3.5	4	1.5	3.5	. 4	3.33
Pot Cap-1 Maneuver				-	-		555	486		567	506	60
Stage 1							-	- 4	-	645	573	
Stage 2							632	556	-	-	-	
Platoon blocked, %												
Mov Cap-1 Maneuver					-	-	555	0		567	0	60
Mov Cap-2 Maneuver						+	555	0	-	567	0	
Stage 1					-	-	-	0	-	645	0	
Stage 2							632	0			0	
Approach				WB			NB			SB		
HCM Control Delay, s							1.19			11.3		
HCM LOS										В		
Minor Lane/Major Mvmt	NBLn1	WBL	WBT	WBR SBLn1								
Capacity (veh/h)	1100011			- 607								
HCM Lane V/C Ratio				- 0.057								
HCM Control Delay (s)				- 11.3								
HCM Lane LOS		, i		- B								
				- 0.2								
HCM 95th %tile Q(veh)	-	*	*	• 0.2								

	*	→	*	-	—	*	4	†	-	-	1	1
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations					4			र्स			1	
Volume (vph)	0	0	0	9	283	21	2	6	0	0	6	29
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	13	12	12	12	12	12	14	12
Satd. Flow (prot)	0	0	0	0	1718	0	0	1691	0	0	1618	0
Flt Permitted					0.999			0.989				
Satd. Flow (perm)	0	0	0	0	1718	0	0	1691	0	0	1618	0
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		688			881			230			651	
Travel Time (s)		15.6			20.0			5.2			14.8	
Peak Hour Factor	0.83	0.83	0.83	0.83	0.83	0.83	0.83	0.83	0.83	0.83	0.83	0.83
Heavy Vehicles (%)	2%	2%	2%	0%	2%	0%	0%	0%	0%	0%	0%	0%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	0	0	0	377	0	0	9	0	0	42	0
Sign Control		Free			Free			Stop			Stop	
11 11 0			200	15000000	1						- 79 00	

Area Type: CBD Control Type: Unsignalized

Intersection Capacity Utilization 28.5%

Analysis Period (min) 15

Intersection	157295.4.0			- Table	1	1 5 YV	S A VALUE	W.	V. 78 18			T P
Int Delay, s/veh	1											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Vol, veh/h	0	0	0	9	283	21	2	6	0	0	6	29
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	. 	77.	None			None
Storage Length					-		ita, ita				1	/*
Veh in Median Storage, #		0	-		0	54.5	-	0		*	0	
Grade, %		0			0		4	0	- a		0	
Peak Hour Factor	83	83	83	83	83	83	83	83	83	83	83	83
Heavy Vehicles, %	2	2	2	0	2	0	0	0	0	0	0	C
Mvmt Flow	0	0	0	11	341	25	2	7	0	0	7	35
Major/Minor	1600-510	U 72	1 719	Major2		Duran S	Minor1			Minor2	2015	
Conflicting Flow All				0	0	0	396	388	0	379	375	354
Stage 1				2-1 545		347	0	0	*	375	375	
Stage 2						248	396	388	14	4	0	
Critical Hdwy				1	2	- 12	6.4	6.5	- 2	6.4	6.5	6.2
Critical Hdwy Stg 1					-					5.4	5.5	
Critical Hdwy Stg 2						-	5.4	5.5	-	2		
Follow-up Hdwy							3.5	4		3.5	4	3.3
Pot Cap-1 Maneuver				1=			613	550		627	559	694
Stage 1										699	621	
Stage 2					-		684	612	-	-	-	
Platoon blocked, %						1 1						
Mov Cap-1 Maneuver						-	613	0	121	627	0	694
Mov Cap-1 Maneuver					- 2		613	0		627	0	
Stage 1				×			-	0		699	0	
Stage 2							684	0	-		0	
Annroach	MILLUL S	X	DK, SHE	WB		VELVILLE	NB		2	SB	75 (WITT.
Approach HCM Control Dolov s				110			110			10.5		
HCM Control Delay, s							_			В		
HCM LOS												
Minor Lane/Major Mvmt	NBLn1	WBL	WBT	WBR SBLn1	li pla i		Value Surg		911	Chi Carled	e Kyle	2-15
Capacity (veh/h)	*		3 * 3	- 694								
HCM Lane V/C Ratio			()	- 0.061								
HCM Control Delay (s)		K	09	⊸ 10.5								
HCM Lane LOS	2	72		- B								
HCM 95th %tile Q(veh)				0.2								

	*	→	7	•	←	*	4	†	-	1	Ţ	1
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		101011			4			र्स			7	
Volume (vph)	0	- 0	0	3	359	51	1	4	0	0	3	31
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	13	12	12	12	12	12	14	12
Satd. Flow (prot)	0	0	0	0	1707	0	0	1696	0	0	1539	0
Fit Permitted	:				11,50			0.992				
, ,	0	0	0	0	1707	0	0	1696	0	0	1539	0
Satd. Flow (perm)	Ų	30	· ·		30		_	30			30	
Link Speed (mph)		688			881			230			440	
Link Distance (ft)	: .	15.6			20.0			5.2			10.0	
Travel Time (s)		/	0.86	0.86	0.86	0.86	0.86	0.86	0.86	0.86	0.86	0.86
Peak Hour Factor	0.86	0.86				0.00	0.00	0%	0%	0%	0%	4%
Heavy Vehicles (%)	2%	2%	2%	0%	2%	U70	0 70	0 /0	0 /0	070		20.00
Shared Lane Traffic (%)					170	0	0		0	0	39	0
Lane Group Flow (vph)	0	0	0	0	479	0	0	6	0	U		U
Sign Control		Free			Free			Stop			Stop	
C. T. C. C. A. C. C. D. S. D. C. S. C.												

Area Type:

CBD

Control Type: Unsignalized

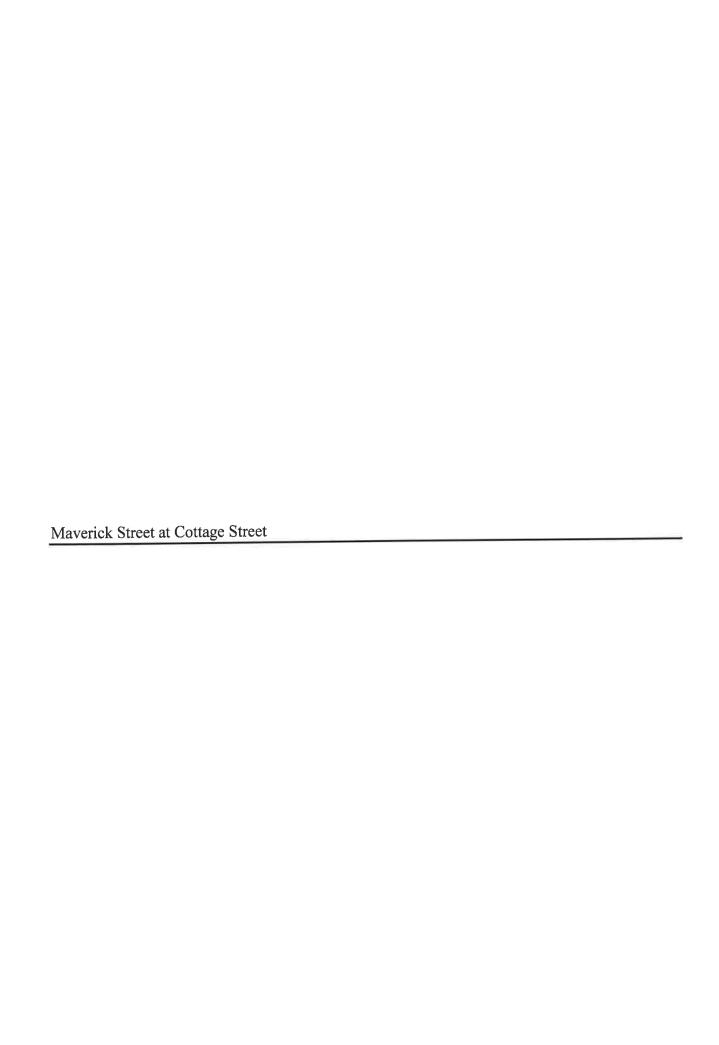
Intersection Capacity Utilization 34.6%

Analysis Period (min) 15

Intersection												
Int Delay, s/veh).9											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBF
Vol, veh/h	0	0	0	3	359	51	1	4	0	0	3	31
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	(
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized		-	None	1	1.	None	-	-	None	*		None
Storage Length			-		-	-	- 4					
Veh in Median Storage, #		0	-	-	0	-	-	0	4		0	
Grade, %		0	14		0			0	-	-	0	1 2
Peak Hour Factor	86	86	86	86		86	86	86	86	86	86	86
Heavy Vehicles, %	2	2	2	0		0	0	0	0	0	0	- 4
Mymt Flow	0	0	0	3		59	1	5	0	0	3	36
V €				Major2			Minor1			Minor2		
Major/Minor		-	-13-			0	474	484	0	456	454	447
Conflicting Flow All				0		-	0	0	-	454	454	777
Stage 1							474	484		2	0	
Stage 2							6.4	6.5		6.4	6.5	6.24
Critical Hdwy									-	5.4	5.5	0.2
Critical Hdwy Stg 1					10		- - 5 /	5.5	2.5		0,0	
Critical Hdwy Stg 2							5.4		-	2.5	4	3.336
Follow-up Hdwy				-			3.5	4		3.5	505	607
Pot Cap-1 Maneuver							553	486	-	566		
Stage 1							-	-		644	573	
Stage 2							630	555			-	
Platoon blocked, %										500	_	00
Mov Cap-1 Maneuver					- 4	1.4	553	0		566	0	
Mov Cap-2 Maneuver						-	553	0		566	0	
Stage 1						(+	-	0		644	0	
Stage 2						-	630	0			0	
Approach				WE			NB			SB		
HCM Control Delay, s										11.3		
HCM LOS							-			В		
MALE CONTRACTOR AND CONTRACTOR	NIDLed	10/101	WBT	WBR SBLn1								
Minor Lane/Major Mvmt	NBLn1	WBL	VVDI									
Capacity (veh/h)	-			- 607								
HCM Lane V/C Ratio	-			- 0.065								
HCM Control Delay (s)		7	-	- 11.3								
HCM Lane LOS				- E								
HCM 95th %tile Q(veh)	-	+		- 0.2								

	٠	→	*	•	←	*	4	†	-	-	↓	1
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations					4			सी			1	
Volume (vph)	0	0	0	9	283	24	2	6	0	0	6	32
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	13	12	12	12	12	12	14	12
Satd. Flow (prot)	0	0	0	0	1717	0	0	1691	0	0	1616	0
Flt Permitted					0.999			0.989				
Satd. Flow (perm)	0	0	0	0	1717	0	0	1691	0	0	1616	0
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		688			881			230			440	
Travel Time (s)		15.6			20.0			5.2			10.0	
Peak Hour Factor	0.83	0.83	0.83	0.83	0.83	0.83	0.83	0.83	0.83	0.83	0.83	0.83
Heavy Vehicles (%)	2%	2%	2%	0%	2%	0%	0%	0%	0%	0%	0%	0%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	. 0	0	0	0	381	0	0	9	0	0	46	0
Sign Control	, , , , , , , , , , , , , , , , , , , ,	Free			Free			Stop			Stop	
Intersection Summary	Life	H.S.W.	41211	SAM				La year	Total Indian	BELL	TA KING	- Strike
Area Type: Control Type: Unsignalized Intersection Capacity Utiliza Analysis Period (min) 15				Į(CU Level	of Service	eΑ					

NBL N	BR	NBL N	NBT	NBR	SBL	SBT	SBF
2	24	2	6	0	0	6	32
0	0	0	0	0	0	0	(
Stop S	ree	Stop S	Stop	Stop	Stop	Stop	Stop
	one			None	-	-	None
			-			-	
-	4	-	0	-	-	0	
			0	-		0	
83	83	83	83	83	83	83	83
0	0	0	0	0	0	0	0
2	29		7	0	0	7	39
Minor1	M	nor1	200		Minor2		
	0		392	0	381	377	355
0			0		377	377	
The state of the s		I I was a summer of	392	-	4	0	
			6.5		6.4	6.5	6.2
			-		5.4	5.5	-
			5.5		-	-	
3.5			4		3.5	4	3.3
			547		625	558	693
010			047		698	619	000
			610		050	010	
001		001	010	-			
040	•	640	^		COF	0	693
610			0	•	625		093
610	*		0		625	0	
-	141		0		698	0	
681		681	0		-	0	
NB		NB		- Y	SB		
					10.6		
(#					В		
3000							



	•	→	+	1	←	*	1	†	-	1	↓	1
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	- Style-Ut				4						1	
Volume (vph)	0	0	0	12	207	0	0	0	0	0	63	257
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	13	12	12	12	12	12	14	12
Satd. Flow (prot)	0	0	0	0	1713	0	0	0	0	0	1593	0
Fit Permitted					0.997							
Satd. Flow (perm)	0	0	0	0	1713	0	0	0	0	0	1593	0
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		881			297			136			587	
Travel Time (s)		20.0			6.8			3.1			13.3	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	2%	2%	2%	0%	3%	0%	2%	2%	2%	0%	2%	2%
Shared Lane Traffic (%)	2.10	£4 /0	270									
Lane Group Flow (vph)	. 0	: 0	0	0	238	0	0	0	0	0	347	C
Sign Control) V	Stop			Stop			Stop			Stop	

Area Type:

CBD

Control Type: Unsignalized Intersection Capacity Utilization 40.8%

Analysis Period (min) 15

Intersection												
Intersection Delay, s/veh	9.6											
Intersection LOS	A											
Movement	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBR
Vol., veh/h	0	0	0	0	0	12	207	0	0	0	0	(
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles, %	2	2	2	2	2	0	3	0	2	2	2	2
Mymt Flow	0	0	0	0	0	13	225	0	0	0	0	(
Number of Lanes	0	0	0	0	0	0	1	0	0	0	0	(
Approach	777					WB						
Opposing Approach												
Opposing Lanes						0						
Conflicting Approach Left												
Conflicting Lanes Left						0						
Conflicting Approach Right						SB						
Conflicting Lanes Right	A COMPT POLICE					1						
HCM Control Delay	7 119 M. 1 1 1 1					9.7						
HCM LOS						Α						
TO SERVICE AND SHAPE AREAS AND ADMINISTRATION OF THE PERSON OF THE PERSO												
Lane		WBLn1	SBLn1				6368					
Lane Vol Left, %		WBLn1 5%	0%				6.8					
Vol Left, %		NAME OF TAXABLE PARTY.					88					
Vol Left, % Vol Thru, %		5%	0%				E 2 E					
Vol Left, % Vol Thru, % Vol Right, %		5% 95%	0% 20%									
Vol Left, % Vol Thru, % Vol Right, % Sign Control		5% 95% 0%	0% 20% 80%									
Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane		5% 95% 0% Stop	0% 20% 80% Stop									
Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol		5% 95% 0% Stop 219	0% 20% 80% Stop 320									
Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol		5% 95% 0% Stop 219	0% 20% 80% Stop 320 0									
Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol		5% 95% 0% Stop 219 12 207 0	0% 20% 80% Stop 320 0									
Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate		5% 95% 0% Stop 219 12 207	0% 20% 80% Stop 320 0 63 257									
Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp		5% 95% 0% Stop 219 12 207 0 238	0% 20% 80% Stop 320 0 63 257 348									
Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X)		5% 95% 0% Stop 219 12 207 0 238	0% 20% 80% Stop 320 0 63 257 348									
Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd)		5% 95% 0% Stop 219 12 207 0 238 1 0.307	0% 20% 80% Stop 320 0 63 257 348 1 0.39									
Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N		5% 95% 0% Stop 219 12 207 0 238 1 0.307 4.646	0% 20% 80% Stop 320 0 63 257 348 1 0.39 4.037									
Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap		5% 95% 0% Stop 219 12 207 0 238 1 0.307 4.646 Yes 774	0% 20% 80% Stop 320 0 63 257 348 1 0.39 4.037 Yes									
Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time		5% 95% 0% Stop 219 12 207 0 238 1 0.307 4.646 Yes 774 2.678	0% 20% 80% Stop 320 0 63 257 348 1 0.39 4.037 Yes 894 2.053									
Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time HCM Lane V/C Ratio		5% 95% 0% Stop 219 12 207 0 238 1 0.307 4.646 Yes 774 2.678 0.307	0% 20% 80% Stop 320 0 63 257 348 1 0.39 4.037 Yes 894									
Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time		5% 95% 0% Stop 219 12 207 0 238 1 0.307 4.646 Yes 774 2.678	0% 20% 80% Stop 320 0 63 257 348 1 0.39 4.037 Yes 894 2.053 0.389									

1.9

HCM 95th-tile Q

ntersection Delay, s/veh					
ntersection LOS					
Titersection LOS					
Movement	SBU	SBL	SBT	SBR	
Vol, veh/h	0	0	63	257	
Peak Hour Factor	0.92	0.92	0.92	0.92	
Heavy Vehicles, %	2	0	2	2	
Mvmt Flow	0	0	68	279	
Number of Lanes	0	0	1	0	
Approach	The trans	5 1478 0	SB	i a	
Opposing Approach					
Opposing Lanes			0		
Conflicting Approach Left			WB		
Conflicting Lanes Left			1		
Conflicting Approach Right					
Conflicting Lanes Right			0		
HCM Control Delay			9.6		
HCM LOS			Α		

12: Maverick Street & Cottage Street

	<i>•</i>	-	*	•	←	*	4	†	-	-	ļ	1
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations					€Î						7>	
Volume (vph)	0	0	0	26	192	0	0	0	0	0	97	102
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	13	12	12	12	12	12	14	12
Satd. Flow (prot)	0	0	0	0	1726	0	0	0	0	0	1698	0
Flt Permitted					0.994							
Satd. Flow (perm)	0	0	0	0	1726	0	0	0	0	0	1698	0
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		881			297			136			587	
Travel Time (s)		20.0			6.8			3.1			13.3	
Peak Hour Factor	0.82	0.82	0.82	0.82	0.82	0.82	0.82	0.82	0.82	0.82	0.82	0.82
Heavy Vehicles (%)	2%	2%	2%	0%	2%	0%	2%	2%	2%	0%	0%	0%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	0	0	0	266	0	0	0	0	0	242	0
Sign Control		Stop			Stop			Stop			Stop	

Intersection Summary

Area Type:

CBD

Control Type: Unsignalized

Intersection Capacity Utilization 32.1%

Analysis Period (min) 15

ICU Level of Service A

Intersection Delay, s/veh	9.3 A											
Movement	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBF
Vol, veh/h	0	0	0	0	0	26	192	0	0	0	0	(
Peak Hour Factor	0.92	0.82	0.82	0.82	0.92	0.82	0.82	0.82	0.92	0.82	0.82	0.82
Heavy Vehicles, %	2	2	2	2	2	0	2	0	2	2	2	2
Mymt Flow	0	0	0	0	0	32	234	0	0	0	0	(
Number of Lanes	0	0	0	0	0	0	1	0	0	0	0	(
Approach					7	WB						
Opposing Approach												
Opposing Lanes						0						
Conflicting Approach Left												
Conflicting Lanes Left						0						
Conflicting Approach Right						SB						
Conflicting Lanes Right						1						
HCM Control Delay						9.7						
HCM LOS						Α						
Lane	-	WBLn1	SBLn1									
Vol Left, %		12%	0%									
Vol Left, % Vol Thru, %		12% 88%	0% 49%									
		12% 88% 0%	0% 49% 51%									
Vol Thru, %		12% 88% 0% Stop	0% 49% 51% Stop									
Vol Thru, % Vol Right, %		12% 88% 0% Stop 218	0% 49% 51% Stop 199									
Vol Thru, % Vol Right, % Sign Control		12% 88% 0% Stop 218 26	0% 49% 51% Stop 199									
Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane		12% 88% 0% Stop 218 26 192	0% 49% 51% Stop 199 0									
Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol		12% 88% 0% Stop 218 26 192 0	0% 49% 51% Stop 199 0 97									
Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol		12% 88% 0% Stop 218 26 192	0% 49% 51% Stop 199 0 97 102 243									
Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol		12% 88% 0% Stop 218 26 192 0 266	0% 49% 51% Stop 199 0 97 102 243									
Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate		12% 88% 0% Stop 218 26 192 0 266 1	0% 49% 51% Stop 199 0 97 102 243 1 0.285									
Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X)		12% 88% 0% Stop 218 26 192 0 266 1 0.329 4.461	0% 49% 51% Stop 199 0 97 102 243 1 0.285 4.222									
Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd)		12% 88% 0% Stop 218 26 192 0 266 1	0% 49% 51% Stop 199 0 97 102 243 1 0.285 4.222 Yes									
Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X)		12% 88% 0% Stop 218 26 192 0 266 1 0.329 4.461 Yes 806	0% 49% 51% Stop 199 0 97 102 243 1 0.285 4.222 Yes 854									
Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N		12% 88% 0% Stop 218 26 192 0 266 1 0.329 4.461 Yes	0% 49% 51% Stop 199 0 97 102 243 1 0.285 4.222 Yes 854 2.237									
Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap		12% 88% 0% Stop 218 26 192 0 266 1 0.329 4.461 Yes 806	0% 49% 51% Stop 199 0 97 102 243 1 0.285 4.222 Yes 854 2.237 0.285									
Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time		12% 88% 0% Stop 218 26 192 0 266 1 0.329 4.461 Yes 806 2.484	0% 49% 51% Stop 199 0 97 102 243 1 0.285 4.222 Yes 854 2.237									
Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time HCM Lane V/C Ratio		12% 88% 0% Stop 218 26 192 0 266 1 0.329 4.461 Yes 806 2.484 0.33	0% 49% 51% Stop 199 0 97 102 243 1 0.285 4.222 Yes 854 2.237 0.285									

Intersection Delay, s/veh					
Intersection LOS					
Movement	SBU	SBL	SBT	SBR	with each to we have a little with a same
Vol, veh/h	0	0	97	102	
Peak Hour Factor	0.92	0.82	0.82	0.82	
Heavy Vehicles, %	2	0	0	0	
Mvmt Flow	0	0	118	124	
Number of Lanes	0	0	1	0	
Approach	100	#550XI5	SB	JE WI	
Opposing Approach					
Opposing Lanes			0		
Conflicting Approach Left			WB		
Conflicting Lanes Left			1		
Conflicting Approach Right					
Conflicting Lanes Right			0		
HCM Control Delay			8.9		
HCM LOS			A		

	•	-	*	1	-	*	4	†	-	-		1
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations					4						1+	
Volume (vph)	0	0	0	13	231	0	0	0	0	0	67	277
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	13	12	12	12	12	12	14	12
Satd. Flow (prot)	0	0	0	0	1713	0	0	0	0	0	1593	0
Flt Permitted					0.997							
Satd. Flow (perm)	0	0	0	0	1713	0	0	0	0	0	1593	0
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		881			297			136			587	
Travel Time (s)		20.0			6.8			3.1			13.3	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	2%	2%	2%	0%	3%	0%	2%	2%	2%	0%	2%	2%
Shared Lane Traffic (%)			-									
Lane Group Flow (vph)	0	0	0	0	265	0	0	0	0	0	374	0
Sign Control		Stop			Stop			Stop			Stop	

Area Type: CBD

Control Type: Unsignalized

Intersection Capacity Utilization 43.9%

Analysis Period (min) 15

ICU Level of Service A

Intersection Delay, s/veh Intersection LOS	10.2 B											
Movement	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBR
Vol, veh/h	0	0	0	0	0	13	231	0	0	0	0	0
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles, %	2	2	2	2	2	0	3	0	2	2	2	2
Mymt Flow	0	0	0	0	0	14	251	0	0	0	0	0
Number of Lanes	0	0	0	0	0	0	1	0	0	0	0	0

Approach	WB	
Opposing Approach		
Opposing Lanes	0	
Conflicting Approach Left		
Conflicting Lanes Left	0	
Conflicting Approach Right	SB	
Conflicting Lanes Right	1	
HCM Control Delay	10.3	
HCM LOS	В	

Lane	WBLn1	SBLn1	
Vol Left, %	5%	0%	
Vol Thru, %	95%	19%	
Vol Right, %	0%	81%	
Sign Control	Stop	Stop	
Traffic Vol by Lane	244	344	
LT Vol	13	0	
Through Vol	231	67	
RT Vol	0	277	
Lane Flow Rate	265	374	
Geometry Grp	1	1	
Degree of Util (X)	0.347	0.427	
Departure Headway (Hd)	4.709	4.109	
Convergence, Y/N	Yes	Yes	
Сар	761	879	
Service Time	2.75	2.13	
HCM Lane V/C Ratio	0.348	0.425	
HCM Control Delay	10.3	10.2	
HCM Lane LOS	В	В	
HCM 95th-tile Q	1.6	2.2	

Intersection	The same	V 5 1 2			
Intersection Delay, s/veh					
Movement	SBU	SBL	SBT	SBR	
Vol, veh/h	0	0	67	277	
Peak Hour Factor	0.92	0.92	0.92	0.92	
Heavy Vehicles, %	2	0	2	2	
Mvmt Flow	0	0	73	301	
Number of Lanes	0	0	1	0	
Approach		BULBE	SB	ME SEE A	
Opposing Approach					
Opposing Lanes			0		
Conflicting Approach Left			WB		
Conflicting Lanes Left			1		
Conflicting Approach Right					
Conflicting Lanes Right			0		
HCM Control Delay			10.2		
HCM LOS			В		

Ideal Flow (vphpl) 1900 1900 1900 1900 Lane Width (ft) 12 12 12 12 Satd. Flow (prot) 0 0 0 Flt Permitted Satd. Flow (perm) 0 0 0 Link Speed (mph) 30 Link Distance (ft) 881	28 219	0 0 1900 3 12 6 0	0 1900 12 0	0 1900 12 0	0 1900 12 0	0 1900 12 0	\$BT 104 1900 14 1696	112 1900 12 0
Volume (vph) 0 0 0 2 Ideal Flow (vphpl) 1900 1900 1900 190 Lane Width (ft) 12 12 12 12 Satd. Flow (prot) 0 0 0 Flt Permitted Satd. Flow (perm) 0 0 0 Link Speed (mph) 30 Link Distance (ft) 881	28 219 00 1900 12 13 0 1726 0.994	0 1900 3 12 6 0	1900 12 0	1900 12 0	1900 12 0	1900 12 0	104 1900 14 1696	1900 12
Ideal Flow (vphpl) 1900 1900 1900 1900 Lane Width (ft) 12 12 12 12 Satd. Flow (prot) 0 0 0 Flt Permitted Satd. Flow (perm) 0 0 0 Link Speed (mph) 30 Link Distance (ft) 881	00 1900 12 13 0 1726 0.994	1900 3 12 6 0	1900 12 0	1900 12 0	1900 12 0	1900 12 0	1900 14 1696	1900 12
Lane Width (ft) 12 12 12 1 Satd. Flow (prot) 0 0 0 Flt Permitted Satd. Flow (perm) 0 0 0 Link Speed (mph) 30 30 30 Link Distance (ft) 881 881	0 1726 0 0.994	3 12 5 0	12 0	12 0	12 0	12 0	14 1696	12
Satd. Flow (prot) 0 0 0 Flt Permitted 0 0 0 Satd. Flow (perm) 0 0 0 Link Speed (mph) 30 30 Link Distance (ft) 881	0 1726	6 0 1	0	0	0	0	1696	
Fit Permitted Satd. Flow (perm) 0 0 0 Link Speed (mph) 30 Link Distance (ft) 881	0.994	1	7					0
Satd. Flow (perm) 0 0 0 Link Speed (mph) 30 30 Link Distance (ft) 881			0	0				
Link Speed (mph) 30 Link Distance (ft) 881	0 1726	3 0	٥	0	^	_		
Link Speed (mph) 30 Link Distance (ft) 881			U	0	0	0	1696	0
Link Distance (ft) 881	30)		30			30	
	297	7		136			587	
Travel Time (s) 20.0	6.8	3		3.1			13.3	
Peak Hour Factor 0.82 0.82 0.82 0.82	32 0.82	0.82	0.82	0.82	0.82	0.82	0.82	0.82
Heavy Vehicles (%) 2% 2% 2% 0	% 2%	0%	2%	2%	2%	0%	0%	0%
Shared Lane Traffic (%)								
Lane Group Flow (vph) 0 0 0	0 301	0	0	0	0	- 0	264	0
Sign Control Stop	Stop)		Stop			Stop	

Area Type:

CBD

Control Type: Unsignalized

Intersection Capacity Utilization 34.9%

ICU Level of Service A

Analysis Period (min) 15

Intersection	11-2											
Intersection Delay, s/veh	9.8											
Intersection LOS	Α											
Movement	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBF
Vol, veh/h	0	0	0	0	0	28	219	0	0	0	0	(
Peak Hour Factor	0.92	0.82	0.82	0.82	0.92	0.82	0.82	0.82	0.92	0.82	0.82	0.82
Heavy Vehicles, %	2	2	2	2	2	0	2	0	2	2	2	2
Mvmt Flow	0	0	0	0	0	34	267	0	0	0	0	(
Number of Lanes	0	0	0	0	0	0	1	0	0	0	0	(
Approach						WB		2.5				
Opposing Approach												
Opposing Lanes						0						
Conflicting Approach Left												
Conflicting Lanes Left						0						
Conflicting Approach Right						SB						
Conflicting Lanes Right						1						
HCM Control Delay						10.3						
HCM LOS						В						
Lane		WBLn1	SBLn1									
Vol Left, %		11%	0%									
Vol Thru, %		89%	48%									
Vol Right, %		0%	52%									
Sign Control		Stop	Stop									
Traffic Vol by Lane		247	216									
LT Vol		28	0									
Through Vol		219	104									
RT Vol		0	112									
Lane Flow Rate		301	263									
Geometry Grp		1	1									
Degree of Util (X)		0.378	0.315									
Departure Headway (Hd)		4.514	4.308									
Convergence, Y/N		Yes	Yes									
Cap		798	835									
Service Time		2.542	2.33									
HCM Lane V/C Ratio		0.377	0.315									
HCM Control Delay		10.3	9.3									
HCM Lane LOS		В	Α									

HCM 95th-tile Q

1.8

1.4

Intersection Delay, s/veh Intersection LOS					
Movement	SBU	SBL	SBT	SBR	
Vol, veh/h	0	0	104	112	
Peak Hour Factor	0.92	0.82	0.82	0.82	
Heavy Vehicles, %	2	0	0	0	
Mymt Flow	0	0	127	137	
Number of Lanes	0	0	1	0	
Approach	disorfin/	A PART OF	SB		
Opposing Approach					
Opposing Lanes			0		
Conflicting Approach Left			WB		
Conflicting Lanes Left			1		
Conflicting Approach Right					
Conflicting Lanes Right			0		
HCM Control Delay			9.3		
HCM LOS			A		

	×	→	*	•	←	*	4	†	1	>	↓	1
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations					4						7>	
Volume (vph)	0	0	0	13	232	0	0	0	0	0	67	277
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	13	12	12	12	12	12	14	12
Satd. Flow (prot)	0	0	0	0	1713	0	0	0	0	0	1593	0
Flt Permitted					0.997							
Satd. Flow (perm)	0	0	0	0	1713	0	0	0	0	0	1593	0
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		881			297			136			587	
Travel Time (s)		20.0			6.8			3.1			13.3	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	2%	2%	2%	0%	3%	0%	2%	2%	2%	0%	2%	2%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	0	0	0	266	0	0	0	0	0	374	0
Sign Control		Stop			Stop			Stop			Stop	
Intersection Summary		2 10/2				SAN, EU	31. 4		Y Program		1 8 20	
Area Type:	CBD											
Control Type: Unsignalized												
Intersection Capacity Utiliza	tion 43.9%			10	CU Level	of Service	A					

Analysis Period (min) 15

Intersection Delay, s/veh Intersection LOS	10.2 B											
Movement	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBR
Vol. veh/h	0	0	0	0	0	13	232	0	0	0	0	0
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles, %	2	2	2	2	2	0	3	0	2	2	2	2
Mymt Flow	0	0	0	0	0	14	252	0	0	0	0	0
Number of Lanes	0	0	0	0	0	0	1	0	0	0	0	0

Approach	WB	
Opposing Approach		
Opposing Lanes		
Conflicting Approach Left		
Conflicting Lanes Left		
Conflicting Approach Right	SB	
Conflicting Lanes Right	The state of the s	
HCM Control Delay	10.3	
HCM LOS	В	

Lane	WBLn1	SBLn1	
Vol Left, %	5%	0%	
Vol Thru, %	95%	19%	
Vol Right, %	0%	81%	
Sign Control	Stop	Stop	
Traffic Vol by Lane	245	344	
LT Vol	13	0	
Through Vol	232	67	
RT Vol	0	277	
Lane Flow Rate	266	374	
Geometry Grp	1	1	
Degree of Util (X)	0.348	0.427	
Departure Headway (Hd)	4.711	4.113	
Convergence, Y/N	Yes	Yes	
Сар	762	879	
Service Time	2.75	2.132	
HCM Lane V/C Ratio	0.349	0.425	
HCM Control Delay	10.3	10.2	
HCM Lane LOS	В	В	
HCM 95th-tile Q	1.6	2.2	

Intersection Delay, s/veh					
Intersection LOS					
intersection LOS					
Movement	SBU	SBL	SBT	SBR	
Vol, veh/h	0	0	67	277	
Peak Hour Factor	0.92	0.92	0.92	0.92	
Heavy Vehicles, %	2	0	2	2	
Mvmt Flow	0	0	73	301	
Number of Lanes	0	0	1	0	
Approach	WEE H		SB	S PARTE	
Opposing Approach					
Opposing Lanes			0		
Conflicting Approach Left			WB		
Conflicting Lanes Left			1		
Conflicting Approach Right					
Conflicting Lanes Right			0		
HCM Control Delay			10.2		
HCM LOS			В		

,	۶	→	*	-	←	4	4	†	1	-	1	1
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations					सी						f >	
Volume (vph)	0	0	0	28	222	0	0	0	0	0	104	112
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	13	12	12	12	12	12	14	12
Satd. Flow (prot)	0	0	0	0	1726	0	0	0	0	0	1696	0
Flt Permitted					0.994							
Satd. Flow (perm)	0	0	0	0	1726	0	0	0	0	0	1696	0
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		881			297			136			587	
Travel Time (s)		20.0			6.8			3.1			13.3	
Peak Hour Factor	0.82	0.82	0.82	0.82	0.82	0.82	0.82	0.82	0.82	0.82	0.82	0.82
Heavy Vehicles (%)	2%	2%	2%	0%	2%	0%	2%	2%	2%	0%	0%	0%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	. 0	0	0	305	0	0	0	0	0	264	0
Sign Control		Stop			Stop			Stop			Stop	

Area Type:

CBD

Control Type: Unsignalized

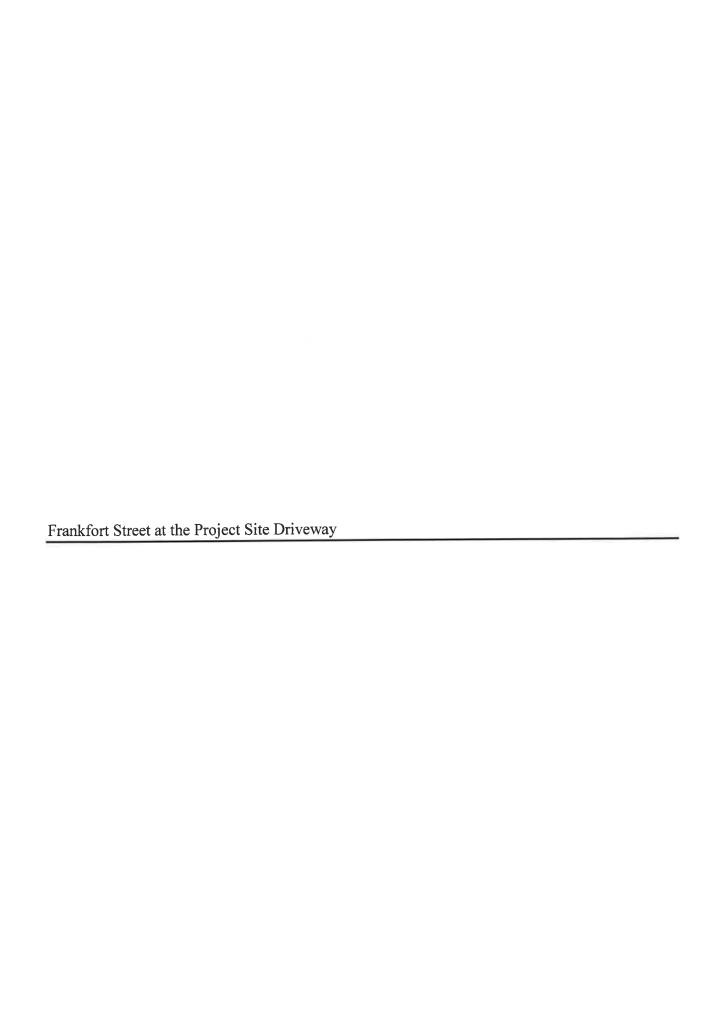
Intersection Capacity Utilization 35.1%

Analysis Period (min) 15

ICU Level of Service A

Intersection								E-910				
Intersection Delay, s/veh Intersection LOS	9,8 A											
Movement	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBF
Vol, veh/h	0	0	0	0	0	28	222	0	0	0	0	(
Peak Hour Factor	0.92	0.82	0.82	0.82	0.92	0.82	0.82	0.82	0.92	0.82	0.82	0.83
Heavy Vehicles, %	2	2	2	2	2	0	2	0	2	2	2	1
Mvmt Flow	0	0	0	0	0	34	271	0	0	0	0	(
Number of Lanes	0	0	0	0	0	0	1	0	0	0	0	(
Approach	- 10		301			WB				34.		
Opposing Approach												
Opposing Lanes						0						
Conflicting Approach Left						0						
Conflicting Lanes Left Conflicting Approach Right						SB						
Conflicting Lanes Right						1						
HCM Control Delay						10.3						
I IOIVI OUITII UI DEIAV												
HCM LOS						В						
						В						
HCM LOS	(WBLn1	SBLn1			В		S = F				
Lane Vol Left, %	l	11%	0%			B):		E =1°				
Lane Vol Left, % Vol Thru, %	(11% 89%	0% 48%			В						
Lane Vol Left, % Vol Thru, % Vol Right, %	V	11% 89% 0%	0% 48% 52%			В						
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control	(11% 89% 0% Stop	0% 48% 52% Stop			В						
HCM LOS Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane	(11% 89% 0% Stop 250	0% 48% 52% Stop 216			В		O HI				
Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control	(11% 89% 0% Stop 250 28	0% 48% 52% Stop 216 0			В						
HCM LOS Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol	\	11% 89% 0% Stop 250 28 222	0% 48% 52% Stop 216 0 104			В		e e ri				
HCM LOS Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol	V	11% 89% 0% Stop 250 28 222 0	0% 48% 52% Stop 216 0 104 112			В						
HCM LOS Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate	V	11% 89% 0% Stop 250 28 222 0 305	0% 48% 52% Stop 216 0 104 112 263			В		e e n				
HCM LOS Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol	V	11% 89% 0% Stop 250 28 222 0 305	0% 48% 52% Stop 216 0 104 112 263			В		e e l'i				
HCM LOS Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X)		11% 89% 0% Stop 250 28 222 0 305 1 0.382	0% 48% 52% Stop 216 0 104 112 263 1 0.316			В						
HCM LOS Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp		11% 89% 0% Stop 250 28 222 0 305 1 0.382 4.514	0% 48% 52% Stop 216 0 104 112 263 1 0.316 4.316			В						
HCM LOS Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X)	V	11% 89% 0% Stop 250 28 222 0 305 1 0.382 4.514 Yes	0% 48% 52% Stop 216 0 104 112 263 1 0.316 4.316 Yes			В						
HCM LOS Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd)	\	11% 89% 0% Stop 250 28 222 0 305 1 0.382 4.514 Yes 796	0% 48% 52% Stop 216 0 104 112 263 1 0.316 4.316 Yes 835			В						
HCM LOS Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N	\	11% 89% 0% Stop 250 28 222 0 305 1 0.382 4.514 Yes 796 2.544	0% 48% 52% Stop 216 0 104 112 263 1 0.316 4.316 Yes 835 2.338			В		e e l'i				
HCM LOS Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap		11% 89% 0% Stop 250 28 222 0 305 1 0.382 4.514 Yes 796 2.544 0.383	0% 48% 52% Stop 216 0 104 112 263 1 0.316 4.316 Yes 835 2.338 0.315			В						
HCM LOS Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time		11% 89% 0% Stop 250 28 222 0 305 1 0.382 4.514 Yes 796 2.544	0% 48% 52% Stop 216 0 104 112 263 1 0.316 4.316 Yes 835 2.338			В						
HCM LOS Lane Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time HCM Lane V/C Ratio		11% 89% 0% Stop 250 28 222 0 305 1 0.382 4.514 Yes 796 2.544 0.383	0% 48% 52% Stop 216 0 104 112 263 1 0.316 4.316 Yes 835 2.338 0.315			В						

Intersection Delay alvoh					
Intersection Delay, s/veh Intersection LOS					
Movement	SBU	SBL	SBT	SBR	
Vol, veh/h	0	0	104	112	
Peak Hour Factor	0.92	0.82	0.82	0.82	
Heavy Vehicles, %	2	0	0	0	
Mvmt Flow	0	0	127	137	
Number of Lanes	0	0	1	0	
Approach	Party.	(II)(= '	SB		Tales Transport of the state of
Opposing Approach					
Opposing Lanes			0		
Conflicting Approach Left			WB		
Conflicting Lanes Left			1		
Conflicting Approach Right					
Conflicting Lanes Right			0		
HCM Control Delay			9.3		
HCM LOS			A		



	1	*	†	-	1	↓	
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT	
Lane Configurations	- NA		12			र्स	
Volume (vph)	4	8	59	1	4	25	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Satd. Flow (prot)	1498	0	1673	0	0	1666	
Flt Permitted	0.985					0.994	
Satd. Flow (perm)	1498	0	1673	0	0	1666	
Link Speed (mph)	30		30			30	
Link Distance (ft)	88		440			211	
Travel Time (s)	2.0		10.0			4.8	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	13	0	65	0	0	31	
Sign Control	Stop		Free			Free	
Information Commons			100				

Area Type:

CBD

Control Type: Unsignalized

Intersection Capacity Utilization 15.2%

ICU Level of Service A

Analysis Period (min) 15

Intersection								
Int Delay, s/veh 1	.3							
Movement	WBL	WBR		NBT	NBR	SBL	SBT	ne .
Vol, veh/h	4	8		59	1	4	25	
Conflicting Peds, #/hr	0	0		0	0	0	0	
Sign Control	Stop	Stop		Free	Free	Free	Free	
RT Channelized	-	None	,	_	None	2.	None	
Storage Length	0			- 2				
Veh in Median Storage, #	0			0			0	
Grade, %	0	4		0	-	-	0	
Peak Hour Factor	92	92		92	92	92	92	
Heavy Vehicles, %	2	2		2	2	2	2	
Mymt Flow	4	9		64	1	4	27	
MAINT LIOM	-4	3		04				
Major/Minor	Minor1			Major1	The state of the s	Major2		جالا جمهام
Conflicting Flow All	101	65		0	0	65	0	
Stage 1	65	=		-	-	-	-	
Stage 2	36							
Critical Hdwy	6.42	6.22		-	-	4.12		
Critical Hdwy Stg 1	5.42					-		
Critical Hdwy Stg 2	5.42			-	4		-	
Follow-up Hdwy	3.518	3.318				2.218	-	
Pot Cap-1 Maneuver	898	999				1537	-	
Stage 1	958	000						
	986			1			- 4	
Stage 2	900	7						
Platoon blocked, %	005	999				1537	1	
Mov Cap-1 Maneuver	895	999		-	-	1007		
Mov Cap-2 Maneuver	895	-				-	-	
Stage 1	958			- 7			-	
Stage 2	983	-				-		
Approach	WB			NB	0.536	SB		
HCM Control Delay, s	8.8			0		1		
HCM LOS	Α							
HCM FO2	A							
Minor Lane/Major Mvmt	NBT	NBRWBLn1	SBL	SBT				
Capacity (veh/h)	-	- 962	1537	4.				
HCM Lane V/C Ratio			0.003	_				
HCM Control Delay (s)		- 8.8	7.3	0				
		- A	Α.	A				
HCM Lane LOS		- 0	0	^				
HCM 95th %tile Q(veh)		- 0	U	-				

	•	*	†	1	-	↓	
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT	
Lane Configurations	1/4		4			4	
Volume (vph)	3	7	26	3	10	39	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Satd. Flow (prot)	1658	0	1839	0	0	1844	
Flt Permitted	0.987					0.990	
Satd. Flow (perm)	1658	0	1839	0	0	1844	
Link Speed (mph)	30		30			30	
Link Distance (ft)	123		440			211	
Travel Time (s)	2.8		10.0			4.8	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	11	0	31	0	0	53	
Sign Control	Stop		Free			Free	
Intersection Summary		WE TO			W - 30		
	Other						
Control Type: Unsignalized		1					
Intersection Capacity Utilizat				IC	U Level	of Service A	
Analysis Period (min) 15							

Intersection Int Delay, s/veh 1.5	Q								
	U								
Movement	WBL	WBR		NBT	NE	BR .	SBL	SBT	
Vol, veh/h	3	7		26		3	10	39	
Conflicting Peds, #/hr	0	0		0		0	0	0	
Sign Control	Stop	Stop		Free	Fr	ee	Free	Free	
RT Channelized	-	None			No	ne	-	None	
Storage Length	0					-	-	-	
Veh in Median Storage, #	0	Ų.		()			0	
Grade, %	0			()	-		0	
Peak Hour Factor	92	92		92		92	92	92	
Heavy Vehicles, %	2	2		2		2	2	2	
Mvmt Flow	3	8		28	}	3	11	42	
Major/Minor	Minor1	-		Major1			Major2		
Conflicting Flow All	94	30		(Major		0	32	0	
Stage 1	30	00				-	-	-	
	64							-	
Stage 2 Critical Hdwy	6.42	6.22					4.12	1	
	5.42	0.22				_	1.1.		
Critical Hdwy Stg 1	5.42						4	-	
Critical Hdwy Stg 2	3.518	3.318				2	2.218		
Follow-up Hdwy		1044					1580		
Pot Cap-1 Maneuver	906	1044					1000		
Stage 1	993	-							
Stage 2	959					-	-	-	
Platoon blocked, %	000	4044					1580	-	
Mov Cap-1 Maneuver	900	1044			4	9	1500	- 1	
Mov Cap-2 Maneuver	900					7		-	
Stage 1	993					-	7	~	
Stage 2	952	-				-		-	
Approach	WB			NE	}		SB		
HCM Control Delay, s	8.7			()		1.5		
HCM LOS	Α								
Minor Lane/Major Mvmt	NBT	NBRWBLn1	SBL	SBT					
Capacity (veh/h)	19	- 996	1580	-					
HCM Lane V/C Ratio	-	- 0.011		-					
HCM Control Delay (s)		- 8.7	7.3	0					
HCM Lane LOS	*	- A	Α	A					
HCM 95th %tile Q(veh)	-	- 0	0	-					



	٦	*	4	†		1	
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR	
Lane Configurations	W	10/10/02		4	₽		
Volume (vph)	1	0	0	8	50	0	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Satd. Flow (prot)	1593	0	0	1676	1676	0	
Flt Permitted	0.950						
Satd. Flow (perm)	1593	0	0	1676	1676	0	
Link Speed (mph)	30			30	30		
Link Distance (ft)	87			175	304		
Travel Time (s)	2.0			4.0	6.9		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	1.	0	0	9	54	0	
Sign Control	Stop			Free	Free		
Intersection Summary	S. 185. A		Angrand	191 . BY		4,513	
Area Type:	CBD						
Control Type: Unsignalize	ed						
Intersection Capacity Util Analysis Period (min) 15				IC	CU Level o	of Service	e A

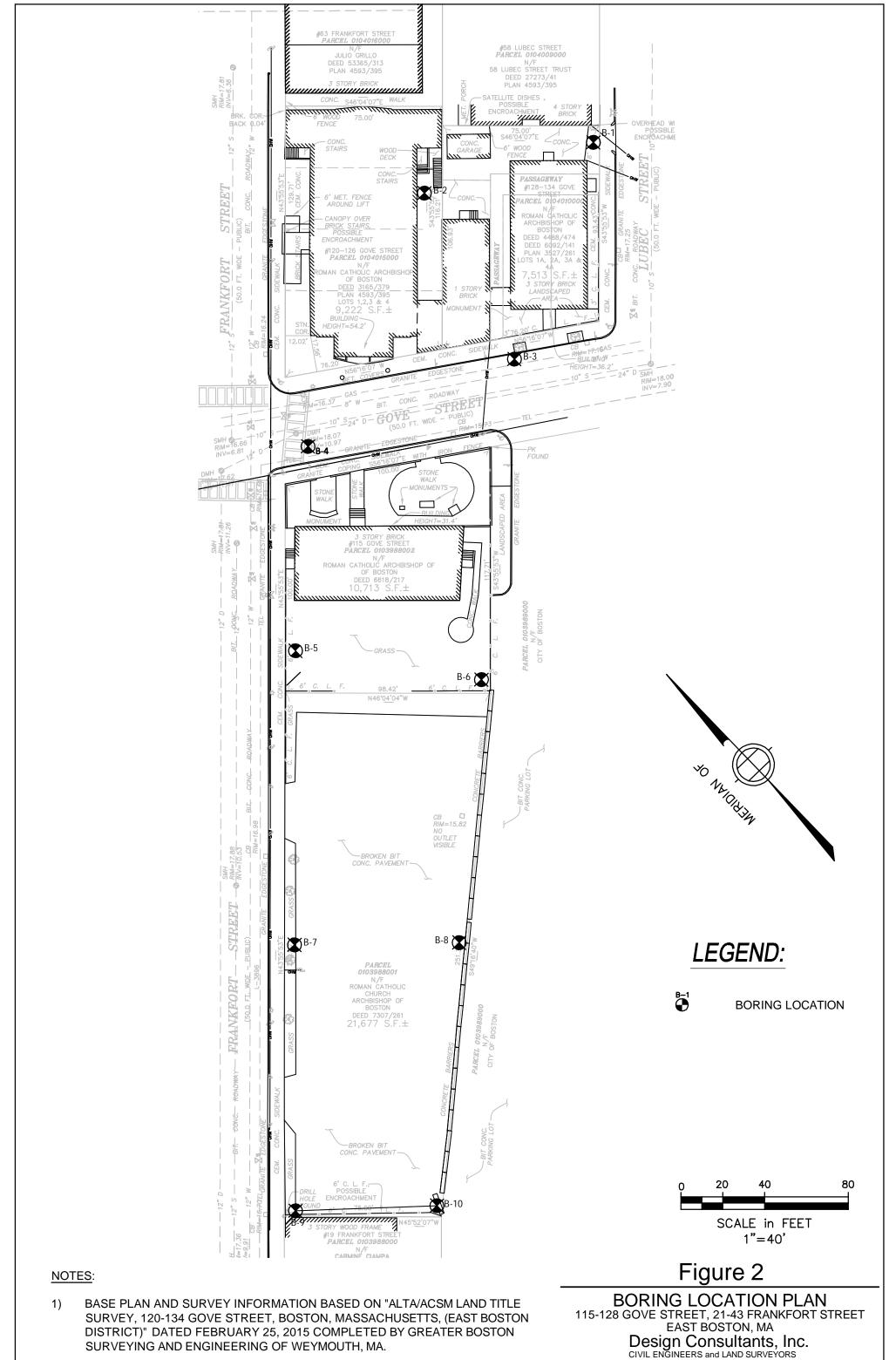
Intersection	139	AND RESERVE			The state of the state of	W. F.		
Int Delay, s/veh 0	.1	5.5						
Movement	EBL	EBR	NBL	NBT	SBT	SBR		W.S.
Vol, veh/h	1	0	0		50	0		
Conflicting Peds, #/hr	0	0	0		0	0		
Sign Control	Stop	Stop	Free	Free	Free	Free		
RT Channelized	-	None		None	-	None		
Storage Length	0	110110						
/eh in Median Storage, #	0			0	0	-		
Grade, %	0	The Par	1 7 7 4	0	0	120		
Peak Hour Factor	92	92	92		92	92		
Heavy Vehicles, %	2	2	2		2	2		
Mvmt Flow	1	0	0		54	0		
MALIIT LIOM	In the	0						
Major/Minor	Minor2		Major1		Major2			los.
Conflicting Flow All	63	54	54	0		0		11
Stage 1	54			-				
Stage 2	9	S#3	ST III	-				
Critical Hdwy	6.42	6.22	4.12	141		₹#		
Critical Hdwy Stg 1	5.42	1. 1 1 12						
Critical Hdwy Stg 2	5.42		-	-	-	-		
Follow-up Hdwy	3.518	3.318	2.218			- 15.0		
Pot Cap-1 Maneuver	943	1013	1551					
	969	1013	1001					
Stage 1	1014							
Stage 2	1014	**				- I		
Platoon blocked, %	0.40	1012	1551					
Mov Cap-1 Maneuver	943	1013	1001					
Mov Cap-2 Maneuver	943							
Stage 1	969					:=		
Stage 2	1014	:*:		*	5	3		
Approach	EB	printer to the	NB	JUST GIT	SB	BONE S	1 8 W 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	ij,
HCM Control Delay, s	8.8		0		0			
HCM LOS	A		· · · · · · · · · · · · · · · · · · ·					
10111 200	,,							
Minor Lane/Major Mvmt	NBL	NBT EBLn1	SBT SBR	5 118		100		P) X
Capacity (veh/h)	1551	- 943	(2) (2)					
HCM Lane V/C Ratio	-	0.001						
HCM Control Delay (s)	0	8.8	950 05					
HCM Lane LOS	A	Α	:e ::e					
HCM 95th %tile Q(veh)	0	0						

	→	*	1	†	↓	1	
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR	
Lane Configurations	N/			र्भ	4		
Volume (vph)	1	0	0	3	26	1	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Satd. Flow (prot)	1770	0	0	1863	1853	0	
Flt Permitted	0.950						
Satd. Flow (perm)	1770	0	0	1863	1853	0	
Link Speed (mph)	30			30	30		
Link Distance (ft)	122			194	215		
Travel Time (s)	2.8			4.4	4.9		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	1	0	0	3	29	0	
Sign Control	Stop			Free	Free		
Intersection Summary	DE LEGITAL	A 2/2 to	13.51	1361		-81145¥	
Area Type:	Other						
Control Type: Unsignalized	i						
Intersection Capacity Utiliz				IC	U Level o	of Service	e A
Analysis Period (min) 15							

Intersection				7 77 1	ALTERNATION AND A	G LY L	
Int Delay, s/veh	0.3		S0 (=V)				
Movement	EBL	EBR	NBL	NBT	SBT	SBR	
Vol, veh/h	1	0	0	3	26	1	
Conflicting Peds, #/hr	0	0	0	0	0	0	
Sign Control	Stop	Stop	Free	Free	Free	Free	
RT Channelized	- A	None	-	None		None	
Storage Length	0		A LILON .				
/eh in Median Storage, #	0	*	(=)	0	0	(#1)	
Grade, %	0			0	0	- X	
Peak Hour Factor	92	92	92	92	92	92	
Heavy Vehicles, %	2	2	2	2	2	2	
Nymt Flow	ī	ō	ō	3	28	1	
1			14-14	The state of the s	MARIE - 0		
Major/Minor	Minor2		Major1		Major2		
Conflicting Flow All	32	29	29	0	MANAGERIA (C. 16)	0	
Stage 1	29		#R				
Stage 2	3					7.8	
Critical Hdwy	6.42	6.22	4.12	(=)		*	
Critical Hdwy Stg 1	5.42			-		*	
Critical Hdwy Stg 2	5.42	:20	:="	940	2	:2	
ollow-up Hdwy	3.518	3.318	2.218				
ot Cap-1 Maneuver	982	1046	1584		ē		
Stage 1	994			-			
Stage 2	1020	1 8 7		7-7			
Platoon blocked, %							
Nov Cap-1 Maneuver	982	1046	1584				
Nov Cap-2 Maneuver	982	1010		4	KSATTO -		
Stage 1	994		-	-		2	
Stage 2	1020				1181 8	3	
pproach	EB		NB		SB		
ICM Control Delay, s	8.7		0		0		
HCM LOS	Α						
Minor Lane/Major Mvmt	NBL	NBT EBLn1	SBT SBR		HORNOGER (VIII)		
	1584	- 982					
Capacity (veh/h)	1004						
ICM Cantral Dalay (a)	_	- 0.001					
HCM Control Delay (s)	0	8.7	* *				
ICM Lane LOS	A	- A					
HCM 95th %tile Q(veh)	0	- 0	* *				

Attachment C

Geotechnical Information



SOMERVILLE - NEWBURYPORT - QUINCY

P:\2017 Projects\2017-032 120 Gove St E.Boston\Soils Info\Boring Location Plan.dwg

				BORING LOG			
Project: Location: Client: Driller: Drilling Met Weather: Performed I	CARR-DEE (hods: RAINY, 50's By: PJS	ON, MA LTING, INC. CORPORATIO HSA/D&W 1 S Date:		DESIGN CONSULTANTS, INC.	Boring No: I Location: See F Approx. Ground Elevation: Approx. Groundwater Elevation: Date/Time of Groundwater Elevation: 7:30 Datum: BOSTON CITY B Project No. 2017-		
Depth (feet)	Sample No.	Blows per 6-inch	Pen./ Rec.	Soil Description	Stratum Change Depth	Stratum	Note No.
					(feet)	001100575	(4)
1 1 2	S-1	2 38 16 5	24"/6"	S-1, SAND, Some Silt, Little Gravel, Little Glass, Trace Coal, Ash, Brick, Black, Dry, Very Dense	.3'	CONCRETE	(1)
3 4 5 6 7 8	S-2	3 4 4 3	24"/1"	S-2, SAND, Some Silt, Little Gravel, Little Glass, Trace Coal, Ash, Brick, Black, Dry, Loose	8.5'	FILL	
9 10 11 12 12 13 13 15 16 17 17 17 17 17 17 17 17 17 17 17 17 17	S-3	1/12" 1 2 1 2 3 2	24"/12"	S-3, CLAY, Trace Sand, Trace Gravel, Grey, Wet, Soft S-4, Similar to S-3, No Gravel, Medium Stiff		CLAY	(2)
- 14 - 15 - 16 - 17 - 18 - 19	S-5	1 2 3 5 7 4 4 5	24"/24"	S-5, Top 12" Similar to S-4, Trace Seashells, Next 11", SILT, Some Organics, Little Sand, Grey, Wet, Medium Stiff, Bottom 1", PEAT, Brown, Wet, Soft S-6, SILT, Some Clay, Little Sand, Grey, Wet, Medium Stiff to Stiff	15' 16' 17' 17.2'	ORGANIC CLAY W/ SEASHELLS ORGANIC SILT PEAT CLAYEY SILT	
20	S-7	3-6 5-14	24"/24"	S-7, Top 1" Gravel, Next 3" PEAT Bottom 20" See Page 2	19.3'	PEAT CLAYEY SILT	
<u>NOTES:</u> (1) 3-INCHE	S OF CONCRI		JRFACE	LEGEND S - Split Spoon Sample UT - Undisturbed Tube Sample Trace - Approximately 0 to 10% Little - Approximately 10 to 20% 0-10 Coarse Soil N Value - Loose 10-30 Coarse Soil N Value - Medium Dense 0-4 Fine Soil N Value - Soft 4-8 Fine Soil N Value - Medium Stiff	Some - Approxim And - Approxim 30-50 Coarse So >50 Coarse Soil 8-15 Fine Soil N	iollected Off the Augers imately 20 to 35% nately 35 to 50% oil N Value - Dense	il N Value - Hard

				BORING LOG			
Project: Location: Client: Driller: Drilling Meth Weather: Performed B Checked By:	CARR-DEE nods: RAINY, 50's	ON, MA ILTING, INC. CORPORATIO HSA/D&W 1! s Date:		DESIGN CONSULTANTS, INC.	Approx. Gro		B-1 See Plan 18± 9± 7:30 AM ON CITY BASE 2017-032
Depth (feet)	Sample No.	Blows per 6-inch	Pen./ Rec.	Soil Description	Stratum Change Depth	Stratum	Note No.
	S-7	3-6 5-14	24"/24"	S-7, Top 1" Gravel, Next 3" PEAT, Bottom 20" SILT, Some Clay, Little Sand, Grey, Wet, Stiff	(feet)		
				C.O. Circillon de Dette ve ef C.7		CLAYEY SILT	
26 27	S-8	3 5 7 8	24"/18"	S-8, Similar to Bottom of S-7	27'		
	S-9	9 8 7 7	24"/24"	S-9, CLAY, Little Sand, Grey, Wet, Very Stiff	29'	CLAY	
30				BOTTOM OF BORING AT 29' BGS			
<u>NOTES:</u>				LEGEND S - Split Spoon Sample UT - Undisturbed Tube Sample Trace - Approximately 0 to 10% Little - Approximately 10 to 20% 0-10 Coarse Soil N Value - Loose 10-30 Coarse Soil N Value - Medium Dense 0-4 Fine Soil N Value - Soft 4-8 Fine Soil N Value - Medium Stiff	Some - Approxima And - Approxima 30-50 Coarse Soil >50 Coarse Soil N 8-15 Fine Soil N N	tely 35 to 50% I N Value - Dense I Value - Very Dense	Soil N Value - Hard

				BORING LOG			
Project: Location: Client: Driller: Drilling Me: Weather: Performed Checked By	CARR-DEE (thods: RAINY, 40's By: PJS	ON, MA ULTING, INC. CORPORATIC HSA/D&W 1 S Date:		DESIGN CONSULTANTS, INC.	Boring No: B-Location: See Pla Approx. Ground Elevation: 18 Approx. Groundwater Elevation: 11:00 Al Datum: BOSTON CITY BAS Project No. 2017-03		
Depth (feet)	Sample No.	Blows per 6-inch	Pen./ Rec.	Soil Description	Stratum Change Depth (feet)	Stratum	Note No.
_ 1 _ 2	S-1	2 3 3 4	24"/13"	S-1, SAND And Silt, Little Gravel, Trace Coal, Ash, Black, Dry, Loose	.3'	CONCRETE	(1)
3 - 4 - 5 - 6	S-2	1 2 1	24"/13"	S-2, Top 9" Similar to S-1, Little Asphalt Bottom 4", SILT, Little Clay, Trace Sand, Olive, Moist, Soft	6.5'	FILL	
- 7 - 8 - 9 - 10 - 11	S-3	1 1 2 2	24"/19"	S-3, SILT, Little Clay, Little Silt, Trace Gravel, Olive-Grey, Wet, Soft - Med. Stiff 1" SAND and Gravel seam at 15"		CLAYEY SILT	(2)
- 12 - 13 - 14 - 15		2			14'		
— 15 — 16 — 17	S-4	1 2 1 2	24"/12"	S-4, SILT, Little Clay, Little Sand, Trace Seashells, Trace Organics, Grey, Wet Soft		ORGANIC SILT W/ SEASHELLS	
18 19	S-5	3 6 4 4	24"/24"	S-5, Top 16" Similar to S-4, Next 4" PEAT, Brown, Wet, Medium Stiff Bottom 4" SILT, Little Clay, Little Sand, Grey, Wet, Medium Stiff	18' 18.5'	PEAT CLAYEY SILT	
20 <u>NOTES:</u> (1) 3" OF CC (2) WATER		V GROUND SI	JRFACE	LEGEND S - Split Spoon Sample UT - Undisturbed Tube Sample Trace - Approximately 0 to 10% Little - Approximately 10 to 20% 0-10 Coarse Soil N Value - Loose 10-30 Coarse Soil N Value - Medium Dense 0-4 Fine Soil N Value - Medium Stiff	Some - Approxim And - Approxim 30-50 Coarse So >50 Coarse Soil 8-15 Fine Soil N	imately 20 to 35% nately 35 to 50% oil N Value - Dense I N Value - Stiff >30 Fine S N Value - Very Stiff	oil N Value - Hard

BORING LOG

Project: 120 GOVE STREET Location: EAST BOSTON, MA Client: EBI CONSULTING, INC. Driller: CARR-DEE CORPORATION

Weather: RAINY, 40's

Drilling Methods:

Performed By: PJS Date: 4/21/17

PGC Date: 4/26/17 Checked By:

HSA/D&W 10'+



Boring No: B-2

Location: See Plan

Approx. Ground Elevation: 18± Approx. Groundwater Elevation: 8±

Date/Time of Groundwater Elevation: 11:00 AM

Datum: **BOSTON CITY BASE**

Project No. 2017-032

	Depth (feet)	Sample No.	Blows per 6-inch	Pen./ Rec.	Soil Description	Stratum Change Depth (feet)	Stratum	Note No.
F			5 9		S-6, SILT, Some Clay, Some Fine Sand, Olive & Grey, Wet, Very Stiff			
F	21	S-6	15 18	24"/24"	cine a crog, war, vary ann			
L	22		10					
H	23							
F	24							
F	25		6		S-7, SILT, Some Clay, Little Fine Sand,			
E	26	S-7	6	24"/22"	Grey, Wet, Stiff		CLAYEY SILT	
E	27		6 8					
	28							
H	29							
F								
F	30		3 4		S-8, Similar to S-7			
L	31	S-8	5	24"/24"		221		
Ł	32		7		BOTTOM OF BORING AT 32' BGS	32'		
H	33							
F	34							
F	35							
F	36							
L	37							
L	38							
\vdash								
F	39 40							
	NOTES:	I	<u> </u>	I	<u>LEGEND</u>			
					S - Split Spoon Sample	O/A - Sample Co	ollected Off the Augers	

10-30 Coarse Soil N Value - Medium Dense >50 Coarse Soil N Value - Very Dense 0-4 Fine Soil N Value - Soft 8-15 Fine Soil N Value - Stiff >30 Fine Soil N Value - Hard 4-8 Fine Soil N Value - Medium Stiff 15-30 Fine Soil N Value - Very Stiff Page 2 of 2 DESIGN CONSULTANTS, INC.

Some - Approximately 20 to 35%

And - Approximately 35 to 50%

30-50 Coarse Soil N Value - Dense

UT - Undisturbed Tube Sample Trace - Approximately 0 to 10%

Little - Approximately 10 to 20%

0-10 Coarse Soil N Value - Loose

				BORING LOG																											
Project: Location: Client: Driller: Drilling Met Weather: Performed I Checked By	CARR-DEE hods: PARTLY CLO By: PJS	ON, MA ILTING, INC. CORPORATIO HSA/D&W 2 OUDY, 50's Date:		DESIGN CONSULTANTS, INC.	Boring No: I Location: See F Approx. Ground Elevation: Approx. Groundwater Elevation: Date/Time of Groundwater Elevation: 12:45 Datum: BOSTON CITY B Project No. 2017-																										
Depth (feet)	Sample No.	Blows per 6-inch	Pen./ Rec.	Soil Description	Stratum Change Depth (feet)	Stratum	Note No.																								
1 2	S-1	6 7 7 5	24"/10"	S-1, Top 5" SAND, Some Silt, Some Gravel, Brown, Wet, Medium Dense Bottom 5" Similar to Top 5", Black	.5'	ASPHALT	(1)																								
- 3 - 4 - 5 - 5 - 6 - 7 - 8 - 9	S-2	1 1 2 2	24"/10"	S-2, SAND, Some Silt, Little Gravel, Trace Brick, Coal, Ash, Brown, Wet, Loose		FILL																									
10 11 12	S-3	1 2 3 3 2 3 2 2 2 2 2 2 2	2 3 3 2 3 2 2 2 2 2 2 2 3	2 3 3 2 3 2 2 2 2 2 2 2	2 3 3 2 3 2 2 2 2 2 2 2	2 3 3 2 3 2 2 2 2 2 2 2	2 3 3 2 3 2 2 2 2 2 2 2	2 3 3 2 3 2 2 2 2 2 2 2	2 3 3 2 3 2 2 2 2 2 2 2	2 3 3 2 3 2 2 2 2 2 2 2	2 3 3 2 3 2 2 2 2 2 2	24"/24"	S-3, SILT, Some Sand, Little Organics, Little Gravel, Little Clay, Grey-Brown, Wet, Medium Stiff S-4, SAND, Some Silt, Trace Seashells, Grey, Wet, Loose	10'	ORGANIC SILT	(2)															
— 13 — 14 — 15 — 16	S-4 S-5											2 2 2 2 2 2 3	2 2 2 2 2 2	2 2 2 2 2 2 3 6	2 2 2 2 2 2 3	2 2 2 2 2 3 6	2 2 2 2 2 2 3	2 2 2 2 2 2 3 6	2 2 2 2 2 2 3	2 2 2 2 2 2 3	2 2 2 2 2 2 3	2 2 2 2 2 2 3 6	2 2 2 2 2 2 3 6	2 2 2 2 2 2 3 6							
17 18 19 20	S-6	5 6 5	24"/16"	Next 6", SAND, Some Silt, Trace Organics, Grey, Wet, Medium Dense, Bottom 4" CLAY, Some Silt, Little Sand, Trace Gravel, Trace Organics, Grey, Wet, Stiff	17'	ORGANIC SAND CLAY																									
<u>NOTES:</u> (1) 5" OF AS		V GROUND SI	JRFACE	LEGEND S - Split Spoon Sample UT - Undisturbed Tube Sample Trace - Approximately 0 to 10% Little - Approximately 10 to 20% 0-10 Coarse Soil N Value - Loose 10-30 Coarse Soil N Value - Medium Dense 0-4 Fine Soil N Value - Medium Stiff	Some - Approximal And - Approximal 30-50 Coarse Soil >50 Coarse Soil N V	tely 35 to 50% I N Value - Dense I Value - Very Dense	il N Value - Hard																								

BORING LOG Project: 120 GOVE STREET Location: EAST BOSTON, MA Boring No: B-3 Client: EBI CONSULTING, INC. Location: See Plan Approx. Ground Elevation: Driller: **CARR-DEE CORPORATION** 17± Approx. Groundwater Elevation: Drilling Methods: HSA/D&W 20'+ 7± Weather: PARTLY CLOUDY, 50's Date/Time of Groundwater Elevation: 12:45 AM Performed By: PJS Date: 4/20/17 Datum: **BOSTON CITY BASE** PGC Date: 4/26/17 DESIGN CONSULTANTS, INC. Project No. 2017-032 Checked By: Stratum Depth Sample Blows per Soil Description Change Note Pen./ Stratum Depth (feet) No. 6-inch Rec. No. (feet) S-7, CLAY, Little Fine Sand, Grey, Wet, 9 Very Stiff 6 24"/18" 21 S-7 13 13 22 23 24 CLAY 25 8 S-8, Similar to S-7 10 26 S-8 24"/18" 13 15 27 10 S-9, Top 10" Similar to S-8, Bottom 2" SAND, Some Silt, 9 28 S-9 24"/12" 9 Grey, Wet, Medium Dense 28.7 8 29' SAND 29 **BOTTOM OF BORING AT 29' BGS** 30 31 32 33 34 35 36 37 38 39 40 NOTES: <u>LEGEND</u>

S - Split Spoon Sample O/A - Sample Collected Off the Augers UT - Undisturbed Tube Sample Trace - Approximately 0 to 10% Some - Approximately 20 to 35% Little - Approximately 10 to 20% And - Approximately 35 to 50% 0-10 Coarse Soil N Value - Loose 30-50 Coarse Soil N Value - Dense 10-30 Coarse Soil N Value - Medium Dense >50 Coarse Soil N Value - Very Dense 0-4 Fine Soil N Value - Soft 8-15 Fine Soil N Value - Stiff >30 Fine Soil N Value - Hard 4-8 Fine Soil N Value - Medium Stiff 15-30 Fine Soil N Value - Very Stiff

Page 2 of 2 DESIGN CONSULTANTS, INC.

				BORING LOG																															
Project: Location: Client: Driller: Drilling Meth Weather: Performed B Checked By:	CARR-DEE (nods: CLOUDY, 5 dy: PJS	ON, MA LTING, INC. CORPORATIO HSA/D&W 20 O's Date:		DESIGN CONSULTANTS, INC.	Boring No: Location: See Approx. Ground Elevation: 1 Approx. Groundwater Elevation: 9:30 Date/Time of Groundwater Elevation: 9OSTON CITY E Project No. 2017																														
Depth (feet)	Sample No.	Blows per 6-inch	Pen./ Rec.	Soil Description	Stratum Change Depth (feet)	Stratum	Note No.																												
_ 1 _ 1 _ 2 _ 2	S-1	11 16 13 10	24"/10"	S-1, SAND, Some Silt, Trace Gravel, Brick, Ash, Coal, Light Brown to Black, Dry, Medium Dense	.5'	ASPHALT	(1)																												
3 - 4 - 5 - 6 - 7 - 8	S-2	14 7 3 4	24"/12"	S-2, Similar to S-1, Trace Ceramic, Moist, Loose to Medium Dense		FILL																													
9 - 10 - 11	S-3	2 1 2	24"/12"	S-3, Top 6" Similar to S-2 Bottom 6", ORGANICS, Some Silt, Little Sand, Black, Wet, Soft	10.5'	ORGANICS	(2)																												
12 13 14	S-4	2 1 5 4 7 70/5" 2 2 2 5 6	1 5 4 7 70/5" 2 2 2 5	1 5 4 7 70/5" 2 2 2 5	1 5 4 7 70/5" 2 2 2 5	1 5 4 7 70/5" 2 2 2 5	1 5 4 7 70/5" 2 2 2 5	1 5 4 7 70/5" 2 2 2 5	1 5 4 7 70/5" 2 2 2 5	1 5 4 7 70/5" 2 2 2 5	1 5 4 7 70/5" 2 2 2 5	1 5 4 7	1 5 4 7	1 5 4 7	1 5 4 7	1 5 4 7	1 5 4 7	1 5 4 7	1 5 4 7	1 5 4 7	1 5 4 7	1 5 4 7	1 5 4 7	1 5 4 7	1 5 4 7	5 4 7	1 5 4 7	1 5 4 7	1 5 4 7	1 5 4 7	23"/12"	S-4, Top 3" Similar to S-3 Bottom 9", SILT, Some Sand, Trace Organics, Brown and Orange, Wet, 2" of Wood in spoon tip, Stiff	12.3' 14' 14.5'	FILL	(3)
	S-5											24"/18"	S-5, Top 9", SILT, Some Clay, Little Sand, Trace Seashell, Grey, Wet, Medium Stiff, Next 3" PEAT, Brown, Wet, Medium Stiff, Bottom 6" SAND, Little Silt, Trace Organics, Grey, Wet, Med. Dense	15.5' 16'	ORGANIC SILT PEAT SILTY SAND																				
— 18 — 19 — 20					18'	CLAY																													
(3) STRONG	T 10' BELOV		JRFACE	LEGEND S - Split Spoon Sample UT - Undisturbed Tube Sample Trace - Approximately 0 to 10% Some - Approximately 20 to 35% Little - Approximately 10 to 20% And - Approximately 35 to 50% 0-10 Coarse Soil N Value - Loose 10-30 Coarse Soil N Value - Medium Dense 0-4 Fine Soil N Value - Soft 8-15 Fine Soil N Value - Stiff >30 Fine Soil N Value - Very Stiff																															

				BORING LOG			
Project: 120 GOVE STREET Location: EAST BOSTON, MA Client: EBI CONSULTING, INC. Driller: CARR-DEE CORPORATION Drilling Methods: HSA/D&W 20'+ Weather: CLOUDY, 50's Performed By: PJS Date: 4/20/17 Checked By: PGC Date: 4/26/17				DESIGN CONSULTANTS, INC.	Approx. Gro		B-4 See Plan 17.5± 7.5± 9:30 AM V CITY BASE 2017-032
Depth (feet)	Sample No.	Blows per 6-inch	Pen./ Rec.	Soil Description	Stratum Change Depth (feet)	Stratum	Note No.
	S-6	6 7 10 11 4 6 8 8 13 9 9 8	24"/21" 24"/21" 24"/18"	S-6, CLAY, Some Silt, Little Sand, Grey, Wet, Very Stiff S-7, Similar to S-6, Trace Sand, Stiff S-8, Top 9" Similar to S-7, Very Stiff Bottom 9", SAND, Some Silt, Grey, Wet, Medium Dense BOTTOM OF BORING AT 29' BGS	28'	CLAY	
33				<u>LEGEND</u> S - Split Spoon Sample	O/A - Sample Co	llected Off the Augers	
				UT - Undisturbed Tube Sample Trace - Approximately 0 to 10% Little - Approximately 10 to 20% 0-10 Coarse Soil N Value - Loose 10-30 Coarse Soil N Value - Medium Dense 0-4 Fine Soil N Value - Soft 4-8 Fine Soil N Value - Medium Stiff	And - Approxima 30-50 Coarse Soil N >50 Coarse Soil N 8-15 Fine Soil N	Il N Value - Dense N Value - Very Dense	bil N Value - Hard

				BORING LOG			
Project: Location: Client: Driller: Drilling Metl Weather: Performed E Checked By:	CARR-DEE (hods: SUNNY, 50 By: PJS	ON, MA ILTING, INC. CORPORATIO HSA/D&W 1! 's Date:		DESIGN CONSULTANTS, INC.	Approx. Gr		B-5 See Plan 17.5± 8.5± 10:10 AM N CITY BASE 2017-032
Depth (feet)	Sample No.	Blows per 6-inch	Pen./ Rec.	Soil Description	Stratum Change Depth (feet)	Stratum	Note No.
1 2 3 4	S-1	3 5 6 4	24"/12"	S-1, Top 4" Topsoil Bottom 8", SAND, Some Silt, Trace Brick and Coal, Brown to Black, Dry, Medium Dense	.3'	TOPSOIL	(1)
	S-2	6 5 3 4	24"/8"	S-2, Similar to bottom of S-1, Little Gravel, Dry, Black, Loose		FILL	(2)
	S-3	2 4 4 4 3 3 2	24"/3"	S-3, Similar to above, Red Brick Pieces, Wet, Loose S-4, Top 1" Brick Bottom 1" SAND, Some Silt, Brown to Black, Wet, Wood in spoon tip,		_	
14 15 16 17 18 19	S-5 S-6	4 1 2 3 1 2 5 5 8	24"/17"	Loose S-5, CLAY, Some Silt, Little Sand, Trace Organics, Trace Seashells, Grey, Wet, Medium Stiff S-6, Top 15" Similar to S-5, Stiff Next 2" PEAT, Brown, Wet, Stiff Bottom 7" CLAY and Silt, Little Fine Sand, Grey w/ Olive Streaks, Wet, Stiff		ORGANIC CLAY W/ SEASHELLS PEAT SILTY CLAY	
		E DUE TO BRA		LEGEND S - Split Spoon Sample UT - Undisturbed Tube Sample Trace - Approximately 0 to 10% Little - Approximately 10 to 20% 0-10 Coarse Soil N Value - Loose 10-30 Coarse Soil N Value - Medium Dense 0-4 Fine Soil N Value - Soft 4-8 Fine Soil N Value - Medium Stiff	Some - Approxin And - Approxin 30-50 Coarse S >50 Coarse Soil 8-15 Fine Soil N	imately 20 to 35% nately 35 to 50% oil N Value - Dense I N Value - Very Dense I Value - Stiff N Value - Very Stiff	oil N Value - Hard

DESIGN CONSULTANTS, INC. Page 1 of 2

				BORING LOG				
Project: Location: Client: Driller: Drilling Met Weather: Performed I Checked By	CARR-DEE (hods: SUNNY, 50' By: PJS	ON, MA LTING, INC. CORPORATIO HSA/D&W 1 's Date:		DESIGN CONSULTANTS, INC.	Boring No: B Location: See Pl. Approx. Ground Elevation: 17. Approx. Groundwater Elevation: 8. Date/Time of Groundwater Elevation: 10:10 A Datum: BOSTON CITY BA Project No. 2017-0			
Depth (feet)	Sample No.	Blows per 6-inch	Pen./ Rec.	Soil Description	Stratum Change Depth (feet)	Stratum	Note No.	
	S-7	10 10 14 19	24"/24"	S-7, CLAY, Some Silt, Trace Sand, Grey, Wet, Very Stiff				
						SILTY CLAY	(3)	
25 26 27	S-8	2 4 3 4	24"/24"	S-8, Top 12" Similar to S-7, Med. Stiff Next 4" SAND, Little Clay, Grey, Wet Bottom 8" CLAY and Sand, Grey, Wet, Medium Stiff	26' 26.5'	CLAYEY SAND		
28 29 30						SANDY CLAY		
_ 31 _ 32 _ 32	S-9	5 4 8 8	24"/16"	S-9, Top 6" CLAY, Some Sand, Grey, Wet, Stiff Bottom 10" SAND, Little Clay, Grey, Wet, Medium Dense	31'			
— 33 — 34 — 35		14		C 10 CAND Little Class and Cite Cours		CLAYEY SAND		
	S-10	11 11 8 9	24"/15"	S-10, SAND, Little Clay and Silt, Grey, Wet, Medium Dense BOTTOM OF BORING AT 37' BGS	37'			
38 39 40				DOTTON OF BURING AT 37 DG3				
<u>NOTES:</u>	OF INCHES C	l Of Gravel at	23' BGS	LEGEND S - Split Spoon Sample UT - Undisturbed Tube Sample Trace - Approximately 0 to 10% Little - Approximately 10 to 20% 0-10 Coarse Soil N Value - Loose 10-30 Coarse Soil N Value - Medium Dense	Some - Approxima And - Approxima 30-50 Coarse So	illected Off the Augers nately 20 to 35% stely 35 to 50% il N Value - Dense N Value - Very Dense		
				0-4 Fine Soil N Value - Soft 4-8 Fine Soil N Value - Medium Stiff	8-15 Fine Soil N		oil N Value - Hard	

DESIGN CONSULTANTS, INC.

Page 2 of 2

				BORING LOG			
Location: Client: Driller: Drilling Meth	CARR-DEE (nods: SUNNY, 50 y: PJS	ON, MA LTING, INC. CORPORATIO HSA/D&W 1! 's Date:		DESIGN CONSULTANTS, INC.	Approx. G		B-6 See Plan 17.5± 8.5± 7:00 AM N CITY BASE 2017-032
Depth (feet)	Sample No.	Blows per 6-inch	Pen./ Rec.	Soil Description	Stratum Change Depth (feet)	Stratum	Note No.
1 1 2	S-1	2 1 8 7	24"/11"	S-1, Top 4" Topsoil Next 7", SAND, Little Silt, Trace Brick, Coal, Drown, Dry, Loose	.3'	TOPSOIL	
- 3 - 4 - 5 - 6 - 7 - 8 - 9	S-2	2 2 3 3	24"/14"	S-2, SILT and SAND, Little Ash, Trace Wood, Coal, Black, Dry, Loose to Medium Stiff		FILL	(1)
	S-3	2 1 2 2	24"/6"	S-3, Similar to S-2, Wet, Loose to Soft			
— 12 — 13 — 14	S-4	1 1 1 1 2	24"/10"	S-4, SAND, Some Silt, Trace Wood, Trace Ash, Black, Wet, Loose	14.5'		
	S-5	1/12" 5	24"/18"	S-5, Top 12" PEAT, Brown, Wet, Soft, Next 2" SAND And Silt, Trace Organics, Trace Seashells, Grey, Wet, Med. Stiff,	16.5'	PEAT	
— 17 — 18 — 19 — 20		13		Next 2" Cobble, Bottom 2" Similar to Middle 2"	18.5'	ORGANIC SILTY SAND W/ SEASHELLS CLAY	
NOTES: (1) WATER A	T 9' BELOW	GROUND SUF	RFACE	LEGEND S - Split Spoon Sample UT - Undisturbed Tube Sample Trace - Approximately 0 to 10% Little - Approximately 10 to 20% O-10 Coarse Soil N Value - Loose 10-30 Coarse Soil N Value - Medium Dense 0-4 Fine Soil N Value - Soft 4-8 Fine Soil N Value - Medium Stiff	Some - Approximado - Approximado - Approximado - Approximado - Son Coarse Son - Son Coarse Son - Approximado - App	Collected Off the Augers kimately 20 to 35% mately 35 to 50% Soil N Value - Dense il N Value - Very Dense N Value - Stiff >30 Fine Si	oil N Value - Hard

DESIGN CONSULTANTS, INC. Page 1 of 2

				BORING LOG			
Project: Location: Client: Driller: Drilling Met Weather: Performed I Checked By	Carr-dee thods: Sunny, 50 By: PJS	CON, MA ILTING, INC. CORPORATIO HSA/D&W 1 ''s Date:		DESIGN CONSULTANTS, INC.	Approx. Gr		B-6 See Plan 17.5± 8.5± 7:00 AM CITY BASE 2017-032
Depth (feet)	Sample No.	Blows per 6-inch	Pen./ Rec.	Soil Description	Change Depth (feet)	Stratum	Note No.
	S-6	10 15 21 25	24"/12"	S-6, CLAY, Some Silt, Little Fine Sand, Grey, Wet, Very Stiff			
24 25 26 27 28	S-7	3 2 4 4	24"/16"	S-7, Top 6" CLAY, Little Fine Sand, Grey, Wet, Medium Stiff, Next 2", SAND, Little Clay, Grey, Wet Next 4" Similar to Top 6" Bottom 1" Similar to Middle 2"		CLAY W/ ALTERNATING SAND SEAMS	
28 — 29 — 30 — 31 — 32	S-8	5 5 6 7	24"/15"	S-8, Top 4" Similar to Top 6" of S-7 Next 9", Similar to Middle 2" of S-7, Medium Stiff Bottom 2" similar to Top 4"			
		4		C.O. CANID. Little Cilt. Crov. West	34'		
	S-9	6 6 8 10	24"/16"	S-9, SAND, Little Silt, Grey, Wet, Medium Dense	37'	SAND	
				BOTTOM OF BORING AT 37' BGS			
NOTES:				LEGEND S - Split Spoon Sample UT - Undisturbed Tube Sample Trace - Approximately 0 to 10% Little - Approximately 10 to 20% 0-10 Coarse Soil N Value - Loose 10-30 Coarse Soil N Value - Medium Dense 0-4 Fine Soil N Value - Soft 4-8 Fine Soil N Value - Medium Stiff	Some - Approxim And - Approxim 30-50 Coarse So >50 Coarse Soil 8-15 Fine Soil N	mately 20 to 35% mately 35 to 50% pil N Value - Very Dense N Value - Stiff N Value - Very Stiff	l N Value - Hard

DESIGN CONSULTANTS, INC. Page 2 of 2

				BORING LOG			
Project: Location: Client: Driller: Drilling Met Weather: Performed E Checked By:	CARR-DEE (hods: CLOUDY, 5 By: PJS	ON, MA LTING, INC. CORPORATIO HSA/D&W 2 O's Date:		DESIGN CONSULTANTS, INC.	Approx. Gro	Boring No: ound Elevation: oundwater Elevation: of Groundwater Elevation: BOSTON	B-7 See Plan 17.5± 7.5± 7:15 AM N CITY BASE 2017-032
Depth (feet)	Sample No.	Blows per 6-inch	Pen./ Rec.	Soil Description	Stratum Change Depth (feet)	Stratum	Note No.
1 2 	S-1	9 5 3 3	24"/6"	S-1, Top 2" Topsoil Next 2" SAND, Little Silt, Light Brown, Dry, Loose Bottom 2", SAND, Some Silt, Little Coal, Olive, Dry, Loose	.2'	TOPSOIL	
- 3 - 4 - 5 - 6 - 7 - 8 - 9	S-2	11 8 9 6	24"/16"	S-2, Similar to bottom of S-1, Little Gravel, Trace Brick, Trace Ceramic, Black, Dry, Medium Dense		FILL	
	S-3	1/12" 1/12" 1 1	24"/16"	S-3, SAND, Some Silt, Little Gravel, Trace Coal, Grey, Wet, Loose S-4, Top 7" Similar to S-3, Bottom 6" PEAT, Olive, Wet, Soft	12.5'		(1)
_ 13 _ 14 _ 15 _ 16 _ 17	S-4 S-5 S-6	1 1 1 1 1 2 2 1 2 3	24"/13" 24"/17"	S-5, Similar to S-4, Grey, Trace Seashells, Soft S-6, Top 15" similar to S-5, Bottom 3", SILT, Some Clay, Some Sand, Trace Organics, Grey, Wet, Soft to		PEAT	
— 18 — 19 — 20		3		Medium Stiff	18'	ORGANIC SILT	
<i>NOTES:</i> (1) WATER A	T 10' BELOV	V GROUND SU	JRFACE	LEGEND S - Split Spoon Sample UT - Undisturbed Tube Sample Trace - Approximately 0 to 10% Little - Approximately 10 to 20% 0-10 Coarse Soil N Value - Loose 10-30 Coarse Soil N Value - Medium Dense 0-4 Fine Soil N Value - Medium Stiff	Some - Approxima And - Approximat 30-50 Coarse Soil >50 Coarse Soil N V	tely 35 to 50% I N Value - Dense I Value - Very Dense	oil N Value - Hard

DESIGN CONSULTANTS, INC. Page 1 of 2

					BORING LOG			
Loc Clie Dri Dri We	Project: 120 GOVE STREET Location: EAST BOSTON, MA Client: EBI CONSULTING, INC. Driller: CARR-DEE CORPORATION Drilling Methods: HSA/D&W 20'+ Weather: CLOUDY, 50's Performed By: PJS Date: 4/20/17 Checked By: PGC Date: 4/26/17				D(Approx. Gro	Boring No: ound Elevation: oundwater Elevation: of Groundwater Elevation: BOSTC	B-7 See Plan 17.5± 7.5± 7:15 AM ON CITY BASE
	<u> </u>	PGC	Date:	4/26/17	DESIGN CONSULTANTS, INC.	Project No. Stratum		2017-032
	Depth (feet)	Sample No.	Blows per 6-inch	Pen./ Rec.	Soil Description	Change Depth (feet)	Stratum	Note No.
_ _ _ _	21 22	S-7	3 5 6 8	24"/24"	S-7, CLAY, Little Silt, Little Sand, Trace Organics, Wet, Grey, Stiff		CLAY	
_ _ _ _	23 24					23.5'		
 - - -	252627	S-8	13 10 15 24	24"/15"	S-8 SAND, Some Silt, Grey, Wet, Medium Dense		SAND	
<u> </u>	28	S-9	21 18 26 18	24"/24"	S-9, Similar to S-8, Little Silt, Dense	29'		
	29		10		BOTTOM OF BORING AT 29' BGS			
H	30							
_	31							
<u> </u>	32 33							
	34							
	35							
	36							
	37							
	38 39							
F	40							
NO	OTES:	1	ı	<u>I</u>	<u>LEGEND</u>			ı
	_				S - Split Spoon Sample UT - Undisturbed Tube Sample Trace - Approximately 0 to 10%		llected Off the Augers nately 20 to 35%	
					Little - Approximately 10 to 20%	And - Approxima	ately 35 to 50%	
					0-10 Coarse Soil N Value - Loose		il N Value - Dense	
					10-30 Coarse Soil N Value - Medium Dense		N Value - Very Dense	
					0-4 Fine Soil N Value - Soft 4-8 Fine Soil N Value - Medium Stiff	8-15 Fine Soil N 15-30 Fine Soil N	Value - Stiff >30 Fine I Value - Very Stiff	Soil N Value - Hard

DESIGN CONSULTANTS, INC. Page 2 of 2

				BORING LOG			
Project: Location: Client: Driller: Drilling Met Weather: Performed I Checked By	CARR-DEE (hods: PARTLY CLO By: PJS	ON, MA LTING, INC. CORPORATIO HSA/D&W 1 OUDY, 50's Date:		DESIGN CONSULTANTS, INC.	1	ndwater Elevation: Groundwater Elevation:	B-8 See Plan 17.5± 7.5± 7:30 AM I CITY BASE 2017-032
Depth (feet)	Sample No.	Blows per 6-inch	Pen./ Rec.	Soil Description	Stratum Change Depth (feet)	Stratum	Note No.
	S-1	6 5 11 8	24"/11"	S-1, SAND and SILT, Little Gravel, Black and Grey, Dry, Medium Dense	.2'	ASPHALT	(1)
3 4 5 6 7 	S-2	4 1 1 1	24"/8"	S-2, SILT and SAND, Trace Wood, Coal, Ash, Black, Moist, Loose		FILL	
9	S-3	5 3 1 3	24"/5"	S-3, Top 2" Similar to S-2, Bottom 3" SILT, Little Sand, Gry, Wet, Medium Stiff to Soft	10'	SILT	(2)
14	S-4	2 3 4 5	24"/3"	S-4, PEAT, Some Silt, Little Sand, Trace Wood, Olive, Wet, Medium Stiff	18.5'	PEAT	
— 19 — 20						CLAY	
	ies of Asph <i>i</i> At 10' Belov	ALT V GROUND SU	JRFACE	LEGEND S - Split Spoon Sample UT - Undisturbed Tube Sample Trace - Approximately 0 to 10% Little - Approximately 10 to 20% 0-10 Coarse Soil N Value - Loose 10-30 Coarse Soil N Value - Medium Dense 0-4 Fine Soil N Value - Medium Stiff	O/A - Sample Collect Some - Approximate And - Approximate 30-50 Coarse Soil N Val 8-15 Fine Soil N Val 15-30 Fine Soil N V	ly 35 to 50% I Value - Dense Yalue - Very Dense ue - Stiff >30 Fine Sc	il N Value - Hard

DESIGN CONSULTANTS, INC. Page 1 of 2

				BORING LOG			
,	120 GOVE			N		Doring Mo.	B-8
	EAST BOST	LTING, INC.			Location:	Boring No:	See Plan
		CORPORATIO	N			und Elevation:	17.5±
Drilling Meth		HSA/D&W 10				und Elevation: undwater Elevation:	7.5±
_	PARTLY CLO		0 +			of Groundwater Elevation:	7:30 AM
Performed B			4/19/17		Datum:		ON CITY BASE
Checked By:	PGC		4/26/17	DESIGN CONSULTANTS, INC.	Project No.	50310	2017-032
· · · · · · ·					Stratum		
Depth (feet)	Sample No.	Blows per 6-inch	Pen./ Rec.	Soil Description	Change Depth (feet)	Stratum	Note No.
_	S-5A	10 8		S-5, Top 5" CLAY, Little Gravel, Grey, Wet, Stiff	20.5'	CLAY	
– 21 –	S-5B	8 8	24"/18"	Bottom 13", SAND, Trace Silt, Grey, Wet, Medium Dense		SAND	(3)
- 22 -					23'	5.1.15	
- 23 -							
– 24 –							
- 25 -		5 5		S-6, CLAY, Little Sand, Grey, Wet, Stiff			(4)
– 26 –	S-6	5 9	24"/17"			CLAY	(',
– 27 –		ŕ					
- 28 -					29'		
- 29							
– 30 –		6 5		S-7, SAND, Trace Clay, Grey, Wet, Medium Dense		SAND	(5)
- 31 -	S-7	10 5	24"/14"		32'		, ,
- 32 -				BOTTOM OF BORING AT 32' BGS			
– 33 –							
– 34 –							
- 35 -							
- 36 -							
- 37 -							
- 38 -							
- 39 - 40							
NOTES:			1	LEGEND			
(3) TWO 1/4-	INCH CLAY	BANDS IN SAN	ND	S - Split Spoon Sample	O/A - Sample Colle	ected Off the Augers	
(4) TWO 1/2-	INCH BAND	S OF SAND		UT - Undisturbed Tube Sample			
(5) 1-INCH CL	AY BAND T	HREE INCHES	INTO S-7	Trace - Approximately 0 to 10%	Some - Approxima	ately 20 to 35%	
				Little - Approximately 10 to 20%	And - Approximate	ely 35 to 50%	
				0-10 Coarse Soil N Value - Loose	30-50 Coarse Soil I	N Value - Dense	
				10-30 Coarse Soil N Value - Medium Dense	>50 Coarse Soil N	Value - Very Dense	
				0-4 Fine Soil N Value - Soft	8-15 Fine Soil N Va	alue - Stiff >30 Fine	Soil N Value - Hard
				4-8 Fine Soil N Value - Medium Stiff	15-30 Fine Soil N \	Value - Very Stiff	

DESIGN CONSULTANTS, INC. Page 2 of 2

				BORING LOG			
Project: Location: Client: Driller: Drilling Meth Weather: Performed B Checked By:	CARR-DEE (nods: SUNNY, 50 sy: PJS	ON, MA LTING, INC. CORPORATIO HSA/D&W 20 's Date:		DESIGN CONSULTANTS, INC.	Approx. Gro		B-9 See Plan 17.5± 7.5± 11:45 AM I CITY BASE 2017-032
Depth (feet)	Sample No.	Blows per 6-inch	Pen./ Rec.	Soil Description	Stratum Change Depth (feet)	Stratum	Note No.
1 1 2	S-1	6 4 4 2	24"/14"	S-1, SAND, Some Silt, Trace Gravel, Trace Coal, Trace Ash, Dark Brown, Dry, Loose	.2'	ASPHALT	(1)
345678	S-2	8 7 7 8	24"/18"	S-2, Similar to S-1, Grey, Medium Dense		FILL	
- 9 - 10 - 11	S-3	2 2 1	24"/6"	S-3, Top 3" Similar to S-2 Bottom 3" PEAT, Some Silt, Little Sand, Little Organics, Wet, Soft	11'		(2)
12 - 13	S-4	1 2 2 2 3	24"/4"	S-4, Similar to S-3, Soft to Med. Stiff			
— 14 — 15 — 16	S-5	2 3 3 3	24"/24"	S-5, Top 10" Similar to S-4, Bottom 14" Similar to S-4, Some Organics, Brown, Medium Stiff		PEAT	
17 18	S-6	4 3 3 5	24"/24"	S-6, Top 23" Similar to top of S-5, Bottom 1", SAND, Trace Silt, Grey, Wet, Medium Dense	18'		
						SAND	
<u>NOTES:</u> (1) 2 INCHES (2) WATER A		T V GROUND SL	IRFACE	LEGEND S - Split Spoon Sample UT - Undisturbed Tube Sample Trace - Approximately 0 to 10% Little - Approximately 10 to 20% 0-10 Coarse Soil N Value - Loose 10-30 Coarse Soil N Value - Medium Dense 0-4 Fine Soil N Value - Soft 4-8 Fine Soil N Value - Medium Stiff	Some - Approxima And - Approxima 30-50 Coarse Soil N 8-15 Fine Soil N	ntely 35 to 50% Il N Value - Dense N Value - Very Dense	il N Value - Hard

DESIGN CONSULTANTS, INC. Page 1 of 3

				BORING LOG			
Weather: Performed	Location: EAST BOSTON, MA Client: EBI CONSULTING, INC. Driller: CARR-DEE CORPORATION Drilling Methods: HSA/D&W 20'+			DESIGN CONSULTANTS, INC.	Boring No: Location: Se Approx. Ground Elevation: Approx. Groundwater Elevation: Date/Time of Groundwater Elevation: 11: Datum: BOSTON CIT Project No. 20		
Depth (feet)	Sample No.	Blows per 6-inch	Pen./ Rec.	Soil Description	Stratum Change Depth (feet)	Stratum	Note No.
	S-7	4 6 10 13	24"/13"	S-7, SAND, Little Silt, Grey, Wet, Medium Dense		SAND	
23 24 25 26 27 28 29 30	S-8	4 4 4 8 8	24"/12"	S-8, SAND and CLAY, Grey, Wet, Loose to Medium Stiff S-9, Similar to S-9	23'	CLAYEY SAND	
- 31 - 32 - 33 - 33 - 34 - 35	S-9	6 6 7 3	24"/10"	S-10, Top 4" SAND and CLAY, Grey, Wet, Medium Dense,	35.5'		
- 36 - 37 - 38 - 39 - 40	S-10	4 5	24"/16"	Next 2" CLAY, Grey, Wet, Medium Stiff Next 4" Same as Top 4" Next 4" Same as Middle 2" Bottom 2" Same as Top 4"		CLAY	
NOTES:				LEGEND S - Split Spoon Sample UT - Undisturbed Tube Sample Trace - Approximately 0 to 10% Little - Approximately 10 to 20% 0-10 Coarse Soil N Value - Loose 10-30 Coarse Soil N Value - Medium Dense 0-4 Fine Soil N Value - Medium Stiff	Some - Approxima And - Approxima 30-50 Coarse So >50 Coarse Soil I 8-15 Fine Soil N	il N Value - Dense N Value - Very Dense	il N Value - Hard

DESIGN CONSULTANTS, INC. Page 2 of 3

BORING LOG

Project: 120 GOVE STREET Location: EAST BOSTON, MA Client: EBI CONSULTING, INC. CARR-DEE CORPORATION Driller: Drilling Methods: HSA/D&W 20'+

Weather: SUNNY, 50's

Performed By: PJS Date: 4/19/17

Checked By: PGC Date: 4/26/17



DESIGN CONSULTANTS, INC.

B-9 Boring No:

11:45 AM

Location: See Plan

Approx. Ground Elevation: 17.5± Approx. Groundwater Elevation: 7.5±

Datum: **BOSTON CITY BASE**

Date/Time of Groundwater Elevation:

2017-032 Project No.

Depth (feet)	Sample No.	Blows per 6-inch	Pen./ Rec.	Soil Description	Stratum Change Depth (feet)	Stratum	Note No.
		3		S-11, Top " CLAY, Some Sand, Grey, Wet,			
— 41 —	S-11	3 4	24"/20"	Medium Stiff; Middle 5" SAND And Clay, Grey, Wet; Bottom 10" CLAY, Grey, Wet,		CLAY	
— — 42		4		Medium Stiff BOTTOM OF BORING AT 42' BGS	42'		
<u>L</u>				BOTTOM OF BORING AT 42 BGS			
— 43 —							
- 44							
45							
— — 46							
- 43 - 44 - 45 - 46 - 47							
— — 48							
— — 49							
— — 50							
51 52 53							
_ _ 53							
— — 54							
— — 55							
— — 56							
— — 57							
— — 58							
— — 59							
60							
<u>NOTES:</u>				<u>LEGEND</u>			
				S - Split Spoon Sample	O/A - Sample Co	ollected Off the Augers	

S - Split Spoon Sample O/A - Sample Collected Off the Augers UT - Undisturbed Tube Sample Trace - Approximately 0 to 10% Some - Approximately 20 to 35% Little - Approximately 10 to 20% And - Approximately 35 to 50% 0-10 Coarse Soil N Value - Loose 30-50 Coarse Soil N Value - Dense 10-30 Coarse Soil N Value - Medium Dense >50 Coarse Soil N Value - Very Dense 0-4 Fine Soil N Value - Soft 8-15 Fine Soil N Value - Stiff >30 Fine Soil N Value - Hard 4-8 Fine Soil N Value - Medium Stiff 15-30 Fine Soil N Value - Very Stiff

Page 3 of 3 DESIGN CONSULTANTS, INC.

				BORING LOG			
Project: Location: Client: Driller: Drilling Met Weather: Performed B	CARR-DEE (hods: SUNNY, 50 By: PJS	ON, MA LTING, INC. CORPORATIO HSA/D&W 2 's Date:		DESIGN CONSULTANTS, INC.	Approx. Grou	Boring No: und Elevation: undwater Elevation: f Groundwater Elevation: BOSTO!	B-10 See Plan 17.5± 7.5± 9:30 AM N CITY BASE 2017-032
Depth (feet)	Sample No.	Blows per 6-inch	Pen./ Rec.	Soil Description	Stratum Change Depth (feet)	Stratum	Note No.
1 1 2	S-1	11 12 7 2	24"/8"	S-1, Top 1" ASPHALT Bottom 23" SAND, Little Silt, Little Gravel, Trace Coal, Dark Brown, Dry, Medium Dense	.1'	ASPHALT	(1), (2)
345678	S-2	3 9 8 6	24"/8"	S-2, Top 3" SAND And Silt, Olive, Dry, Medium Dense Bottom 5" similar to Top 3", Trace Coal, Black, Dry, Medium Dense		FILL	
9 10 11	S-3	1 1/10"	24"/10"	S-3, Top 8" Similar to Bottom of S-2, Bottom 2" PEAT, Some Silt, Little Sand, Little Organics, Olive, Wet, Very Soft	10.5'		(3)
	S-4	1/8" 1/18" 1 1 1	24"/18"	S-4, Similar to bottom of S-3 S-5, Similar to S-4, Brown, Soft		PEAT	
— 15 — 16 — 17	S-5 S-6	2 2 1 1 6	24"/24"	S-6, Top 10" Similar to S-5 Bottom 5" SAND, Little Silt, Grey, Wet, Medium Dense	17'		
— 18 — 19 — 20		10				SAND	
(2) 1-INCH C	F ASPHALT	ne due to w V ground su		LEGEND S - Split Spoon Sample UT - Undisturbed Tube Sample Trace - Approximately 0 to 10% Little - Approximately 10 to 20% 0-10 Coarse Soil N Value - Loose 10-30 Coarse Soil N Value - Medium Dense 0-4 Fine Soil N Value - Soft 4-8 Fine Soil N Value - Medium Stiff	Some - Approxima And - Approximate 30-50 Coarse Soil I	N Value - Dense Value - Very Dense silue - Stiff >30 Fine Sc	oil N Value - Hard

DESIGN CONSULTANTS, INC. Page 1 of 2

BORING LOG

Project: 120 GOVE STREET

Location: EAST BOSTON, MA

Client: EBI CONSULTING, INC.

Driller: CARR-DEE CORPORATION

Drilling Methods: HSA/D&W 20'+

Weather: SUNNY, 50's

Performed By: PJS Date: 4/19/17

Checked By: PGC Date: 4/26/17



Boring No: B-10

Location: See Plan Approx. Ground Elevation: $17.5\pm$

Approx. Ground Elevation: $17.5\pm$ Approx. Groundwater Elevation: $7.5\pm$ Date/Time of Groundwater Elevation: 9:30 AM

Datum: BOSTON CITY BASE

Project No. 2017-032

oncontou by:		Dato.	., 20, .,	DESIGN CONSCENTING, INC.		•	2017 002
Depth (feet)	Sample No.	Blows per 6-inch	Pen./ Rec.	Soil Description	Stratum Change Depth (feet)	Stratum	Note No.
21 22	S-7	5 7 7 9	24"/17"	S-7, SAND, Some Clay, Grey, Wet, Medium Dense Two 1/4" CLAY lenses in S-7			
_ _ 23							
24							
— 25 —		5 5		S-8, Top 10" Similar to S-7, Little Clay Middle 1" CLAY, Grey, Wet, Very Stiff			
— 26 —	S-8	9	24"/16"	Bottom 5" Similar to Top 10"		CLAYEY SAND	
— 27 — 20		10					
28 29							
- 30		_					
_ _ 31	S-9	7 11 12	24"/11"	S-9, SAND, Some Clay, Grey, Wet, Medium Dense			
_ _ 32		11		BOTTOM OF BORING AT 32' BGS	32'		
_ 33							
34 							
— 35 —							
— 36 —							
— 37 — 38							
_ _ 39							
40							
<u>NOTES:</u>				<u>LEGEND</u>			
				S - Split Spoon Sample	O/A - Sample C	ollected Off the Augers	
				UT - Undisturbed Tube Sample			

DESIGN CONSULTANTS, INC. Page 2 of 2

Some - Approximately 20 to 35%

And - Approximately 35 to 50%

8-15 Fine Soil N Value - Stiff

30-50 Coarse Soil N Value - Dense

>50 Coarse Soil N Value - Very Dense

15-30 Fine Soil N Value - Very Stiff

>30 Fine Soil N Value - Hard

Trace - Approximately 0 to 10%

Little - Approximately 10 to 20%

0-10 Coarse Soil N Value - Loose

4-8 Fine Soil N Value - Medium Stiff

0-4 Fine Soil N Value - Soft

10-30 Coarse Soil N Value - Medium Dense

Attachment D

Climate Change Checklist



Submitted: 10/02/2018 12:47:43

A.1 - Project Information

Project Name: Frankfort Gove Street Housing Project

Project Address: 115 Gove Street, 120 Gove Street, 128-134 Gove Street, 21-43 Frankfort Street

Filing Type: Initial (PNF, EPNF, NPC or other substantial filing)

Filing Contact: Fiona **Epsilon** fvardy@epsilonassociat 9784616243

> Vardy Associates, Inc. es.com

MEPA date: Is MEPA approval required? No

A.2 - Project Team

Frankfort Gove LLC Owner / Developer:

Architect: **Bruner-Cott & Associates**

Engineer: **BSC Group**

Sustainability / LEED: **Bruner-Cott & Associates**

Permitting: **Epsilon Associates**

Construction Management: Cranshaw Construction

A.3 - Project Description and Design Conditions

List the principal Building Uses: Residential Multifamily, Assembly

List the First Floor Uses: Residential Multifamily, Storage, Mechanical

List any Critical Site Infrastructure N/A

and or Building Uses:

Site and Building:

Site Area (SF): 49140 Building Area (SF): 120430 Building Height (Ft): 65 Building Height (Stories): 6

Existing Site Elevation - Low Existing Site Elevation - High 16.46

(Ft BCB): (Ft BCB):

Proposed Site Elevation - Low Proposed Site Elevation – High 16.46

(Ft BCB): (Ft BCB):

Proposed First Floor Elevation Below grade spaces/levels (#): 19.50

(Ft BCB):

Article 37 Green Building:

16.46

19.50

1



LEED Version - Rating System:	v4 BD+C New	LEED Certification:	No
	Construction and		
	Major Renovation		
Proposed LEED rating:	Silver	Proposed LEED point score (Pts.):	Minimum of 40 pts

Minimum of 40 pts

Building Envelope:

When reporting R values, differentiate between R discontinuous and R continuous. For example, use "R13" to show R13 discontinuous and use R10c.i. to show R10 continuous. When reporting U value, report total assembly U value including supports and structural elements.

supports and structural elements.			
Roof:	30ci	Exposed Floor:	30
Foundation Wall:	7.5ci	Slab Edge (at or below grade):	10
Vertical Above-grade Assemblies (%	's are of total vertical	area and together should total 100%):	
Area of Opaque Curtain Wall & Spandrel Assembly:	4	Wall & Spandrel Assembly Value:	0.38
Area of Framed & Insulated / Standard Wall:	75	Wall Value:	R20 and R7.5ci
Area of Vision Window:	16	Window Glazing Assembly Value:	varies per table, C402.4
		Window Glazing SHGC:	0.40 for S, E, W facing, 0.53 for N facing only (SHGC)
Area of Doors:	5	Door Assembly Value :	Opaque 0.37, 0.77 entry door

Energy Loads and Performance

For this filing – describe how energy loads & performance were determined	Peak Loads and performance were calculated based on prior modeled projects with similar building envelope, orientation, size and systems, where Carrier Hourly Analysis Program (HAP) was used for simulation.			
Annual Electric (kWh):	630000	Peak Electric (kW):	175	
Annual Heating (MMbtu/hr):	1200	Peak Heating (MMbtu):	1.2	
Annual Cooling (Tons/hr):	900	Peak Cooling (Tons):	95	
Energy Use - Below ASHRAE 90.1 - 2013 (%):	23	Have the local utilities reviewed the building energy performance?:	No	
Energy Use - Below Mass. Code (%):	23	Energy Use Intensity (kBtu/SF):	25	

Back-up / Emergency Power System

Electrical Generation Output (kW):	80	Number of Power Units:	1
System Type (kW):	combustion engine	Fuel Source:	natural gas



Electric (kW):	60	Heating (MMbtu/hr):	0.04
		Cooling (Tons/hr):	3

B - Greenhouse Gas Reduction and Net Zero / Net Positive Carbon Building Performance

Reducing greenhouse gas emissions is critical to avoiding more extreme climate change conditions. To achieve the City's goal of carbon-neutrality by 2050 the performance of new buildings will need to progressively improve to carbon net zero and net positive.

B.1 - GHG Emissions - Design Conditions

For this filing - Annual Building GHG Emissions (Tons):	2200	

For this filing - describe how building energy performance has been integrated into project planning, design, and engineering and any supporting analysis or modeling:

High performance building envelope, high-efficiency mechanical and lighting systems, and EnergyStar labeled appliances will be incorporated into the project to reduce the overall building energy usage.

Describe building specific passive energy efficiency measures including orientation, massing, building envelop, and systems:

High-performance building envelope will be provided for the project to reduce the heating and cooling loads of the buildings.

Describe building specific active energy efficiency measures including high performance equipment, controls, fixtures, and systems:

High-efficiency mechanical systems, light fixtures with high efficacy LED/CFL bulbs, and EnergyStar labeled appliances will be provided for the project to reduce the building energy usage. Provisions for PV panels will be provided for future integration (PV ready)

Describe building specific load reduction strategies including on-site renewable energy, clean energy, and storage systems:

Provisions for PV panels will be provided for future integration (PV ready)

Describe any area or district scale emission reduction strategies including renewable energy, central energy plants, distributed energy systems, and smart grid infrastructure:

N/A

Describe any energy efficiency assistance or support provided or to be provided to the project:



The project will participate in the MassSave New Construction Program for incentives related to the installation of efficiency equipment.

B.2 - GHG Reduction - Adaptation Strategies

Describe how the building and its systems will evolve to further reduce GHG emissions and achieve annual carbon net zero and net positive performance (e.g. added efficiency measures, renewable energy, energy storage, etc.) and the timeline for meeting that goal (by 2050):

Provisions for PV panels will be provided for future integration (PV ready) to further reduce the carbon footprint of the project.

C - Extreme Heat Events

Annual average temperature in Boston increased by about 2°F in the past hundred years and will continue to rise due to climate change. By the end of the century, the average annual temperature could be 56° (compared to 46° now) and the number of days above 90° (currently about 10 a year) could rise to 90.

C.1 - Extreme Heat - Design Conditions

Temperature Range - Low (Deg.):	7	Temperature Range - High (Deg.):	91			
Annual Heating Degree Days:	5512	Annual Cooling Degree Days	776			
What Extreme Heat Event characteristics will be / have been used for project planning						

what Extreme Heat Event characteristics will be / have been used for project planning

Days - Above 90° (#):	5	Days - Above 100° (#):	2
Number of Heatwaves / Year (#):	5	Average Duration of Heatwave (Days):	3

Describe all building and site measures to reduce heat-island effect at the site and in the surrounding area:

Design intent aims to include a combination of white roof areas, green roofs, low-reflective paved surfaces, and an expanded green space at the corner of Lubec and Gove streets.

C.2 - Extreme Heat - Adaptation Strategies

Describe how the building and its systems will be adapted to efficiently manage future higher average temperatures, higher extreme temperatures, additional annual heatwaves, and longer heatwaves:

Design intent aims to meet/exceed current Boston building energy efficiency code requirements, investigate various passive strategies, and improve the buildings' passive survivability performance.

Describe all mechanical and non-mechanical strategies that will support building functionality and use during extended interruptions of utility services and infrastructure including proposed and future adaptations:

Mechanicals will be set at or above base floor elevation (19.5 BCB)



D - Extreme Precipitation Events

From 1958 to 2010, there was a 70 percent increase in the amount of precipitation that fell on the days with the heaviest precipitation. Currently, the 10-Year, 24-Hour Design Storm precipitation level is 5.25". There is a significant probability that this will increase to at least 6" by the end of the century. Additionally, fewer, larger storms are likely to be accompanied by more frequent droughts.

D.1 – Extreme Precipitation - De	sign Conditions

What is the project design precipitation level? (In. / 24 Hours)

5.25

Describe all building and site measures for reducing storm water run-off:

Design intent will be the 10-year, 24-hour design storm (5.25" - 6") through infiltration and retention strategies.

D.2 - Extreme Precipitation - Adaptation Strategies

Describe how site and building systems will be adapted to efficiently accommodate future more significant rain events (e.g. rainwater harvesting, on-site storm water retention, bio swales, green roofs):

Design intent includes utilization of green roof space, expanded landscape area on the corner of Gove and Lubec streets, and further investigation of increased infiltration strategies across the site including rainwater harvesting.

E - Sea Level Rise and Storms

Under any plausible greenhouse gas emissions scenario, the sea level in Boston will continue to rise throughout the century. This will increase the number of buildings in Boston susceptible to coastal flooding and the likely frequency of flooding for those already in the floodplain.

Is any portion of the site in a FEMA Special Flood Hazard Area?	No	What Zone:	
What is the current FEMA SFHA Zone Base Flood Elevation for the site (Ft BCB)?			19.3

Is any portion of the site in the BPDA Sea Level Rise Flood Hazard Area (see <u>SLR-FHA online map</u>)? Yes



If you answered YES to either of the above questions, please complete the following questions.

Otherwise you have completed the questionnaire; thank you!

E.1 - Sea Level Rise and Storms - Design Conditions

Proposed projects should identify immediate and future adaptation strategies for managing the flooding scenario represented by the Sea Level Rise Flood Hazard Area (SLR-FHA), which includes 3.2' of sea level rise above 2013 tide levels, an additional 2.5" to account for subsidence, and the 1% Annual Chance Flood. After using the SLR-FHA to identify a project's Sea Level Rise Base Flood Elevation, proponents should calculate the Sea Level Rise Design Flood Elevation by adding 12" of freeboard for buildings, and 24" of freeboard for critical facilities and infrastructure and any ground floor residential units.

What is the Sea Level Rise - Base Flood Elevation for the site (Ft BCB)?	19.30		
What is the Sea Level Rise - Design Flood Elevation for the site (Ft BCB)?	21.30	First Floor Elevation (Ft BCB):	19.50
What are the Site Elevations at Building (Ft BCB)?	16.46	What is the Accessible Route Elevation (Ft BCB)?	19.50

Describe site design strategies for adapting to sea level rise including building access during flood events, elevated site areas, hard and soft barriers, wave / velocity breaks, storm water systems, utility services, etc.:

Design intent aims to investigate various building level mitigation strategies as described by the requirements, as well as work with landscape architecture and civil engineering to explore site level mitigation strategies.

Describe how the proposed Building Design Flood Elevation will be achieved including dry / wet flood proofing, critical systems protection, utility service protection, temporary flood barriers, waste and drain water back flow prevention, etc.:

Design intent aims to explore several methods of floor protection methods including a flood proof door to mechanical rooms, possible flood gate strategies, and consideration of elevating crucial building facilities.

Describe how occupants might shelter in place during a flooding event including any emergency power, water, and waste water provisions and the expected availability of any such measures:

Design intent includes investigating first aid and emergency supplies stored onsite in public common areas, tenant education program, preparedness coordination by property management, emergency generator for common area refrigeration and limited emergency power.

Describe any strategies that would support rapid recovery after a weather event:

Design intent aims to explore the use of hard surfaces where possible on ground floor elevations to facilitate quick cleanup as well as protecting building mechanical and electrical equipment from flood damage by raising equipment elevations.

E.2 - Sea Level Rise and Storms - Adaptation Strategies



Describe future site design and or infrastructure adaptation strategies for responding to sea level rise including future elevating of site areas and access routes, barriers, wave / velocity breaks, storm water systems, utility services, etc.:

The project is investigating stormwater infiltration system strategies to promote significant groundwater infiltration now and in the future.

Describe future building adaptation strategies for raising the Sea Level Rise Design Flood Elevation and further protecting critical systems, including permanent and temporary measures:

Design intent includes potential future adaptation of the access and amenity space to be located off of Frankfort Street including possible retrofit of the common area access.

Thank you for completing the Boston Climate Change Checklist!

For questions or comments about this checklist or Climate Change best practices, please contact: <u>John.Dalzell@boston.gov</u>

Attachment E

Accessibility Checklist

Article 80 - Accessibility Checklist

A requirement of the Boston Planning & Development Agency (BPDA) Article 80 Development Review Process

The Mayor's Commission for Persons with Disabilities strives to reduce architectural, procedural, attitudinal, and communication barriers that affect persons with disabilities in the City of Boston. In 2009, a Disability Advisory Board was appointed by the Mayor to work alongside the Commission in creating universal access throughout the city's built environment. The Disability Advisory Board is made up of 13 volunteer Boston residents with disabilities who have been tasked with representing the accessibility needs of their neighborhoods and increasing inclusion of people with disabilities.

In conformance with this directive, the BDPA has instituted this Accessibility Checklist as a tool to encourage developers to begin thinking about access and inclusion at the beginning of development projects, and strive to go beyond meeting only minimum MAAB / ADAAG compliance requirements. Instead, our goal is for developers to create ideal design for accessibility which will ensure that the built environment provides equitable experiences for all people, regardless of their abilities. As such, any project subject to Boston Zoning Article 80 Small or Large Project Review, including Institutional Master Plan modifications and updates, must complete this Accessibility Checklist thoroughly to provide specific detail about accessibility and inclusion, including descriptions, diagrams, and data.

For more information on compliance requirements, advancing best practices, and learning about progressive approaches to expand accessibility throughout Boston's built environment. Proponents are highly encouraged to meet with Commission staff, prior to filing.

Accessibility Analysis Information Sources:

- Americans with Disabilities Act 2010 ADA Standards for Accessible Design http://www.ada.gov/2010ADAstandards index.htm
- 2. Massachusetts Architectural Access Board 521 CMR http://www.mass.gov/eopss/consumer-prot-and-bus-lic/license-type/aab/aab-rules-and-regulations-pdf.html
- 3. Massachusetts State Building Code 780 CMR
 - $\underline{\text{http://www.mass.gov/eopss/consumer-prot-and-bus-lic/license-type/csl/building-codebbrs.html}}$
- 4. Massachusetts Office of Disability Disabled Parking Regulations http://www.mass.gov/anf/docs/mod/hp-parking-regulations-summary-mod.pdf
- MBTA Fixed Route Accessible Transit Stations
 http://www.mbta.com/riding the t/accessible services/
- 6. City of Boston Complete Street Guidelines http://bostoncompletestreets.org/
- City of Boston Mayor's Commission for Persons with Disabilities Advisory Board www.boston.gov/disability
- 8. City of Boston Public Works Sidewalk Reconstruction Policy http://www.cityofboston.gov/images documents/sidewalk%20policy%200114 tcm3-41668.pdf
- 9. City of Boston Public Improvement Commission Sidewalk Café Policy http://www.cityofboston.gov/images-documents/Sidewalk-cafes-tcm3-1845.pdf

Glossary of Terms:

- 1. Accessible Route A continuous and unobstructed path of travel that meets or exceeds the dimensional and inclusionary requirements set forth by MAAB 521 CMR: Section 20
- 2. Accessible Group 2 Units Residential units with additional floor space that meet or exceed the dimensional and inclusionary requirements set forth by MAAB 521 CMR: Section 9.4
- 3. *Accessible Guestrooms* Guestrooms with additional floor space, that meet or exceed the dimensional and inclusionary requirements set forth by MAAB 521 CMR: Section 8.4
- 4. *Inclusionary Development Policy (IDP)* Program run by the BPDA that preserves access to affordable housing opportunities, in the City. For more information visit: http://www.bostonplans.org/housing/overview
- Public Improvement Commission (PIC) The regulatory body in charge of managing the public right of way. For more information visit: https://www.boston.gov/pic
- 6. *Visitability* A place's ability to be accessed and visited by persons with disabilities that cause functional limitations; where architectural barriers do not inhibit access to entrances/doors and bathrooms.

1.	Project Information: If this is a multi-phased or multi	i-building project, fill	out a separate Checklist for	each pl	hase/building.
	Project Name:	Frankfort + Gove Street Housing Project			
	Primary Project Address:	115 Gove Street, 120 Gove Street, 128 Gove Street, 21-43 Frankfort Street			
	Total Number of Phases/Buildings:	2			
	Primary Contact (Name / Title / Company / Email / Phone):	Richard Egan/Frankfort Gove LLC / regan9999@gmail.com / 617-422-7000			
	Owner / Developer:	Frankfort Gove, LLC			
	Architect:	Bruner-Cott			
	Civil Engineer:	BSC Group			
	Landscape Architect:	Klopfer Martin Design Group			
	Permitting:	Epsilon Associates, Inc.			
	Construction Management:	Cranshaw Construction			
	At what stage is the project at time of this questionnaire? Select below:				
		PNF / Expanded PNF Submitted	Draft / Final Project Impact Report Submitted	BPDA I	Board Approved
		BPDA Design Approved	Under Construction	Constr	uction Completed:
	Do you anticipate filing for any variances with the Massachusetts Architectural Access Board (MAAB)? <i>If yes,</i> identify and explain.	No			
2.	2. Building Classification and Description: This section identifies preliminary construction information about the project including size and uses.				
	What are the dimensions of the proje	ct?			
	Site Area:	49,140 GSF	Building Area:		120,430 GSF
	Building Height:	Up to 65 FT.	Number of Stories:		Up to 6 Firs.
	First Floor Elevation:	19.5 Ft BCB	Is there below grade space	e:	Yes – Parking
	What is the Construction Type? (Select most appropriate type)				

	Wood Frame	Masonry	Steel Frame	Concrete
What are the principal building uses? (IBC definitions are below – select all appropriate that apply)				
	Residential – One - Three Unit	Residential - Multi- unit, Four +	Institutional	Educational
	Business	Mercantile	Factory	Hospitality
	Laboratory / Medical	Storage, Utility and Other		
List street-level uses of the building:	Residential units, amenity space, mechanical, storage			
Accomment of Evicting Infractructure for Accommission				

3. Assessment of Existing Infrastructure for Accessibility:

This section explores the proximity to accessible transit lines and institutions, such as (but not limited to) hospitals, elderly & disabled housing, and general neighborhood resources. Identify how the area surrounding the development is accessible for people with mobility impairments and analyze the existing condition of the accessible routes through sidewalk and pedestrian ramp reports.

Provide a description of the neighborhood where this development is located and its identifying topographical characteristics:

The area surrounding the Project site includes a mix of residential, commercial and institutional uses. Existing residences are located to the north, west and south of the site. These residences are generally three-stories with brick facades, typical of East Boston. Maverick Square, located within a half mile from the Project site, includes numerous restaurants, shops, access to public transportation, parks and open spaces. Open spaces in the area include Lombardi Memorial Park, Lewis Mall, East Boston Greenway, Piers Park Sailing Center, Brophy Park, and Sumner and Lamson Street Playground. The Project site is located within one half mile from the Maverick MBTA Blue Line station and is along the routes of multiple major bus lines. The Project site is also approximately two miles from Boston Logan International Airport. The proximity to public transit makes the area an ideal location for transit-oriented development.

List the surrounding accessible MBTA transit lines and their proximity to development site: commuter rail / subway stations, bus stops:

The Project site is located within a half mile of Maverick Square, a major MBTA for subway (blue line) and bus access. The Project site is situated along the paths of multiple major bus lines and approximately two miles from Logan airport.

List the surrounding institutions: hospitals, public housing, elderly and disabled housing developments, educational facilities, others: The Donald McKay School and East Boston Early Education Center, East Boston Neighborhood Health Center, Paris Street Community Center, Theodore Lyman School, East Boston Meditation Center

List the surrounding government buildings: libraries, community centers, recreational facilities, and other related facilities: East Boston Memorial Park, East Boston Memorial Stadium, East Boston Greenway, Maverick Square, Lombardi Memorial Park, Lewis Mall, Piers Park Sailing Center, Brophy Park, Sumner and Lamson Street Playground, Golden Stairs Park, East Boston Social Centers Playlot, Jeffries Point Neighborhood Association, Paris Street Pool

4. Surrounding Site Conditions - Existing:

This section identifies current condition of the sidewalks and pedestrian ramps at the development site.

Is the development site within a historic district? *If yes,* identify which district:

No

Are there sidewalks and pedestrian ramps existing at the development site? *If yes*, list the existing sidewalk and pedestrian ramp dimensions, slopes, materials, and physical condition at the development site:

At Gove/Frankfort eastern ramp is a non-compliant apex ramp, western ramp is compliant in eastern direction (tactile warning is gray not yellow) and northern direction is encumbered by utility pole and catch basin, and has no ramp. At Gove/Lubec, the ramp is non-compliant.

Are the sidewalks and pedestrian ramps existing-to-remain? *If yes,* have they been verified as ADA / MAAB compliant (with yellow composite detectable warning surfaces, cast in concrete)? *If yes,* provide description and photos:

Sidewalks are frequently not compliant and will likely be rebuilt to accommodate street tree planting and repair pavement. No compliant tactile warning tiles are present.

5. Surrounding Site Conditions - Proposed

This section identifies the proposed condition of the walkways and pedestrian ramps around the development site. Sidewalk width contributes to the degree of comfort walking along a street. Narrow sidewalks do not support lively pedestrian activity, and may create dangerous conditions that force people to walk in the street. Wider sidewalks allow people to walk side by side and pass each other comfortably walking alone, walking in pairs, or using a wheelchair.

Are the proposed sidewalks consistent with the Boston Complete Street Guidelines? *If yes*, choose which Street Type was applied: Downtown Commercial, Downtown Mixed-use, Neighborhood Main, Connector, Residential, Industrial, Shared Street, Parkway, or Boulevard.

The project falls into the Neighborhood Residential category of the Complete Streets guidelines, and are consistent with those guidelines.

What are the total dimensions and slopes of the proposed sidewalks? List the widths of the proposed zones: Frontage, Pedestrian and Furnishing Zone:

They will have a 1.5% cross slope generally, and 2% maximum where necessary. Sidewalk width minimum is 7 feet.

List the proposed materials for each Zone. Will the proposed materials be on private property or will the proposed materials be on the City of Boston pedestrian right-of-way?	Precast concrete pavers are proposed in the furniture/landscape zone only, on Gove street, with broom finish concrete in all other sidewalk areas and to allow accessibility per Boston standards.
Will sidewalk cafes or other furnishings be programmed for the pedestrian right-of-way? <i>If yes,</i> what are the proposed dimensions of the sidewalk café or furnishings and what will the remaining right-of-way clearance be?	No furnishings are planned for sidewalk areas.
If the pedestrian right-of-way is on private property, will the proponent seek a pedestrian easement with the Public Improvement Commission (PIC)?	Pedestrian ROW is all on public ROW.
Will any portion of the Project be going through the PIC? <i>If yes,</i> identify PIC actions and provide details.	Pavers, handicap ramps, and street tree planting will require PIC approval and permitting.
6. Accessible Parking:	
Parking Regulations.	and the Massachusetts Office of Disability – Disabled
See Massachusetts Architectural Access Board Ru regarding accessible parking requirement counts a	_
See Massachusetts Architectural Access Board Ruregarding accessible parking requirement counts a Parking Regulations. What is the total number of parking spaces provided at the development site? Will these be in a parking lot or	Total project will include 84 parking spaces – 13 surface spaces located off of Lubec Street adjacent to the Church Building, and 71 located in the below-grade parking
See Massachusetts Architectural Access Board Ruregarding accessible parking requirement counts a Parking Regulations. What is the total number of parking spaces provided at the development site? Will these be in a parking lot or garage? What is the total number of accessible spaces provided at the development site? How many of these are "Van	Total project will include 84 parking spaces – 13 surface spaces located off of Lubec Street adjacent to the Church Building, and 71 located in the below-grade parking garage. 1 located at the Church Building surface parking lot. 3 located below-grade in the Frankfort Street garage. The
See Massachusetts Architectural Access Board Ruregarding accessible parking requirement counts a Parking Regulations. What is the total number of parking spaces provided at the development site? Will these be in a parking lot or garage? What is the total number of accessible spaces provided at the development site? How many of these are "Van Accessible" spaces with an 8 foot access aisle? Will any on-street accessible parking spaces be required? If yes, has the proponent contacted the Commission for	Total project will include 84 parking spaces – 13 surface spaces located off of Lubec Street adjacent to the Church Building, and 71 located in the below-grade parking garage. 1 located at the Church Building surface parking lot. 3 located below-grade in the Frankfort Street garage. The design will incorporate an 8-foot access aisle. Requirement will be determined after review with the

accessible?	will be accessible.	
7. Circulation and Accessible Routes: The primary objective in designing smooth and continuous paths of travel is to create universal access to entryways and common spaces, which accommodates persons of all abilities and allows for visitability with neighbors.		
Describe accessibility at each entryway: Example: Flush Condition, Stairs, Ramp, Lift or Elevator:	Church renovation: There will be a flush entry to the stairs and an accessible ramp. Frankfort main entrance: Stairs and accessible ramp to flush entry. Frankfort street entrance: Flush entry to stairs and lift	
Are the accessible entrances and standard entrance integrated? <i>If yes,</i> describe. <i>If no,</i> what is the reason?	Yes, they are located together at the main entrances. One at the Church renovation, and two at the Frankfort Street building.	
If project is subject to Large Project Review/Institutional Master Plan, describe the accessible routes way-finding / signage package.	Large project review. Accessible routes across the project will be provided from the primary entrances to the main accessible elevator access points. Signage compliant with 521 CMR will be provided for residents and visitors for clear way-finding.	
8. Accessible Units (Group 2) and Guestrooms: (If applied in order to facilitate access to housing and hospital units that are proposed for the development site the	lity, this section addresses the number of accessible	
What is the total number of proposed housing units or hotel rooms for the development?	112 units.	
If a residential development, how many units are for sale? How many are for rent? What is the breakdown of market value units vs. IDP (Inclusionary Development Policy) units?	All units will be condominiums for sale. 13% of the units will be IDP (15 units)	
If a residential development, how many accessible Group 2 units are being proposed?	Project will meet the requirement for Group 2 units.	
If a residential development, how many accessible Group 2 units will also be IDP units? If none, describe reason.	Will meet the BPDA requested 5% of Group 2.	
If a hospitality development, how many accessible units will feature a wheel-in shower? Will accessible equipment be provided as well? If yes, provide amount and location of equipment.	Not applicable.	

Do standard units have architectural barriers that would prevent entry or use of common space for persons with mobility impairments? Example: stairs / thresholds at entry, step to balcony, others. <i>If yes</i> , provide reason.	First floor units along Frankfort street have two entry points. One is a stoop with stairs and landing on Frankfort street. The other is an accessible entry off the accessible route to the accessibly building entrances and elevator.
Are there interior elevators, ramps or lifts located in the development for access around architectural barriers and/or to separate floors? <i>If yes</i> , describe:	There will be an accessible ramp and a lift located at the first floor of the new building along Frankfort st. to provide access from grade to first floor and main elevators. The church renovation will provide a ramp at the main entry to access the lower level and main elevator.
9. Community Impact: Accessibility and inclusion extend past required co scheme that allows full and equal participation of passet to the surrounding community.	mpliance with building codes. Providing an overall persons with disabilities makes the development an
Is this project providing any funding or improvements to the surrounding neighborhood? Examples: adding extra street trees, building or refurbishing a local park, or supporting other community-based initiatives?	The Project is proposing additional street trees along Frankfort, Gove, and Lubec streets. the project will also create a green space on the corner of Gove and Lubec streets.
What inclusion elements does this development provide for persons with disabilities in common social and open spaces? Example: Indoor seating and TVs in common rooms; outdoor seating and barbeque grills in yard. Will all of these spaces and features provide accessibility?	All common and social spaces will be accessible.
Are any restrooms planned in common public spaces? If yes, will any be single-stall, ADA compliant and designated as "Family"/ "Companion" restrooms? If no, explain why not.	Yes, there will be single stall, unisex accessible restrooms.
Has the proponent reviewed the proposed plan with the City of Boston Disability Commissioner or with their Architectural Access staff? <i>If yes,</i> did they approve? <i>If no,</i> what were their comments?	The Project has not been reviewed with the Disability Commissioner or the Architectural staff.
Has the proponent presented the proposed plan to the Disability Advisory Board at one of their monthly meetings? Did the Advisory Board vote to support this project? <i>If no,</i> what recommendations did the Advisory	The Project has not been presented to the disability advisory board.

Board give to make this project more accessible?	The Project has not been presented to the disability advisory board.			
10 Attachments				
10. Attachments Include a list of all documents you are submitting with this Checklist. This may include drawings, diagrams, photos, or any other material that describes the accessible and inclusive elements of this project.				
Provide a diagram of the accessible routes to and from the accessible parking lot/garage and drop-off areas to the development entry locations, including route distances.				
Provide a diagram of the accessible route connections through the site, including distances.				
Provide a diagram the accessible route to any roof decks or outdoor courtyard space? (if applicable)				
Provide a plan and diagram of the accessible Group 2 units, including locations and route from accessible entry.				
Provide any additional drawings, diagrams, photos, or any of elements of this project. • • • • •	other material that describes the inclusive and accessible			

This completes the Article 80 Accessibility Checklist required for your project. Prior to and during the review process, Commission staff are able to provide technical assistance and design review, in order to help achieve ideal accessibility and to ensure that all buildings, sidewalks, parks, and open spaces are usable and welcoming to Boston's diverse residents and visitors, including those with physical, sensory, and other disabilities.

For questions or comments about this checklist, or for more information on best practices for improving accessibility and inclusion, visit www.boston.gov/disability, or our office:

The Mayor's Commission for Persons with Disabilities 1 City Hall Square, Room 967, Boston MA 02201.

Architectural Access staff can be reached at:

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